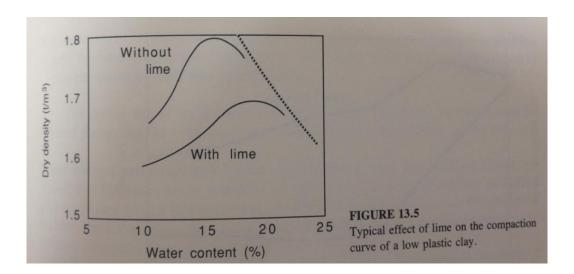
SOIL IMPROVEMENT

HW #1

- 1. Write down the laboratory tests that may be performed to determine the strength properties of compacted clay soils.
- **2.** The wearing surface of the highway pavement may be damaged due to swelling-shrinkage potential of the subbase, subgrade and base course of the highway pavement. What kind of stabilization methods can be applied to prevent or reduce this potential?
- **3.** According to sieve analysis and consistency limit test results, a soil sample is characterized as SC according to USCS classification system. By using the information given in Table 4.1 and Table 6.1, answer the following questions:
 - **a.** Give approximate ranges of typical properties of this type of soil if it is compacted (Use Table 4.1).
 - **b.** Rank its suitability as a fill material for a water retaining earth dam (Use Table 6.1).
- **4.** Write down two main reasons for the use of bitumen as a stabilization admixture.
- 5. The compaction results of two similar clay samples with lime and without lime are given in the below figure. According to the given experimental results, answer the following questions:
 - a. How the optimum water content of the clay changes with lime addition?
 - b. Compare the compaction curves obtained from two samples and explain the difference with reasons.



6. Considering the engineering properties of fly ash, answer/complete the following

- The average grain size (D₅₀) of the fly ash is
- The plasticity index of fly ash is
- The friction angle of fly ash is
- The compaction properties of fly ash
- The compressibility properties of fly ash.....
- The permeability of fly ash

		Relative desirability for various uses (no. 1 is considered the best; no. 14 is least desirable)											
Group symbol		Rolled earth fill dams			Canal sections		Foundations		Roadways				
	Soil type	Homog. Embank- ment	Core	Shell	Erosion resistance	Comp. earth lining	Seepage important	Seepage not important	Frost heave not poss. (fills)	Frost heave possible (fills)	Surfacin		
GW	Well-graded gravels, gravel-sand mixtures, little or no fines	-	-	1	1	-	-	1	1	1	3		
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines		-	2	2	-	-	3	3	3	-		
iM.	Silty gravels, poorly graded gravel-sand-silt mixtures	2	4	-	4	4	1	4	4	9	5		
3C	Clayey gravels, poorly graded gravel-sand-clay mixtures	1	1	-	3	1	2	6	5	5	1		
sw	Weil-graded sands, gravelly sands, little or no fines	-	-	3 if gravelly	6	-	-	2	2	2	4		
SP	Poorly graded sands, gravelly sands, little or no tines.	-	-	4 if gravelly	7 if gravelly	1	-	5	6	4			

SM	Silty sands, poorly graded sand-silt mixtures	+	5	-	8 if gravelly	5 erosion critical	3	7	6	10	6
SC	Clayey sands, poorly graded sand-clay mixtures	3	2	-	5	2	4	8	7	6	2
MEL	Inorganic silts and very fine sands, rock floor, silty or clayey fine sands with slight plasticity	6	6	-		6 erosion critical	6	9	10	11	
CL.	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, sitty clays, lean clays	5	3	-	9	3	5	10	9	7	7
OL	Organic silts and organic silt-clays of low plasticity		8	-	7	7 erosion critical	7	11	11	12	-
мн	Inorganic silts, micaceous or diatamaceous fine sandy or silty soils, elastic silts	9	9	-		-	8	12	12	13	-
СН	Inorganic clays of high plasticity, fat clays	7	7	-	10	8 volume change critical	9	13	13	8	-
OH	Organic clays of medium-high plasticity	10	10	-	-	-	10	14	14	14	

				Typical value of compression		Typical strength characteristics							
Group		Range of max. dry unit weight, t/m²	Range of optimum moisture,	At about 140 kPa, % orig. beight	At about 380 kPa, % orig. height	Cohesion (as com- pacted), kPa	Cohesion (saturated), kPa	φ' (effective stress envelope), degrees	tan 🏕	Typical coeff. of permeability, m/s	Range of CBR values	Range of subgrade modulus k,×1000 kN/m²	
GW GW	Well-graded clean gravels, gravel-sand mix	2.0-2.2	11-8	0.3	0.6	0	0	> 38	> 0.79	10-5	40-80	80-140	
GP	Poorly graded clean gravels, gravel-sand mix	1.8-2.0	14-11	0.4	0.9	0	0	> 37	> 0.74	5 × 10 ⁻⁵	30-60	70-110	
GM	Silty gravels, poorly graded gravel-sand silt	1.9-2.2	12-8	0.5	1.1	-	-	> 34	> 0.67	> 5 × 10 ⁻¹⁰	20-60	30-110	
GC GC	Clayey gravels, poorly graded gravel-sand clay	1.8-2,1	14-9	0.7	1.6	-	-	> 31	> 0.60	> 5 × 10-1	20-4	0 30-80	
w	Well-graded clean sands, gravelly sands	1.8-2.1	16-9	0.6	1.2	0	0	38	0.7	9 > 5 × 10	7 20-	40 55-80	
	Poorly graded clean sands, sands, sand-gravel mix	1.6-1.9	21-12	0.8	1.4	0	0	37	0.	74 > 5 × 10	-7 10	-40 55-8	
	Silty sands, poorly graded sand-silt mix	1.8-2.0	16-11	0.8	1.6	50	20	34	0	67 > 10-8	10	30-	
-SC	Sand-silt clay mix with slightly plastic fines	1.8-2.1	15-11	0.8	1.4	50	14	33	0	.66 > 10-9	5	-30 30-	
	Clayey sands, poorly graded	1.7-2.0	19–11	1,1	2.2	75	11	31	0	.60 > 10-10	5.	-20 30-	
- 34	sand-clay mix					The same of the sa		32	0	62 > 5 × 10	15	30-5	

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Inorganic silts and clayey silts

LADLE	4.1 (COM.)							1		1	-	
ML-CL	Mixture of inorganic silt and clay	1.6-1.9	22-12	1.0	2.2	65	22	32	0.62	> 10 ⁻¹⁰	-	
CL	Inorganic clays of low to medium plasticity	1.5-1.9	24-12	1.3	2.5	85	13	28	0.54	> 5 × 10 ⁻¹¹ or less	15	15-55
OL	Organic silts and silt-clays, low plasticity	1.3-1.6	33-21	-	-	-		-	-	-	5 or less	15-30
МН	Inorganic clayey silts, elastic silts	1.1-1.5	40-24	2.0	3.8	70	20	25	0.47	> 10-10	10 or less	15-30
СН	Inorganic clays of high plasticity	1.2-1.7	36–19	2.6	3.9	105	11	19	0.35	> 5 × 10 ⁻¹¹	15 or less	15-40
OH	Organic clays and silty clays	1.0-1.6	45-21	-	-	-	-	-	-	-	5 or less	5-30

ML

- Source: Adapted from "Design Manual 7.2," U.S. Navy, 1982.

 Notes:

 1. All properties are for condition of standard Proctor maximum density, except values of k, and CBR which are for modified Proctor maximum density.

 2. Typical strength characteristics are for effective strength envelopes and are obtained from U.S.S.R. data.

 3. Compression values are for vertical loading with complete lateral confinement.

 4. ">" indicates that the typical property is greater than the value shown.

 5. "—" indicates that insufficient data is available for an estimate.

1.5-1.9