

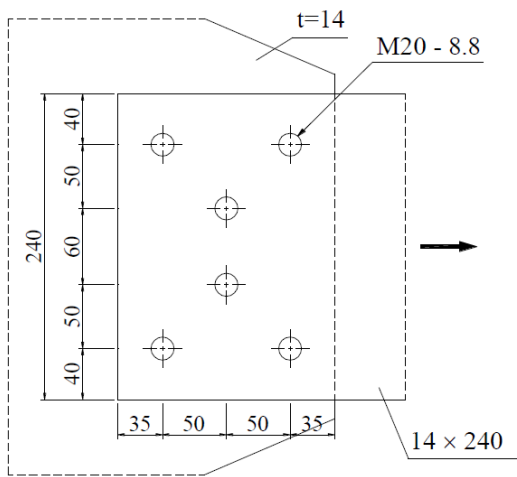


Figure 1.

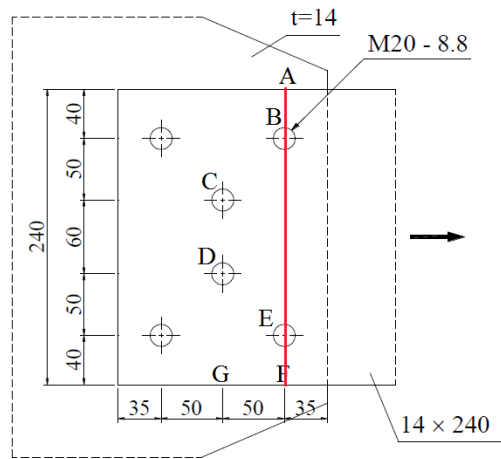


Figure 2.

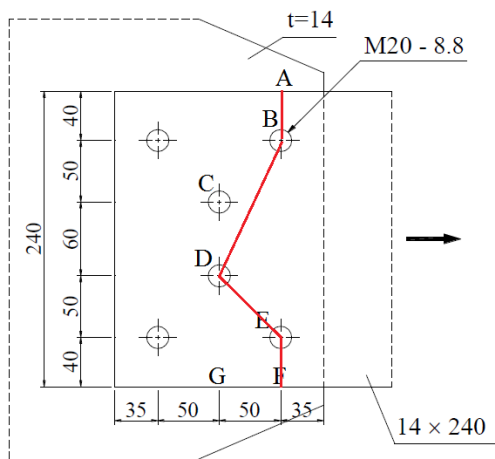


Figure 3.

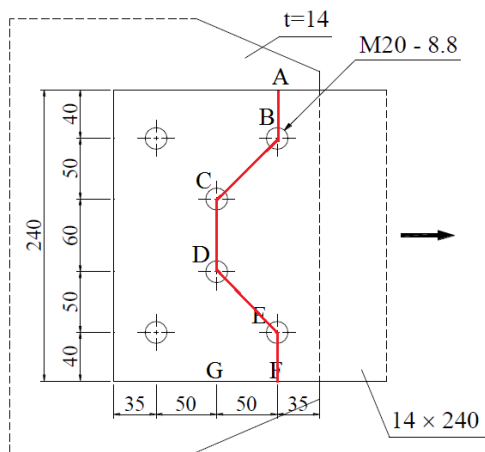


Figure 4.

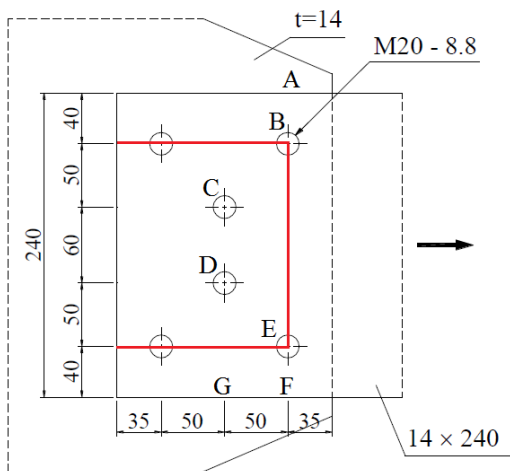


Figure 5.

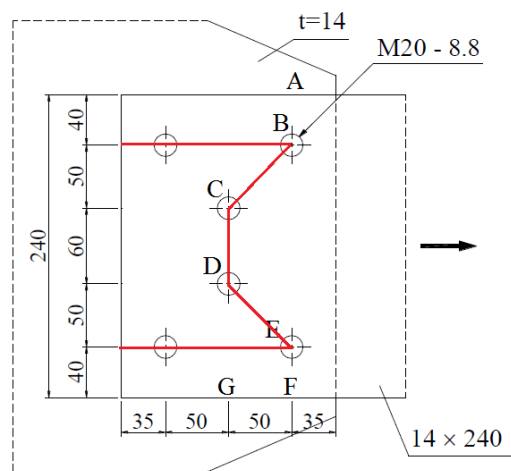


Figure 6.

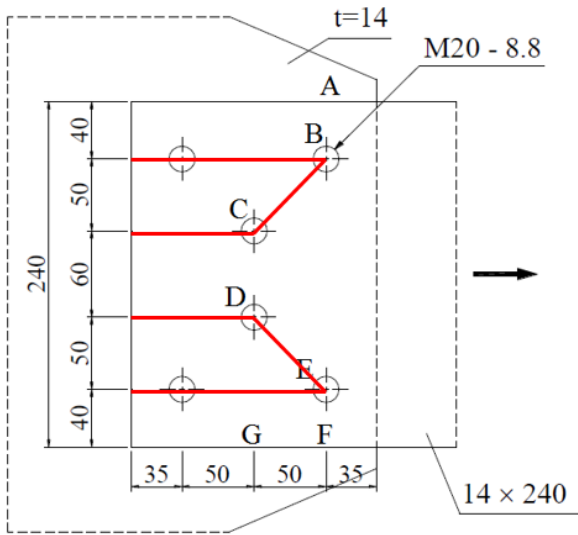


Figure 7.

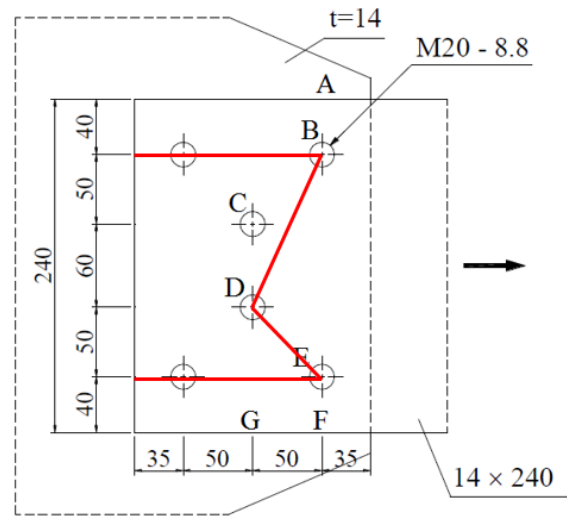


Figure 8.

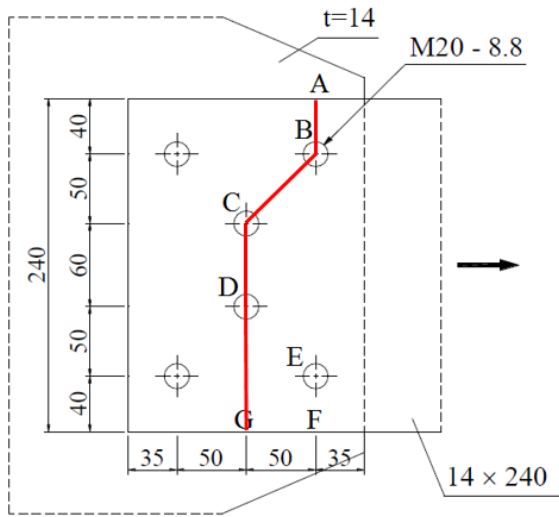


Figure 9.

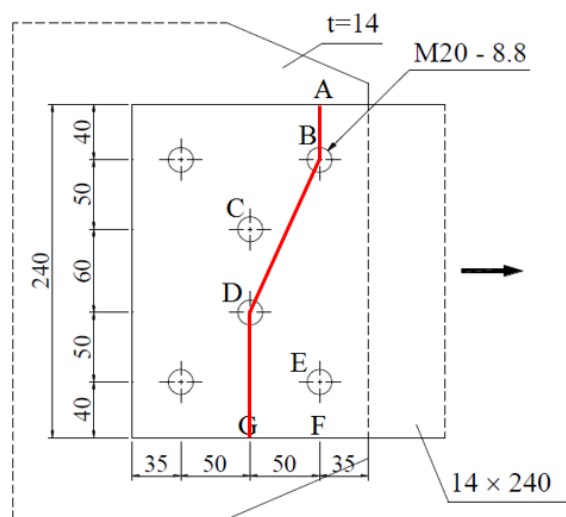


Figure 10.

A tension member, a 14×240 plate, is connected to a gusset plate with staggered bolts as shown in Figure 1. Determine the design strength of the member.

Bolt: M20 (standart bolt hole)

Steel: S275 ($F_y = 27,5 \text{ kN/cm}^2$, $F_u = 43 \text{ kN/cm}^2$)

Şekil 1'de görüldüğü gibi, 14×240 boyutlarındaki çekme üyesi pasırtmalı yerleştirilmiş bulonlar ile mesnet levhasına bağlanmıştır. Çekme üyesinin tasarım dayanımını hesaplayınız.

Bulon: M20 (standart bulon deliği)

Çelik: S275 ($F_y = 27,5 \text{ kN/cm}^2$, $F_u = 43 \text{ kN/cm}^2$)

$$d = 20 \quad d_h = 22 \quad d_e = 22 + 2 = 24$$

$$A_n = A_g - \sum d_e \cdot t + \sum \frac{s^2 t}{4g}$$

→ Akma (Yielding)

$$A_g = 24 \cdot 1,4 = 33,6 \text{ cm}^2$$

$$T_n = 27,5 \cdot 33,6 = 924 \text{ kN}$$

$$\phi_t T_n = 0,90 \cdot 924 = 831,6 \text{ kN}$$

→ Kopma (Fracture)

$$A_n = ?$$

• Case-1 (Figure-2) (A-B-E-F)

$$A_n = 24 \cdot 1,4 - 2 \cdot 2,4 \cdot 1,4 = 26,88 \text{ cm}^2$$

• Case-2 (Figure-3) (A-B-D-E-F)

$$A_n = 24 \cdot 1,4 - 3 \cdot 2,4 \cdot 1,4 + \frac{5^2 \cdot 1,4}{4 \cdot 5} + \frac{5^2 \cdot 1,4}{4 \cdot 11} = 26,07 \text{ cm}^2$$

• Case-3 (Figure-4) (A-B-C-D-E-F)

$$A_n = 24 \cdot 1,4 - 4 \cdot 2,4 \cdot 1,4 + 2 \cdot \frac{5^2 \cdot 1,4}{4 \cdot 5} = 23,66 \text{ cm}^2$$

$$\min A_n = 23,66 \text{ cm}^2 \quad U = 1 \quad A_e = 23,66 \text{ cm}^2$$

$$T_n = F_u \cdot A_e = 43 \cdot 23,66 = 1017,38 \text{ kN}$$

$$\phi_t T_n = 0,75 \cdot 1017,38 = 763,04 \text{ kN}$$

→ Blok Kiri-ma (Block Shear)

• Case-1 (Figure-5)

$$A_{gv} = 2 \cdot (5 + 5 + 3,5) \cdot 1,4 = 37,80 \text{ cm}^2$$

$$A_{nv} = 2 \cdot (5 + 5 + 3,5 - 2,4 - 1,2) \cdot 1,4 = 27,72 \text{ cm}^2$$

$$A_{nt} = (5 + 6 + 5 - 1,2 - 1,2) \cdot 1,4 = 19,04 \text{ cm}^2$$

$$R_n = 0,60 F_u A_{nv} + U_{bs} F_u A_{nt} \leq 0,60 F_y A_{gv} + U_{bs} F_u A_{nt}$$

$$\left. \begin{array}{l} 0,60 \cdot 43 \cdot 27,72 + 1,43 \cdot 19,04 = 1533,9 \text{ kN} \\ 0,60 \cdot 27,5 \cdot 37,80 + 1,43 \cdot 19,04 = 1442,4 \text{ kN} \end{array} \right\} R_n = 1442,4 \text{ kN}$$

$$\phi R_n = 0,75 \cdot 1442,4 = 1081,8 \text{ kN}$$

• Case-2 (Figure-6)

$$A_{gv} = 2 \cdot (3,5 + 5 + 5) \cdot 1,4 = 37,80 \text{ cm}^2$$

$$A_{nv} = 2 \cdot (3,5 + 5 + 5 - 2,4 - 1,2) \cdot 1,4 = 27,72 \text{ cm}^2$$

$$A_{nt} = 23,66 - 2 \cdot (4 - 1,2) \cdot 1,4 = 15,82 \text{ cm}^2$$

↓
(from staggered case-3)

$$\left. \begin{array}{l} R_n = 0,60 \cdot 43 \cdot 27,72 + 1,43 \cdot 15,82 = 1395,44 \text{ kN} \\ 0,60 \cdot 27,5 \cdot 37,80 + 1,43 \cdot 15,82 = 1303,96 \text{ kN} \end{array} \right\} R_n = 1303,96 \text{ kN}$$

$$\phi R_n = 0,75 \cdot 1303,96 = 977,97 \text{ kN}$$

• Case-3 (Figure-7)

$$A_{gv} = 2 \cdot (5 + 5 + 3,5) \cdot 1,4 + 2 \cdot (5 + 3,5) \cdot 1,4 = 61,6 \text{ cm}^2$$

$$A_{nv} = 61,6 - 2 \cdot 2,4 \cdot 1,4 - 4 \cdot 1,2 \cdot 1,4 = 48,16 \text{ cm}^2$$

$$A_{nt} = 23,66 - 2 \cdot (4 - 1,2) \cdot 1,4 - (6 - 2 \cdot 1,2) \cdot 1,4 = 10,78 \text{ cm}^2$$

↓
(from staggered case-3)

$$\left. \begin{array}{l} R_n = 0,60 \cdot 43 \cdot 48,16 + 1,43 \cdot 10,78 = 1706,07 \text{ kN} \\ 0,60 \cdot 27,5 \cdot 61,6 + 1,43 \cdot 10,78 = 1479,94 \text{ kN} \end{array} \right\} R_n = 1479,94 \text{ kN}$$

$$\phi R_n = 0,75 \cdot 1479,94 = 1109,96 \text{ kN}$$

• Case-4 (Figure-8)

$$A_{gv} = 2 \cdot (3,5 + 5 + 5) \cdot 1,4 = 37,80 \text{ cm}^2$$

$$A_{nv} = 2 \cdot (3,5 + 5 + 5 - 2,4 - 1,2) \cdot 1,4 = 27,72 \text{ cm}^2$$

$$A_{nt} = \underline{26,07} - 2 \cdot (4 - 1,2) \cdot 1,4 = 18,23 \text{ cm}^2$$

(from staggered case-2)

$$\left. \begin{aligned} R_n &= 0,60 \cdot 43 \cdot 27,72 + 1 \cdot 43 \cdot 18,23 = 1499,07 \text{ kN} \\ 0,60 \cdot 27,5 \cdot 37,80 + 1 \cdot 43 \cdot 18,23 &= 1407,59 \text{ kN} \end{aligned} \right\} R_n = 1407,59 \text{ kN}$$

$$\phi R_n = 0,75 \cdot 1407,59 = 1055,69 \text{ kN}$$

Design strength: **763,04 kN**

= Additional Information about Net-Section =
(Net Kesit Hakkında İlave Bilgi)

→ Kapma (Fracture)

• Case-4 (Figure-9) (A-B-C-D-G)

$$A_n = 24 \cdot 1,4 - 3 \cdot 2,4 \cdot 1,4 + \frac{5^2 \cdot 1,4}{4 \cdot 5} = 25,27 \text{ cm}^2$$

Assume that the member is subjected to P load. Therefore, each bolt transfers $P/n = P/6$ load. (n is number of bolts)

Because $P/6$ of the load has been transferred from the gusset plate by the bolt at E, this potential failure line must resist only $5P/6$ load. Therefore, the net area of Case-4 should be multiplied by $6/5$ to obtain a net area that can be compared with previous lines that resist the full load.

$$A_n = 25,27 \cdot \frac{6}{5} = 30,32 \text{ cm}^2$$

Gelme grubunun P dış yüküne maruz kaldığı dikkate alınsın. Bu durumda, her bir beton $P/n = P/6$ yükü aktaracaktır. (n beton sayıdır)

E noktasındaki betonun üzerine düşen $P/6$ 'lık kuvvet mesnet levhasına aktarmış olmasından dolayı, dikkate alınan olası kırılma çizgisi $5P/6$ 'lık kuvvete maruz kalacaktır. Bu nedenle, dış kuvvetin tamamına maruz kalan daha önceki kırılma durumlarıyla kıyaslanabilirliği için, bu kırılma çizgisinin net alanı $6/5$ ile çarpılmalıdır.

• Case-5 (Figure-10) (A-B-D-G)

$$A_n = 24 \cdot 1,4 - 2 \cdot 2,4 \cdot 1,4 + \frac{5^2 \cdot 1,4}{4 \cdot 11} = 27,68 \text{ cm}^2$$

$$A_n = 27,68 \cdot \frac{6}{5} = 33,22 \text{ cm}^2$$