

SÖNLU ELEMANLAR METODU DERS NOTLARI

**DOÇ. DR. ALİ KOÇAK
YILDIZ TEKNİK ÜNİVERSİTESİ**

2015

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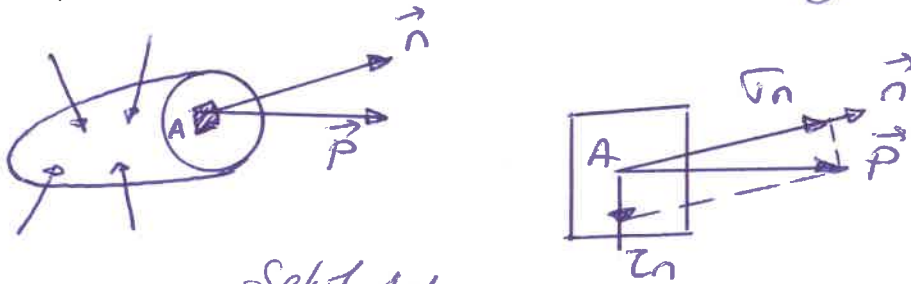
BÖLÜM I

ELASTİSİTE BAĞINTILARI

1) GERİLME HALİ

1.1) GİRİŞ: Denge durumundaki bir sistemde dış kuvvetler ile iç kuvvetler denge halindedir. Dış yüklerin etkisi altındaki bir sistemin enine kesiti üzerinde iç kuvvetler kesit yüzeyi boyunca sürekli bir tarzda yayılı olarak bulunduğu düşünülür. Bu kesit yüzeyi üzerinde birim alana gelen sıddak gerilme denir.

Sistemin gelişigüzel enine yönde kesilmesi sonucunda ortaya parçalarındaki gerilme vektörü yüzeye dik değildir (Şekil 1.1). Bu gerilmeye eğik gerilme vektörü denir.

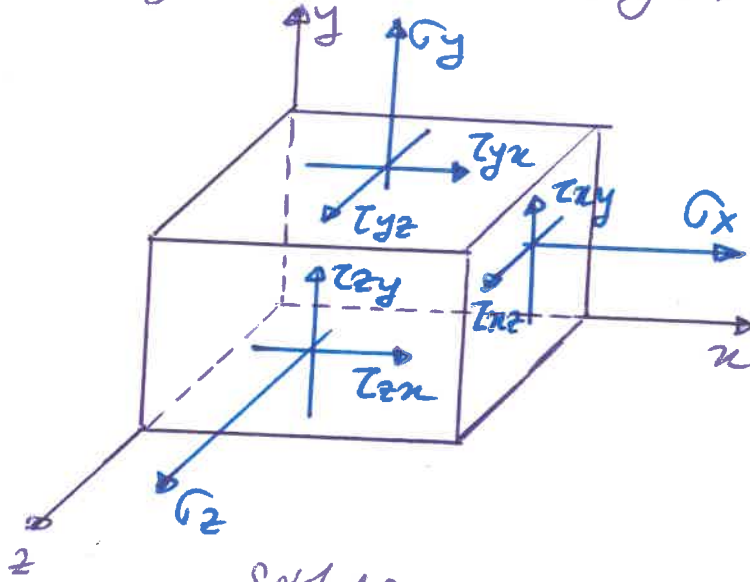


Şekil 1.1.

Eğik gerilme vektörünün, yüzeyin normaline doğrultusundaki bileşenine, başka bir deyişle yüzeye dik bileşenine normal gerilme (σ_n), yüzeyin teğet doğrultusundaki bileşenine de kayma gerilmesi (τ_n) denir (Şekil 1.1).

Şekil 1.1 'de verildiği gibi doğrultu normali ($\vec{n}$) değiştiğinde $\vec{p}$ gerilme vektörü de değişecektir. Dolayısıyla $\vec{n}$ ile $\vec{p}$ arasında bir bağıntı mümkündür ve bu da tek bir büyüklük olarak tanımlanabilir.

Buna göre hali, matematiksel olarak da adına gerilme tenörü denir. Buradan da anlaşılacağı gibi bu gerilme vektörü veya gerilme tenörü birbirlerinden bağımsız değişirler. Şekil 1.1'de verilen A noktasının gerilme hali için σ_x vektörel büyüklüğü veya dokuz skaler büyüklük gerekmektedir. Şekil 1.2'de de verildiği gibi prizmatik elemanın gerilme tenörü 9 gerilme bileşenlidir.



Şekil 1.2.

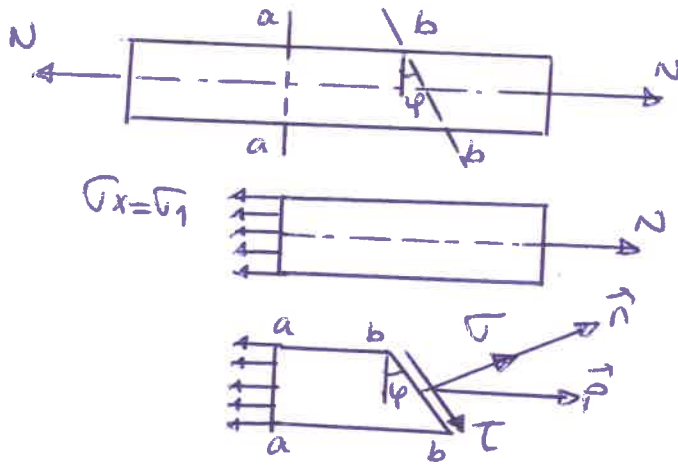
$$[\sigma] = \begin{bmatrix} \sigma_x & \tau_{xy} & \tau_{xz} \\ \tau_{yx} & \sigma_y & \tau_{yz} \\ \tau_{zx} & \tau_{zy} & \sigma_z \end{bmatrix}$$

Gerilme tenöründe normal gerilmeler doğrudan doğruya göre tek indekse belirlenir. Kayma gerilmeleri de birinci indeks kayma gerilmelerinin normalini, ikinci indeks kayma gerilmelerinin yönünü olmak üzere iki indekse belirlenir. Dokuz skaler büyüklükten oluşan gerilme tenörü simetrik olarak göre simetriklik ve altı bağımsız skalerle belirlenir.

$|\tau_{xy}| = |\tau_{yx}|$; $|\tau_{xz}| = |\tau_{zx}|$; $|\tau_{yz}| = |\tau_{zy}|$
Özel durumlarda bu altı skaler büyüklük daha az sayı ile belirlenebilir. Bu gerilme halleri 1,2 ve 3 eksenli gerilme halleridir.

1.2) Birk Elemanl' Gerilme Hall'

Birk elemanın ayırma kesitindeki gerilme vektörlerinin doğrudanlalarının aynı okluğu gerilme halidir. Birk başka tanıma göre; elastik çekme veya basınca maruz birk çubugun birk kesitinde meydana gelen normal gerilmeler (σ) eksen doğrudanına paraleldir. Bu doğrudanına dik kayma gerilmeleri (τ) ise sıfırdır (Şekil 1.3).

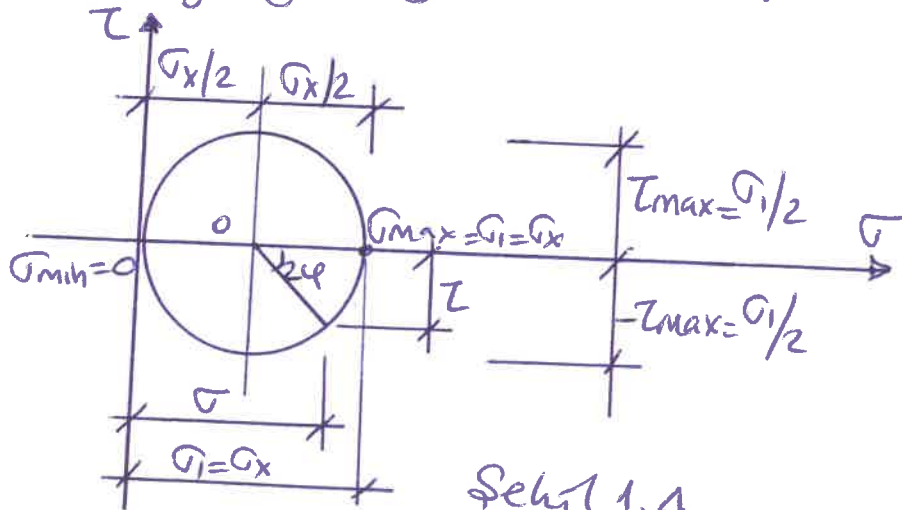


$$\sigma_x = \sigma_1 = \frac{N}{A}$$

$$[\tau_g] = [\sigma_x] = [\sigma_1] = \begin{bmatrix} \sigma_x & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

Şekil 1.3

Şekilde 'de görüleceği gibi aynı eleman üzerinde b-b kesitinde oluşacak gerilmeler de σ ve τ 'dir. Burada $\sigma = \sigma_1 \cdot \cos^2 \varphi$, $\tau = -\sigma_1 \cdot \sin \varphi \cos \varphi$ 'dır. Birk elemanl gerilme halindeki durumunu mohr daireesinde Şekil 1.4 'de verildiği gibi gösterebiliriz.



Şekil 1.4

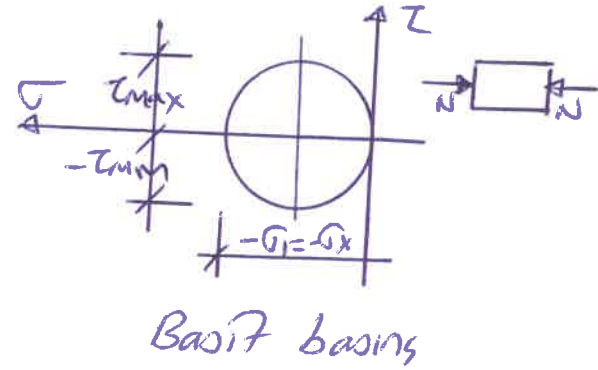
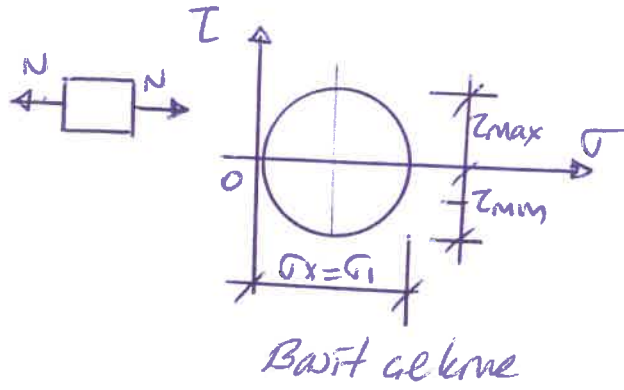
$$\sigma_{\max} = \sigma_1 = \sigma_x$$

$$\sigma_{\min} = 0$$

$$\tau_{\max} = \frac{\sigma_1}{2}$$

$$\tau_{\min} = -\frac{\sigma_1}{2}$$

Görüşlüğü gibi bir eksenli gerilme halinde matriks daireler başlangıç noktasından geçmektedir. Ayrıca bu gerilme halinde basit çekme veya basit basınç durumu da denilmektedir (Şekil 1.5).

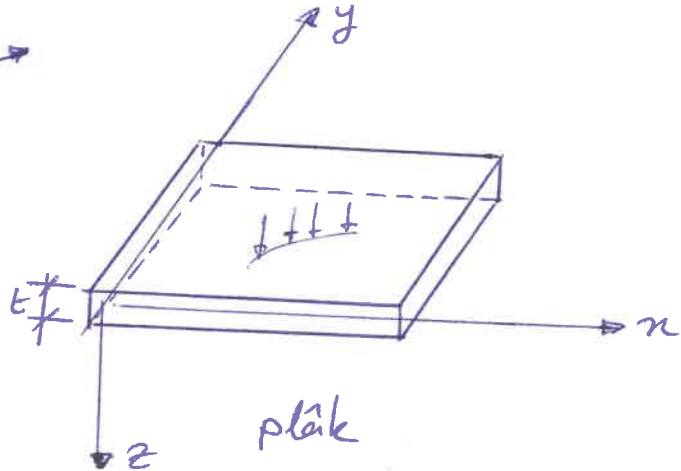
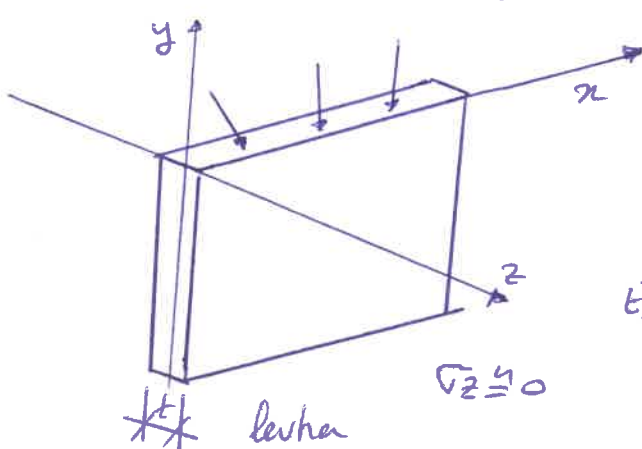


Şekil 1.5.

1.3) İki Eksenli Gerilme Hali (Düzlem Gerilme Hali)

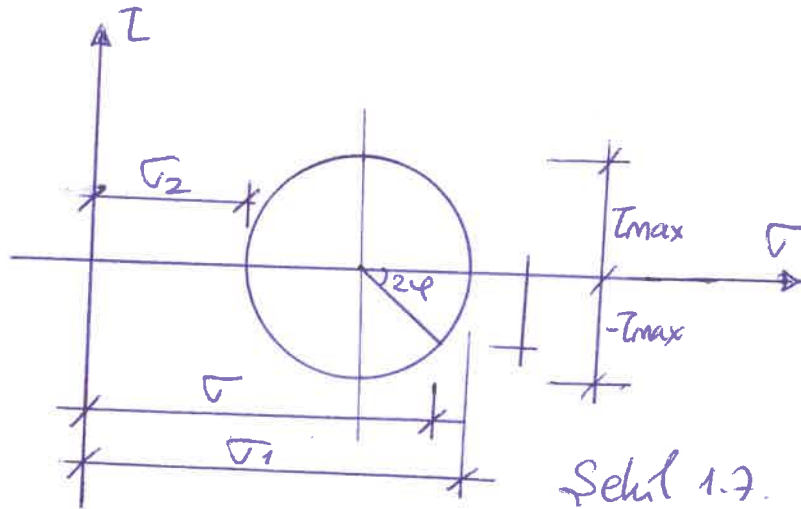
Bir elemanın eksenli kesitindeki gerilme vektörlerinin doğrultuları aynı düzlem içinde ise buna 'iki eksenli gerilme hali' veya 'düzlem gerilme hali' denir. Plak veya levha gibi elemanlar bu gerilme hali ile ifade edilebilir. Düzlem gerilme halinde eleman kalınlığı boyunca gerilmeler sıfırdır (Şekil 1.6) ($\sigma_z = 0$). Buna bağlı olarak kayma gerilmeleri de;

$\tau_{zx} = \tau_{xz} = 0$, $\tau_{zy} = \tau_{yz} = 0$ 'dır. Bu durumda gerilme tenzörü σ_{ij} bağımsız olmak üzere 4 skaler büyüklükten oluşacaktır.



Şekil 1.6.

3D' ekserli gerilme hali için mohr çemberi Şekil 1.7'de verilmiştir.



$$[T_g] = \begin{bmatrix} \sigma_x & \tau_{xy} & 0 \\ \tau_{yx} & \sigma_y & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$|\tau_{xy}| = |\tau_{yx}|$$

Şekil 1.7.

Asıl gerilme değerleri ise;

$$\sigma_{\max/\min} = \sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{\max/\min} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} = \frac{\sigma_1 - \sigma_2}{2} \text{ şeklinde dir.}$$

1.4.) Üç Ekserli Gerilme Hali

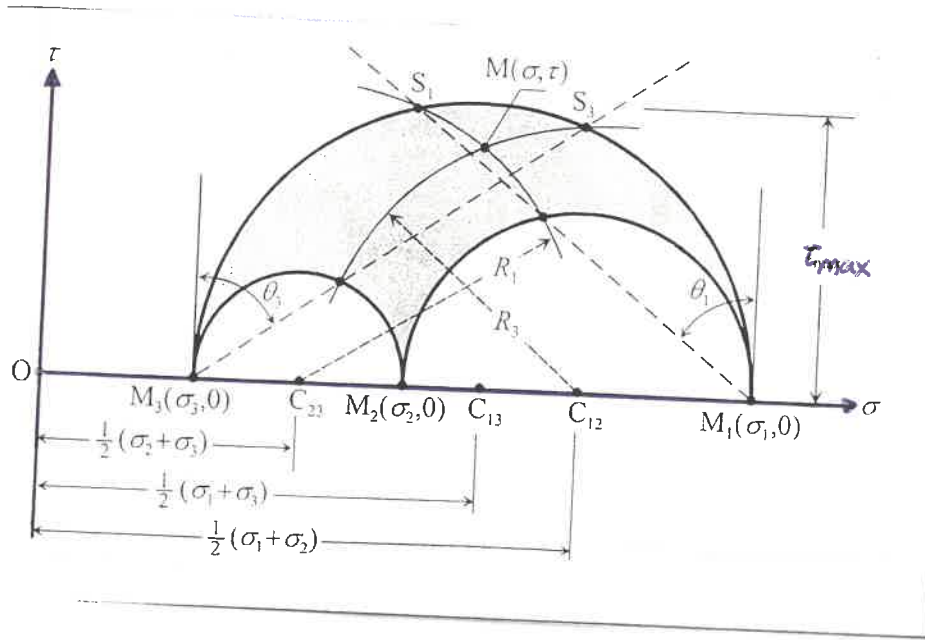
Bir elemanın ayıkma kesitindeki gerilme vektörlerinin doğrultuları farklı ise buna üç ekserli gerilme hali denir. Gerilme tenörü altı bağımsız gerilme bileşeni olarak üçer toplam 9 skaler büyüklükten oluşmaktadır.

$$[T_g] = [\sigma] = \begin{bmatrix} \sigma_x & \tau_{xy} & \tau_{xz} \\ \tau_{yx} & \sigma_y & \tau_{yz} \\ \tau_{zx} & \tau_{zy} & \sigma_z \end{bmatrix}$$

Burada $|\tau_{xy}| = |\tau_{yx}|$, $|\tau_{xz}| = |\tau_{zx}|$ ve

$|\tau_{yz}| = |\tau_{zy}|$ 'dir.

Öğ elverişli' gerilme hali için mohr çemberi Şekil 1.8'de verildiği gibidir.



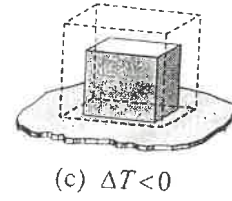
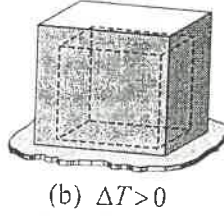
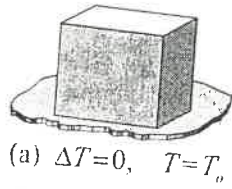
Şekil 1.8.



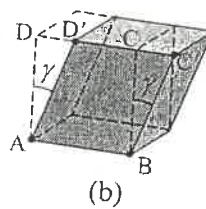
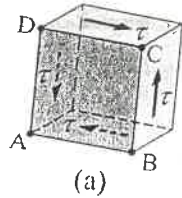
2) ŞEKİL DEĞİŞTİRME HALİ

2.1) **Giriş** : Şekil değiştirme, bir cismin veya bir elemanın noktalarının birbirlerine göre konumlarının veya rölatif durumunun değişmesidir.

Cismin genel olarak geometrik tipinin değişmeden kenarlarının uzaması veya ksalması suretiyle yalnızca kenarlarının değişmesi (Şekil 2.1) veya kenarlarının ağısal olarak değişmesi (Şekil 2.2) şekil değiştirme dediğimiz.



(Şekil 2.1)



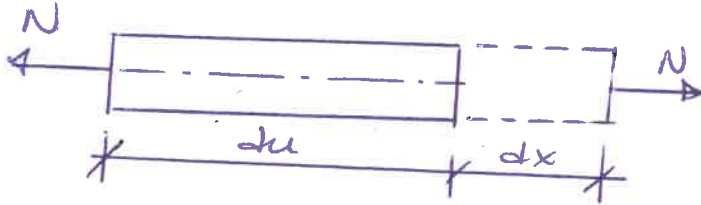
Şekil 2.2

Sözünü ettiğimiz bu şekil değiştirmeler dış yükler, ısı değişimi ve yapı malzemesinin visko-elastik özelliğinden (sınma-rötme) dolayı meydana gelmektedir. Bu şekil değiştirme boy değişimi ve açı değişimi şeklindedir ve şekil değişimi sonucunda cismin şekli ve boyutları değişmektedir.

Cismin şekil değişimine hali de tipki gerilme halinde ki gibi 1, 2 ve 3 eksenli şekil değişimine hali şeklinde de incelemekteyiz.

2.2) Bir Eksenli Şekil Değişimine Hali

Bir eksenli sistemlerde şekil değişimine uyan şekil değişimlidir (Şekil 2.3).



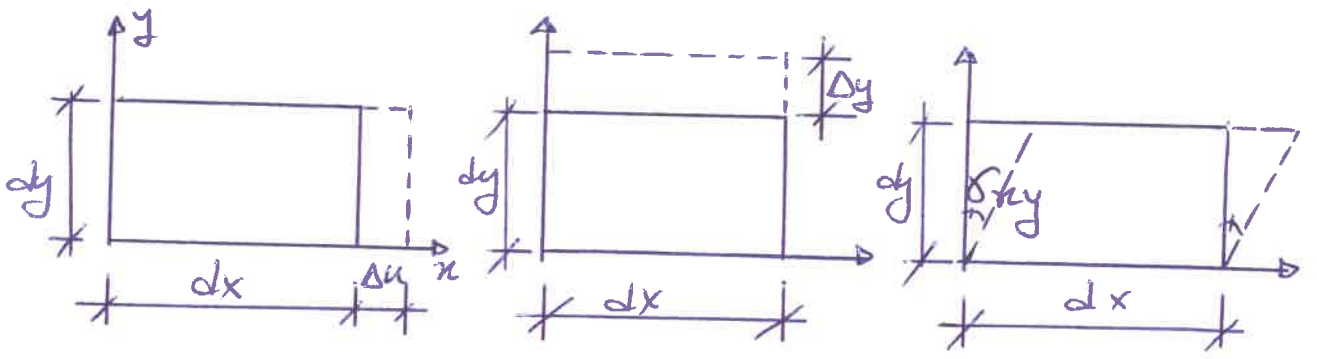
$$\sigma = \frac{N}{A} \quad \epsilon = \frac{\sigma}{E} = \frac{\Delta u}{u}$$

Şekil 2.3

Böyle bir sistemin şekil değişimine tensörü $[\epsilon] = \begin{bmatrix} \epsilon_x & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$ şeklindedir.

2.3) İki Eksenli Şekil Değişimine Hali (Düzlem Şekil Değişimine Hali)

Şekil değiştikmiş bir cismin yer değişimine vektörleri düzlemde kalıyorsa ve diğer 2 doğrultusundan etkilenmiyorsa bu şekil değişimine haline düzlem şekil değişimine hali denir. Tipki bir ısıtılma duvarının veya betonarme perde elemanının uyan doğrultusu gibidir. Bu durumda 2 doğrultusundaki şekil değişimleri sıfır olacaktır ($\epsilon_z = 0, \gamma_{xz} = \gamma_{yz} = 0$). Düzlemsel şekil değişimleri Şekil 2.3 'de verildiği gibi olacaktır.



Şekil 2.3

Düzlem şekil değişimine halinde, şekil değişimine tanjörü 4 skaler büyüklükten oluşmaktadır;

$$[\epsilon] = \begin{bmatrix} \epsilon_x & \gamma_{xy} \\ \gamma_{yx} & \epsilon_y \end{bmatrix}$$

Burada $\gamma_{xy} = \gamma_{yx}$ 'dır. Asal doğrultudaki maksimum şekil değişimleri;

$$\epsilon_{\max/\min} = \frac{\epsilon_x + \epsilon_y}{2} \pm \sqrt{\left(\frac{\epsilon_x - \epsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2}$$

2.4) Üç Eksenli Şekil Değişikme Hali

Prizmatik veya üç eksenli bir cismin şekil değişiminde, şekil değişikme tanjörü 9 adetlik.

$$[\epsilon] = \begin{bmatrix} \epsilon_x & \gamma_{xy} & \gamma_{xz} \\ \gamma_{yx} & \epsilon_y & \gamma_{yz} \\ \gamma_{zx} & \gamma_{zy} & \epsilon_z \end{bmatrix}$$

Burada $\gamma_{xy} = \gamma_{yx}$, $\gamma_{zx} = \gamma_{xz}$ ve $\gamma_{yz} = \gamma_{zy}$ 'dır.

$$\delta_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} + \left[\frac{\partial u}{\partial x} \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \frac{\partial v}{\partial y} + \frac{\partial w}{\partial x} \frac{\partial w}{\partial y} \right]$$

$$\delta_{xz} = \frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} + \left[\frac{\partial u}{\partial x} \frac{\partial u}{\partial z} + \frac{\partial v}{\partial x} \frac{\partial v}{\partial z} + \frac{\partial w}{\partial x} \frac{\partial w}{\partial z} \right]$$

$$\delta_{yz} = \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} + \left[\frac{\partial u}{\partial y} \frac{\partial u}{\partial z} + \frac{\partial v}{\partial y} \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \frac{\partial w}{\partial z} \right]$$

Bağıntılarda ilk terimler lineer durumu, parantez içi terimler ise lineer olmayan (non-linear) durumu göstermektedir. Parantez içinden ilk iki terim son terim yanında ihmal edilebilecek kadar küçük olduğundan analizlerde;

$$\varepsilon_x = \frac{\partial u}{\partial x} + \frac{1}{2} \left(\frac{\partial w}{\partial x} \right)^2$$

$$\varepsilon_y = \frac{\partial v}{\partial y} + \frac{1}{2} \left(\frac{\partial w}{\partial y} \right)^2$$

$$\varepsilon_z = \frac{\partial w}{\partial z} + \frac{1}{2} \left(\frac{\partial w}{\partial z} \right)^2$$

$$\delta_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} + \left[\frac{\partial w}{\partial x} \frac{\partial w}{\partial y} \right]$$

$$\delta_{xz} = \frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} + \left[\frac{\partial w}{\partial x} \frac{\partial w}{\partial z} \right]$$

$$\delta_{yz} = \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \left[\frac{\partial w}{\partial y} + \frac{\partial w}{\partial z} \right]$$

bağıntılarda verilen kısımları almak yeterlidir.

Matris formda yazılabilir;

$$\{\varepsilon\} = \underbrace{\{\varepsilon_0\}}_{\text{linear}} + \underbrace{\{\varepsilon_n\}}_{\text{non-linear}}$$

$$\{\varepsilon\} = \begin{Bmatrix} \varepsilon_{0,m} \\ \varepsilon_{0,e} \end{Bmatrix} + \begin{Bmatrix} \varepsilon_{n,m} \\ \varepsilon_{n,e} \end{Bmatrix} \quad \left| \begin{array}{l} \varepsilon_{0,m} = \text{Linear membran terim} \\ \varepsilon_{0,e} = \text{" eğilme terim} \\ \varepsilon_{n,m} = \text{non-linear membran ter} \\ \varepsilon_{n,e} = \text{" eğilme ter.} \end{array} \right.$$

Düzlem durumda;

$$\{\varepsilon\} = \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \\ k_x \\ k_y \\ k_{xy} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u}{\partial x} \\ \frac{\partial v}{\partial y} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \\ -\frac{\partial^2 w}{\partial x^2} \\ -\frac{\partial^2 w}{\partial y^2} \\ -2\frac{\partial^2 w}{\partial x \partial y} \end{Bmatrix} + \begin{Bmatrix} \frac{1}{2}\left(\frac{\partial w}{\partial x}\right)^2 \\ \frac{1}{2}\left(\frac{\partial w}{\partial y}\right)^2 \\ \frac{\partial w}{\partial x} \frac{\partial w}{\partial y} \\ 0 \\ 0 \\ 0 \end{Bmatrix} = \begin{Bmatrix} \varepsilon_{0,m} \\ \varepsilon_{0,e} \end{Bmatrix} + \begin{Bmatrix} \varepsilon_{n,m} \\ 0 \end{Bmatrix}$$

$$\{\varepsilon_m\} = \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u}{\partial x} + \frac{1}{2}\left(\frac{\partial w}{\partial x}\right)^2 \\ \frac{\partial v}{\partial y} + \frac{1}{2}\left(\frac{\partial w}{\partial y}\right)^2 \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} + \frac{\partial w}{\partial x} \frac{\partial w}{\partial y} \end{Bmatrix}$$

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4) MALZEMELERİN MEKANİK ÖZELLİKLERİ VE GERİLME ŞEKİL DEĞİŞTİRME BAĞINTILARI

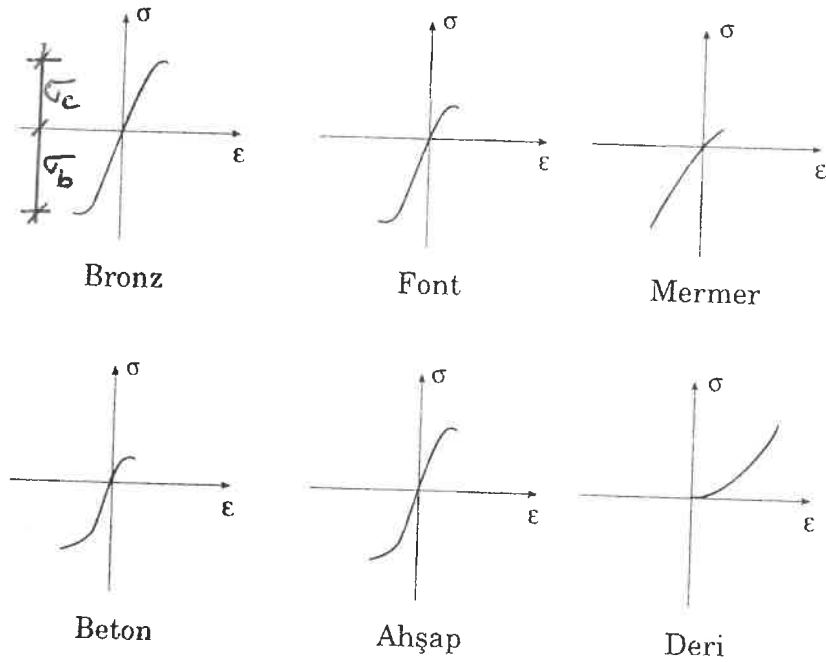
4.1) GİRİŞ : İnşaat mühendisliğinde kullanılan malzemelerin, dış etkilere altındaki şekil değişimlerinin, kırılmasının veya çatlama özelliklerinin incelenmesi ve tespiti malzemenin mekanik özelliklerinin en önemli konusudur. Bu mekanik özelliklerin tespitinde statik ve dinamik deneysel çalışmalar, sürekli ortamlar mekanizmasından yola çıkılarak yapılan araştırmalar ve malzemenin makroskopik yapısı ile atom kavramından yola çıkan çalışmalar rol oynamaktadır. Bu çalışmaların sonucunda elemanın malzemesi ile bunun dış etkilere altında tepki ve davranışlarını gösteren yarı gerilme ile şekil değişime bileşenleri arasındaki ilişkiyi veren elastisite bağıntıları, bir başka deyişle bükme denklemleri bulunmaktadır.

Mühendislikte kullanılan malzemeler;

- a) Metaller ve alaşımlar b) Polimerler
- c) Seramik ve camlar 4) Kompozitler 'dir.

Bunun dışında ayırica malzemeler kırılmaya veya son limit durumuna gelinceye kadar yaptıkları şekil değişime davranışları göz önüne alınarak sünek (ductil), gevrek ve plastik olarak üç grupta toplanabilirler. Sünek cisimler, kopmadan veya son limit durumuna gelmeden önce büyük şekil değişime gösteren cisimlerdir. Buna en iyi örnek çeliklerdir. Gevrek cisimler, akma sınırları olmayan, kopmadan önce büyük uzama göstermeyen cisimlerdir. Buna örnek olarak beton, tahta ve font'un

gösterebiliriz. Plastik cisimler ise elastik özelliği az olan asfalt, ahşap ve deri gibi malzemelerin oluşturduğu cisimlerdir. Aşağıdaki şekilde bazı malzemelerin gerilme-şekil değiştirme eğrileri verilmiştir (Şekil 4.1)



Şekil 4.1.

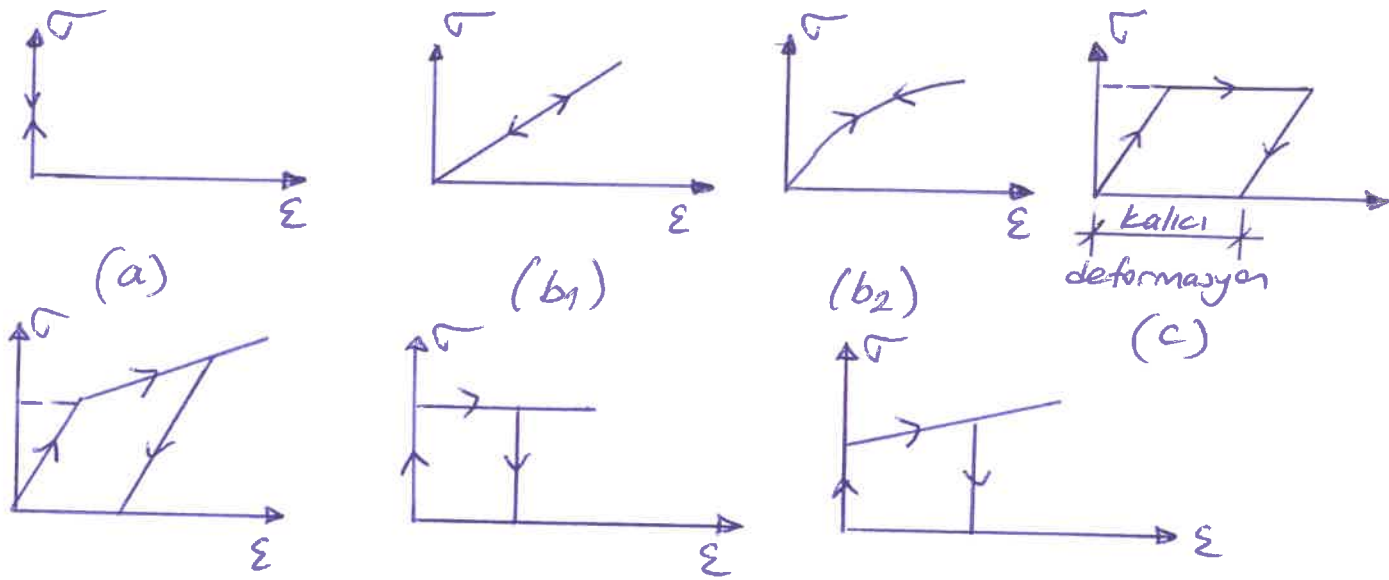
4.2) Gerilme-Şekil Değiştirme Diyagramlarının İdealleştirilmesi (İdeal Cisimler)

Cisimlerin mekanik özellikleri çok karışık ve birbirlerinden çok farklıdır. Herhangi bir mühendislik probleminde bunların tamamının birden göz önünde bulundurulması mümkün değildir. Bütün bilgisi kadar gereğe uygun düşecek bir teori kurulmak isteniyorsa, problemde en esaslı rolü oynayan cismin özellikleri dikkate alınmalı, etkileri az olan faktörler ihmal edilmelidir. Bu nedenle şekil değiştiren cisimlerin mekaniklerinde konuya uygun düşen bir takım ideal cisimler kabul edilir ve bütün

teori' buna göre incelenir.

Cisimlerin elastik ve plastik özellikleri göz önüne alınarak tek eksenli gerilme halinde yapılan idealleştirilmelerden bazıları;

- Rijit cisim,
- Elastik Hooke Cisim,
- Elasto Plastik Cisim
- Pekilesen Elasto Plastik Cisim,
- Rijit Plastik Cisim
- Pekilesen Rijit Plastik Cisim (Şekil 4.2)



Şekil 4.2.

4.3) Hooke Kanunu (Beyne Bağlantıları)

Bir noktanın yakın çevresinde oluşan gerilme ve şekil değişimleri bileşenlerini ilişkilendirirken Hooke kanunları (Hooke Yasaları) denir. Tek eksenli gerilme halinde Hooke Yasası;

$$\epsilon_1 = \frac{\sigma_1}{E}$$

şeklinde dir. Tek eksenli gerilme halinde cisim boyu gerilme vektörü doğrultusunda artarken buna dik doğrultuda kısalır veya uzar. Bu durumda;

$$\epsilon_2 = \epsilon_3 = -\nu \epsilon_1 = -\nu \frac{\sigma_1}{E} \text{ olur. Buradaki Poisson}$$

oranı boyuna ve enine selül değişimine oranı olup $(\nu = -\epsilon_b/\epsilon_e)$ $0 \leq \nu \leq \frac{1}{2}$ arasındadır.

Bağıt kayma etkisi altındaki bir cisimde açı değişimi ile kayma gerilmeleri arasında doğrusal bir bağıntı bulunmaktadır. Bu da $\gamma = \frac{\tau}{G}$ ile tanımlanmaktadır.

4.4) Genel Hooke Kanunu

En genel durumda (x, y, z) takımında bir noktadaki gerilme tensörü $\sigma = \sigma(\sigma_x, \sigma_y, \sigma_z, \tau_{xy}, \tau_{xz}, \tau_{yz})$ ile selül değişimine tensörü $\epsilon = \epsilon(\epsilon_x, \epsilon_y, \epsilon_z, \gamma_{xy}, \gamma_{xz}, \gamma_{yz})$ 'nın bileşenleri arasındaki ilişkileri kuran birtane bağıntıyı superpozisyon kuralı ile elde edilir. Yalnız normal ve kayma gerilmelerinin etkileri birbirlerinden bağımsızdır. Başka bir deyişle 1. mertebe terime göre ϵ elastik gerilme halinde normal gerilmeler açı değişimi, kayma gerilmeleri de boy değişimi yapmayacaklarından gerilme selül değişimine bağıntıları iki ayrı grup halinde incelenebilir. Sonuç olarak normal gerilmelerin doğrudan doğruya uzama oranları;

$$\epsilon_x = \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} - \nu \frac{\sigma_z}{E} = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)]$$

$$\epsilon_y = \frac{\sigma_y}{E} - \nu \frac{\sigma_x}{E} - \nu \frac{\sigma_z}{E} = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)]$$

$$\epsilon_z = \frac{\sigma_z}{E} - \nu \frac{\sigma_y}{E} - \nu \frac{\sigma_x}{E} = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)]$$

Kayma gerilmelerinin sebep olduğu kayma asıları;

$$\gamma_{xy} = \frac{\tau_{xy}}{G} ; \quad \gamma_{zx} = \frac{\tau_{zx}}{G} ; \quad \gamma_{yz} = \frac{\tau_{yz}}{G}$$

olur.

Gerilmeleri şekil değişkenleri cinsinden yazarsak;

$$\sigma_x = \frac{E\nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y + \epsilon_z) + 2G\epsilon_x$$

$$\sigma_y = \frac{E\nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y + \epsilon_z) + 2G\epsilon_y$$

$$\sigma_z = \frac{E\nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y + \epsilon_z) + 2G\epsilon_z$$

$$\tau_{xy} = G \cdot \gamma_{xy} ; \tau_{xz} = G \cdot \gamma_{xz} ; \tau_{yz} = G \cdot \gamma_{yz}$$

olacaktır. Burada verilen G kayma modülüdır ve homojen ve izotrop malzemelerde $G = \frac{E}{2(1+\nu)}$ şeklindedir. Diğer yandan gerilme bağıntısındaki;

$$\frac{E\nu}{(1+\nu)(1-2\nu)} = \lambda \text{ olarak tanımlanır. } G'yi de$$

genel durum için μ şeklinde yazarsak her iki terime (μ, λ) lame sabiti adı verilir. Şekil değişkenlerinin gerilmeler cinsinden yazıldığı bağıntıyı matris formunda yazarsak;

$$\begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{bmatrix} = \begin{bmatrix} 1/E & -\nu/E & -\nu/E & 0 & 0 & 0 \\ -\nu/E & 1/E & -\nu/E & 0 & 0 & 0 \\ -\nu/E & -\nu/E & 1/E & 0 & 0 & 0 \\ 0 & 0 & 0 & 1/G & 0 & 0 \\ 0 & 0 & 0 & 0 & 1/G & 0 \\ 0 & 0 & 0 & 0 & 0 & 1/G \end{bmatrix} \begin{bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{bmatrix}$$

Geometrik ve fiziksel birimlerde yazılımı ise

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{Bmatrix} = \begin{bmatrix} (\lambda + 2\mu) & \lambda & \lambda & 0 & 0 & 0 \\ \lambda & (\lambda + 2\mu) & \lambda & 0 & 0 & 0 \\ \lambda & \lambda & (\lambda + 2\mu) & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix}$$

şeklinde elde edilir. Her iki durumun keşafı şeklinde yazılımı ;

$$\{\epsilon\} = [H] \{\sigma\}$$

$$\{\sigma\} = [D] \{\epsilon\} \text{ 'dur.}$$

matrisleri yeniden yazarsak;

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} = \begin{bmatrix} H_{11} & H_{12} & H_{13} & 0 & 0 & 0 \\ H_{21} & H_{22} & H_{23} & 0 & 0 & 0 \\ H_{31} & H_{32} & H_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & H_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & H_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & H_{66} \end{bmatrix} \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{Bmatrix}$$

Burada ; $H_{11} = 1/E$; $H_{12} = -\nu/E$; $H_{13} = -\nu/E$

şeklinde dir.

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{bmatrix} = \begin{bmatrix} D_{11} & D_{12} & D_{13} & 0 & 0 & 0 \\ D_{21} & D_{22} & D_{23} & 0 & 0 & 0 \\ D_{31} & D_{32} & D_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & D_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & D_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & D_{66} \end{bmatrix} \begin{bmatrix} \varepsilon_x \\ \varepsilon_y \\ \varepsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{bmatrix}$$

Burada $D_{11} = 2 + 2\mu$; $D_{12} = 2$; $D_{13} = 2$ alınmıştır.

4

5. ELASTİSİTE SABİTLERİ

5.1) İZOTROP DURUM

Malzemedeki elastik özellikler koordinat takımından bağımsızdır. Bu malzemeye izotrop, aksi halde anizotrop denir.

5.1.1) Bir Boyutlu Gerilme ve Lineer Elastik İzotrop Durum

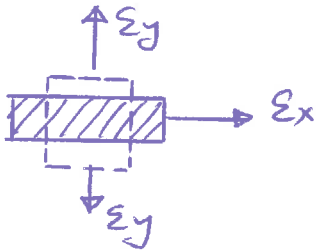


Gerilme

$$\sigma_x = E \cdot \epsilon_x$$

$$\sigma_y = 0$$

$$\tau_{xy} = 0$$



Sehit
Degisiklik

$$\epsilon_x = \frac{\sigma_x}{E}$$

$$\epsilon_y = -\nu \cdot \epsilon_x$$

$$\gamma_{xy} = 0$$

$$\tau_{max} = \frac{\sigma_x}{2}$$

$$\gamma_{max} = \frac{\tau_{max}}{G}$$

Tanımlar

E : Elastisite Modülü

ν : Poisson Oranı

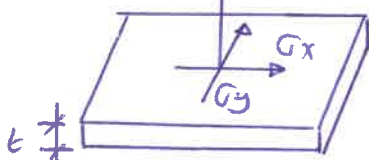
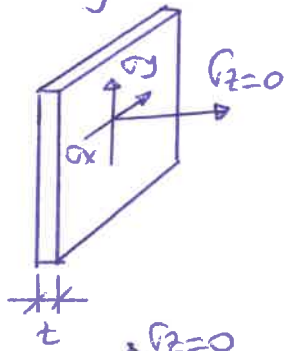
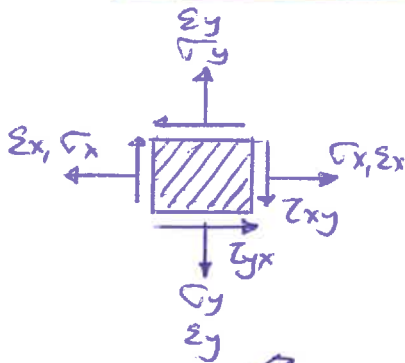
G : Kayma Modülü

5.1.2) Bir Boyutlu Sıkışma Değiştirme ve Lineer Elastik İzotrop Durum

Gerilme	$\left\{ \begin{aligned} \sigma_x &= \frac{E \cdot \nu}{(1+\nu)(1-2\nu)} \epsilon_x \\ \sigma_y = \sigma_z &= \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \epsilon_x \\ \tau_{xy} &= 0 \end{aligned} \right.$	Sıkışma Değiştirme	$\left\{ \begin{aligned} \epsilon_x &= \frac{\sigma_x}{E} \\ \epsilon_y &= 0 \\ \gamma_{xy} &= 0 \end{aligned} \right.$

Burada verilen $\frac{E \cdot \nu}{(1+\nu)(1-2\nu)} = \lambda$ olarak tanımlarsak
Lame sabitidir.

5.1.3) İki Boyutlu Gerilme ve Lineer Elastik İzotrop Durum (Düzlem Gerilme Hali)



Gerilmeler

$$\left\{ \begin{aligned} \sigma_z &= 0 \\ \epsilon_z &\neq 0 \end{aligned} \right.$$

$$\begin{aligned} \sigma_x &= \frac{E \nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y) + 2G \epsilon_x \\ &= \frac{E}{1-\nu^2} (\epsilon_x + \nu \epsilon_y) \end{aligned}$$

$$\begin{aligned} \sigma_y &= \frac{E \nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y) + 2G \epsilon_y \\ &= \frac{E}{1-\nu^2} (\nu \epsilon_x + \epsilon_y) \end{aligned}$$

$$\tau_{xy} = G \gamma_{xy} = \frac{E}{2(1+\nu)} \gamma_{xy}$$

Matris formunda $\{\sigma\} = [D] \{\epsilon\}$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

Şekil
Değişiklikler

$$\epsilon_z = -\frac{\nu}{(1-\nu)} (\epsilon_x + \epsilon_y)$$

$$\epsilon_x = \frac{1}{E} (\sigma_x - \nu \sigma_y)$$

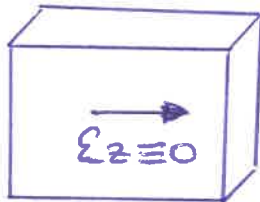
$$\epsilon_y = \frac{1}{E} (\sigma_y - \nu \sigma_x)$$

$$\gamma_{xy} = \frac{\tau_{xy}}{G} = \frac{2(1+\nu)}{E} \tau_{xy}$$

Bağıntılar matris formunda yazılırsa; $\{\epsilon\} = [H] \{\sigma\}$

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -\nu & 0 \\ -\nu & 1 & 0 \\ 0 & 0 & 2(1+\nu) \end{bmatrix} \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}$$

5.1.4) 3 Boyutlu Şekil Değişikliği ve Lineer Elastik İzotrop Durum (Düzlem Şekil Değişikliği Hali)



$$\sigma_z \neq 0$$

$$\sigma_z = \nu(\sigma_x + \sigma_y)$$

$$\epsilon_x = \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} - \nu \frac{\sigma_z}{E}$$

$$\epsilon_y = -\nu \frac{\sigma_x}{E} + \frac{\sigma_y}{E} - \nu \frac{\sigma_z}{E}$$

$$\epsilon_z = -\nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} + \frac{\sigma_z}{E} = 0$$

$$\epsilon_x = \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} - \frac{\nu^2}{E} (\sigma_x + \sigma_y)$$

$$\epsilon_y = -\nu \frac{\sigma_x}{E} + \frac{\sigma_y}{E} - \frac{\nu^2}{E} (\sigma_x + \sigma_y)$$

İfadeleri ekte edilerek ki buradan gerilme ve yer değişikliği bağıntıları aşağıda verildiği şekilde bulunacaktır.

$$\text{Gerçekleşen} \left\{ \begin{aligned} \sigma_x &= \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_x + \nu\epsilon_y] \\ \sigma_y &= \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_y + \nu\epsilon_x] \\ \tau_{xy} &= \frac{E}{2(1+\nu)} \gamma_{xy} = G \gamma_{xy} \end{aligned} \right.$$

Matris formunda yazılabilir;

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

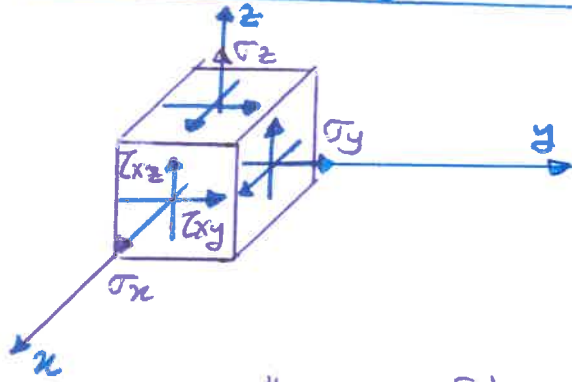
$$\text{Seri 1 Değişkenler} \left\{ \begin{aligned} \epsilon_x &= \frac{(1+\nu)}{E} [(1-\nu)\sigma_x - \nu\sigma_y] \\ \epsilon_y &= \frac{(1+\nu)}{E} [(1-\nu)\sigma_y - \nu\sigma_x] \\ \epsilon_z &= 0 \\ \gamma_{xy} &= \frac{2(1+\nu)}{E} \tau_{xy} = \frac{\tau_{xy}}{G} \end{aligned} \right.$$

Matris formunda yazılabilir;

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \frac{(1+\nu)}{E} \begin{bmatrix} 1-\nu & -\nu & 0 \\ -\nu & 1-\nu & 0 \\ 0 & 0 & 2 \end{bmatrix} \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}$$

bağıntılar elde edilir.

5.1.5) Üç Boyutlu Lineer Elastik İzotrop Durum



Gerilmeler

$$\begin{aligned}\sigma_x &= \frac{E\nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y + \epsilon_z) + 2G\epsilon_x = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_x + \nu(\epsilon_y + \epsilon_z)] \\ \sigma_y &= \frac{E\nu}{(1+\nu)(1-2\nu)} (\epsilon_x + \epsilon_y + \epsilon_z) + 2G\epsilon_y = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_y + \nu(\epsilon_x + \epsilon_z)] \\ \tau_{xy} &= G\gamma_{xy} = \frac{E}{2(1+\nu)} \gamma_{xy} \\ \tau_{xz} &= G\gamma_{xz} = \frac{E}{2(1+\nu)} \gamma_{xz} \\ \tau_{yz} &= G\gamma_{yz} = \frac{E}{2(1+\nu)} \gamma_{yz}\end{aligned}$$

Bağıntılar matris formunda yazılırsa; $\{\sigma\} = [D]\{\epsilon\}$ olur.

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{Bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} (1-\nu) & \nu & \nu & 0 & 0 & 0 \\ \nu & (1-\nu) & \nu & 0 & 0 & 0 \\ \nu & \nu & (1-\nu) & 0 & 0 & 0 \\ 0 & 0 & 0 & \frac{1-2\nu}{2} & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{(1-2\nu)}{2} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix}$$

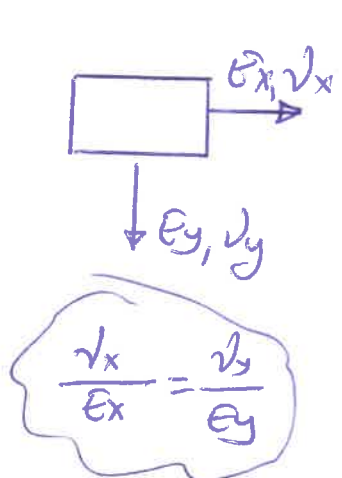
$$\left. \begin{aligned} \lambda &= \frac{E\nu}{(1+\nu)(1-2\nu)} \\ \mu &= G = \frac{E}{2(1+\nu)} \end{aligned} \right\} \text{de } [D] \text{ matrisi} \rightarrow [D] = \begin{bmatrix} (\lambda+2\mu) & \lambda & \lambda & 0 & 0 & 0 \\ \lambda & (\lambda+2\mu) & \lambda & 0 & 0 & 0 \\ \lambda & \lambda & (\lambda+2\mu) & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix}$$

$$\begin{aligned} \epsilon_x &= \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)] \\ \epsilon_y &= \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)] \\ \epsilon_z &= \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)] \\ \gamma_{xy} &= \frac{\tau_{xy}}{G} \quad ; \quad \gamma_{xz} = \frac{\tau_{xz}}{G} \quad ; \quad \gamma_{yz} = \frac{\tau_{yz}}{G} \end{aligned}$$

5.2) ORTOTROP DURUM

Birbirine dik Π_i düzlemlere göre malzeme simetresi olan cisimlerde. Birbirine dik Π_i düzlemlere göre simetresi olmayan durumlarda bu Π_i düzlemlere dik üçüncü düzlemde simetresi kendiliğinden sağlanır. Dolayısıyla birbirine dik 3 düzleme göre simetresi olan malzemelerde bağımsız malzeme sabitlerinin sayısı 9'dur. Bu malzemelere ortogonallik anizotropik kelimelerin kısaltılması olan ortotropik malzeme adı verilir. Örneğin; ahşap, lifli kompozit malzeme.

5.2.1) Π_i Boyutlu Lineer Elastik Ortotrop Durum (Düzlem Gerilme Hali)



$$\begin{aligned} \sigma_x &= \frac{E_x}{1-\nu_x\nu_y} (\epsilon_x + \nu_y\epsilon_y) \\ \sigma_y &= \frac{E_y}{1-\nu_x\nu_y} (\epsilon_y + \nu_x\epsilon_x) \\ \tau_{xy} &= \frac{E_x E_y}{E_x(1+\nu_y) + E_y(1+\nu_x)} \gamma_{xy} = G \gamma_{xy} \end{aligned}$$

Bu bağlamda $G = \frac{E_x E_y}{E_x(1+\nu_y) + E_y(1+\nu_x)}$ şeklinde.

matris formda yazılrsa;

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{1}{1-\nu_x \nu_y} \begin{bmatrix} E_x & E_x \nu_y & 0 \\ E_y \nu_x & E_y & 0 \\ 0 & 0 & G(1-\nu_x \nu_y) \end{bmatrix} \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

şeklinde dir.

Şerit Değiştirme

$$\begin{aligned} \varepsilon_x &= \frac{\sigma_x}{E_x} - \nu_y \frac{\sigma_y}{E_y} \\ \varepsilon_y &= -\nu_x \frac{\sigma_x}{E_x} + \frac{\sigma_y}{E_y} \\ \gamma_{xy} &= \frac{[\sigma_x(1+\nu_y) + \sigma_y(1+\nu_x)]}{E_x E_y} \tau_{xy} = \frac{\tau_{xy}}{G} \end{aligned}$$

5.22) 3D'li Bağıtli Lineer Elastik Ortotrop Durum (Düzlem Şerit Değiştirme Hali)

Gerilmeler

$$\sigma_x = \frac{E_x}{(1-\nu_z \nu_y)(1-\nu_x \nu_z) - \nu_x \nu_y (1+\nu_z)^2} [(1-\nu_z \nu_y) \varepsilon_x + \nu_y (1+\nu_z) \varepsilon_y]$$

$$\sigma_y = \frac{E_y}{(1-\nu_z \nu_y)(1-\nu_x \nu_z) - \nu_x \nu_y (1+\nu_z)^2} [(1+\nu_z) \nu_x \varepsilon_x + (1-\nu_x \nu_z) \varepsilon_y]$$

$$\sigma_z = \frac{E_z}{E_x} \nu_x \sigma_x + \frac{E_z}{E_y} \nu_y \sigma_y$$

$$\tau_{xy} = [(1-\nu_z \nu_y)(1-\nu_x \nu_z) - \nu_x \nu_y (1+\nu_z)^2] G \gamma_{xy}$$

Yer

Değiştirme

$$\varepsilon_z \equiv 0$$

$$\varepsilon_x = \frac{(1-\nu_x \nu_z)}{E_x} \sigma_x - \frac{\nu_y (1+\nu_z)}{E_y} \sigma_y$$

$$\varepsilon_y = -\frac{\nu_x (1+\nu_z)}{E_x} \sigma_x + \frac{(1-\nu_z \nu_y)}{E_y} \sigma_y$$

$$\gamma_{xy} = \frac{\tau_{xy}}{[(1-\nu_z \nu_y)(1-\nu_x \nu_z) - \nu_x \nu_y (1+\nu_z)^2] \cdot \frac{1}{G}}$$

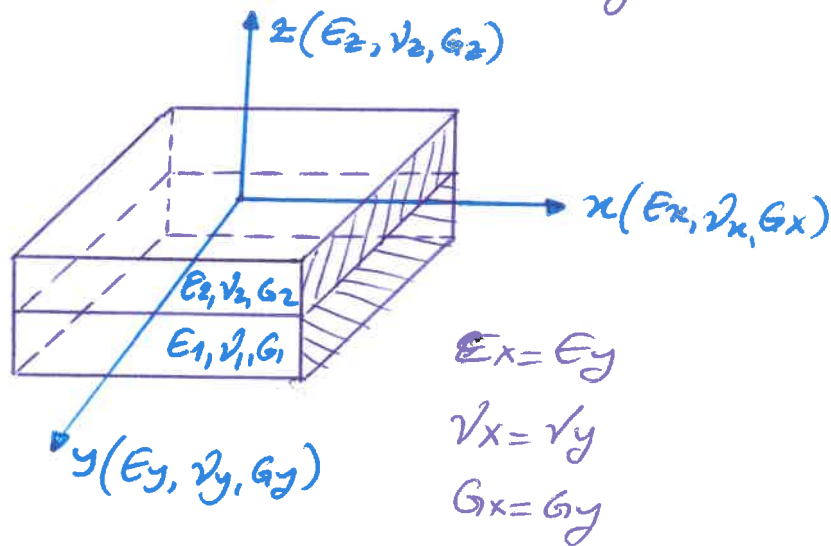
matris formunda yazılırsa,

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{1}{(1-\nu_z\nu_y)(1-\nu_x\nu_z)-\nu_x\nu_y(1+\nu_z)^2} \begin{bmatrix} E_x(1-\nu_z\nu_y) & E_x\nu_y(1+\nu_z) & 0 \\ E_y\nu_x(1+\nu_z) & E_y(1-\nu_x\nu_z) & 0 \\ 0 & 0 & AG \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

$$A = (1-\nu_z\nu_y)(1-\nu_x\nu_z)-\nu_x\nu_y(1+\nu_z)^2$$

5.2.3) Üç Boyutlu Tetragonal Simetriye Sahip Ortotrop Malzeme Durumu

Ortotrop bir cisimde, malzemenin elastik özellikleri, uygun olarak seçilen bir eksen takımının T_{11} ' eksenine doğrultusunda aynı, üçüncü eksen doğrultusunda farklıdır. Yani elastik özellikler birbirine dik üç düzleme göre T_{11} 'er T_{11} 'er simetrikler. Burada buna ek olarak birde T_{11} ' eksenin üçüncü eksen etrafında dönmesi halinde elastik özelliklerin değişmemesi söz konusudur. Kısaca elastik özellikler eksenel simetrikler. Bu özel simetriden dolayı ortotrop malzemeye tetragonal simetrisi denir. Bu durumda bağımsız sabitlerin sayısı 6 olur.



Gesetze

$$\sigma_x = \frac{E_z}{(1+\nu_x)(1-\nu_x-2\frac{E_x}{E_z}\nu_z^2)} \left[(1-\nu_x^2)\epsilon_x + \frac{E_x}{E_z}\nu_z(1+\nu_x)\epsilon_y + \frac{E_x}{E_z}\nu_z(1+\nu_x)\epsilon_z \right]$$

$$\sigma_y = \frac{E_z}{(1+\nu_x)(1-\nu_x-2\frac{E_x}{E_z}\nu_z^2)} \left[\frac{E_x}{E_z}\nu_z(1+\nu_x)\epsilon_x + \frac{E_x}{E_z}\left(1-\frac{E_x}{E_z}\nu_z^2\right)\epsilon_y + \frac{E_x}{E_z}\left(\nu_x+\frac{E_x}{E_z}\nu_z^2\right)\epsilon_z \right]$$

$$\sigma_z = \frac{E_z}{(1+\nu_x)(1-\nu_x-2\frac{E_x}{E_z}\nu_z^2)} \left[\frac{E_x}{E_z}\nu_z(1+\nu_x)\epsilon_x + \frac{E_x}{E_z}\left(\nu_x+\frac{E_x}{E_z}\nu_z^2\right)\epsilon_y + \frac{E_x}{E_z}\left(1-\frac{E_x}{E_z}\nu_z^2\right)\epsilon_z \right]$$

$$\tau_{xy} = G_x \cdot \gamma_{xy} ; \tau_{xz} = G_z \cdot \gamma_{xz} ; \tau_{yz} = G_z \cdot \gamma_{yz}$$

matris formda yazılabilir:

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{bmatrix} = \frac{E_z}{(1+\nu_x)(1-\nu_x-2\frac{E_x}{E_z}\nu_z^2)} \begin{bmatrix} (1-\nu_x^2) & \frac{E_x}{E_z}\nu_z(1+\nu_x) & \frac{E_x}{E_z}\nu_z(1+\nu_x) & 0 & 0 & 0 \\ \frac{E_x}{E_z}\nu_z(1+\nu_x) & \frac{E_x}{E_z}(1-\frac{E_x}{E_z}\nu_z^2) & (\nu_x+\frac{E_x}{E_z}\nu_z^2)\frac{E_x}{E_z} & 0 & 0 & 0 \\ \frac{E_x}{E_z}\nu_z(1+\nu_x) & \frac{E_x}{E_z}(\nu_x+\frac{E_x}{E_z}\nu_z^2) & \frac{E_x}{E_z}(1-\frac{E_x}{E_z}\nu_z^2) & 0 & 0 & 0 \\ 0 & 0 & 0 & AG_x & 0 & 0 \\ 0 & 0 & 0 & 0 & AG_y & 0 \\ 0 & 0 & 0 & 0 & 0 & AG_z \end{bmatrix}$$

olur. Burada verilen $A = \frac{(1-\nu_x)(1-\nu_x-2\frac{E_x}{E_z}\nu_z^2)}{E_z}$ dir.

Seri? Değişkenler

$$\begin{aligned} \epsilon_x &= \frac{\sigma_x}{E_x} - \nu_x \frac{\sigma_y}{E_x} - \nu_z \frac{\sigma_z}{E_z} \\ \epsilon_y &= -\frac{\nu_x}{E_x} \sigma_x + \frac{\sigma_y}{E_x} - \nu_z \frac{\sigma_z}{E_z} \\ \epsilon_z &= -\frac{\nu_x}{E_x} \sigma_x - \frac{\sigma_y}{E_x} + \frac{\sigma_z}{E_z} \end{aligned}$$

$$\gamma_{xy} = \frac{\tau_{xy}}{G_x} ; \gamma_{xz} = \frac{\tau_{xz}}{G_z} ; \gamma_{yz} = \frac{\tau_{yz}}{G_z} \text{ dir}$$

5.3) Isı Değişimi Etkisi ve Elastikite Bağlılıkları

Cisimlerde ısı değişimi etkisi ile yalnızca boy değişimleri (ε) olur. Kayma şekil değişimleri (γ) meydana gelmez. Burada α_t; ısı gerilme katsayısı, Δt ise ısı değişimini göstermek üzere elastikite bağlantıları aşağıda verilmiştir.

5.3.1) Isı Değişimi ve Diğer Gerilme Hali

$$\left. \begin{aligned} \epsilon_x &= \frac{1}{E} (\sigma_x - \nu \sigma_y) + \alpha_t \cdot \Delta t = \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} + \alpha_t \cdot \Delta t \\ \epsilon_y &= \frac{1}{E} (-\nu \sigma_x + \sigma_y) + \alpha_t \cdot \Delta t = -\frac{\nu \sigma_x}{E} + \frac{\sigma_y}{E} + \alpha_t \cdot \Delta t \end{aligned} \right\} \begin{array}{l} \text{Diziydik} \\ + \\ \text{ısı etkisi} \end{array}$$

Yalnız ısı etkisi durumunda;

$$\left\{ \begin{array}{c} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{array} \right\} = \left\{ \begin{array}{c} \alpha_t \cdot \Delta t \\ \alpha_t \cdot \Delta t \\ 0 \end{array} \right\}$$

$$\{\sigma\} = [D] \{\epsilon\} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \left\{ \begin{array}{c} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{array} \right\} \quad \text{İdi.}$$

ısı etkisi durumunda;

$$\left\{ \begin{array}{c} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{array} \right\} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \left\{ \begin{array}{c} \epsilon_x = \alpha_t \cdot \Delta t \\ \epsilon_y = \alpha_t \cdot \Delta t \\ \gamma_{xy} = 0 \end{array} \right\}$$

Buradan;

$$\sigma_x = \frac{E}{1-\nu^2} [\alpha_t \cdot \Delta t + \nu \alpha_t \cdot \Delta t] = \frac{E}{1-\nu} \alpha_t \cdot \Delta t$$

$$\sigma_y = \frac{E}{1-\nu^2} [\nu \alpha_t \cdot \Delta t + \alpha_t \cdot \Delta t] = \frac{E}{1-\nu} \alpha_t \cdot \Delta t$$

$$\tau_{xy} = 0 \quad \text{elde edilir.}$$

Sonuçta 181 deyimimden oluşan geritmeler;

$$\sigma_x = \sigma_y = \frac{\sigma}{1-\nu} \alpha_f \cdot \Delta t \quad \text{dur. Breadan materis halde,}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{1-\nu} \alpha_t \cdot \Delta t \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix}$$

yazabiliriz. Burada

18) ya auf elastische vektors $\{\Delta\} = \frac{E}{1-\nu} \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix}$ die.

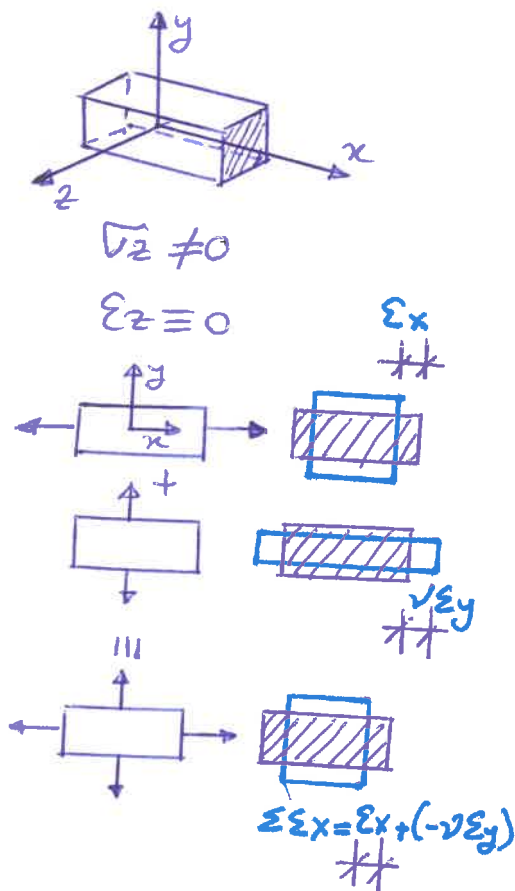
Diş yok etludini le kartarak geritme bagintilari,

$$\sigma_x = \frac{E}{1-\nu^2} (\epsilon_x + \nu \epsilon_y) + \frac{E}{1-\nu} \alpha_f \Delta t$$

$$\sigma_y = \frac{E}{1-\nu^2} (\epsilon_y + \nu \epsilon_x) + \frac{E}{1-\nu} \alpha_t \cdot \Delta t$$

$$\tau_{xy} = G \cdot \gamma_{xy} + 0$$

5.3.2) Isı Değişimini ve Dışlamı Sektör Değişikliğine Hali



Düzlem çoklu doğrultukuna halinde,
birek çoklu doğrultukveler;

$$\Sigma x, \Sigma y, \Sigma z = 0 \quad \text{d.k.} \quad \text{d.e.}$$

$$\begin{aligned} \Sigma x &= \alpha_t \cdot \Delta t & \Sigma x_t &= \Sigma x - \nu \Sigma y \\ \Sigma y &= \alpha_t \cdot \Delta t & \Sigma y_t &= \Sigma y - \nu \Sigma x \end{aligned}$$

Bu durumda,

$$\Sigma x = \alpha_f \Delta t - v \alpha_t \Delta t = (1-v) \alpha_t \Delta t$$

$$\Sigma y = -v \alpha_t \Delta t + \alpha_t \Delta t = (1-v) \alpha_t \Delta t$$

okur.

Geçerli bağıntıları;

$$\sigma_x = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_x + \nu\epsilon_y]$$

$$\sigma_y = \frac{E}{(1+\nu)(1-2\nu)} [\nu\epsilon_x + (1-\nu)\epsilon_y]$$

$$\epsilon_x = (1-\nu)\alpha_t \Delta t$$

$$\epsilon_y = (1-\nu)\alpha_t \Delta t$$

De bu durumda;

$$\sigma_x = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)(1-\nu)\alpha_t \Delta t + \nu(1-\nu)\alpha_t \Delta t]$$

$$\sigma_y = \frac{E}{(1+\nu)(1-2\nu)} [\nu(1-\nu)\alpha_t \Delta t + (1-\nu)(1-\nu)\alpha_t \Delta t]$$

Buradan;

$$\sigma_x = \frac{E(1-\nu)}{(1-2\nu)(1+\nu)} \alpha_t \Delta t$$

$$\sigma_y = \frac{E(1-\nu)}{(1-2\nu)(1+\nu)} \alpha_t \Delta t$$

$$\tau_{xy} = 0$$

Bağıntıların matris formunda yazılması;

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{(1-2\nu)} \left(\frac{1-\nu}{1+\nu} \right) \alpha_t \Delta t \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix} \text{ şeklinde olur. } 1^{\text{nci}} \text{ elastisite}$$

sabitleri matrisi, düzlem sekil değiştirme halinde;

$$\{D\}_t = \frac{E}{(1-2\nu)} \left(\frac{1-\nu}{1+\nu} \right) \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix} \text{ olur.}$$

Düzlem sekil Değiştirme Durumunda;

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

Senaryo olarak dış yüklerle ısı değişiminin birlikte etkileşiminde ki durum selüloz değişimine nasıl tam geçişe bağlıdır;

$$\sigma_x = \frac{E}{(1+\nu)(1-2\nu)} \left\{ [(1-\nu)\epsilon_x + \nu\epsilon_y] + (1-\nu)\alpha_t \Delta t \right\}$$

$$\sigma_y = \frac{E}{(1+\nu)(1-2\nu)} \left\{ [\nu\epsilon_x + (1-\nu)\epsilon_y] + (1-\nu)\alpha_t \Delta t \right\}$$

$$\tau_{xy} = G \gamma_{xy}$$

olarak elde edilir. Şimdi bu durumda selüloz değişimine bağlıdır;

$$\epsilon_x = \frac{1}{E} (\sigma_x - \nu\sigma_y) + (1-\nu)\alpha_t \Delta t$$

$$\epsilon_y = \frac{1}{E} (\sigma_y - \nu\sigma_x) + (1-\nu)\alpha_t \Delta t$$

$$\gamma_{xy} = \frac{\tau_{xy}}{G} \quad \text{şeklinde bulunur.}$$

5.3.3) Isı Değişimi' ve İki Bağıtlu Durum

Isı Değişimi' nedeniyle oluşan birim selüloz değişimleri;

$\epsilon_x = \epsilon_y = \epsilon_z = \alpha_t \Delta t$ 'dir. Buradan geliriz;

$$\sigma_x = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_x - \nu(\epsilon_y + \epsilon_z)]$$

$$= \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\alpha_t \Delta t + \nu(2\alpha_t \Delta t)]$$

$$= \frac{E}{(1-2\nu)} \alpha_t \Delta t$$

$$\sigma_y = \frac{E}{(1-2\nu)} \alpha_t \Delta t$$

$$\tau_{xy} = \tau_{yz} = \tau_{xz} = 0 \quad \text{dur.}$$

$$\sigma_z = \frac{E}{(1-2\nu)} \alpha_t \Delta t$$

Isı elastik sabitler matrisi ise;

$$\{\sigma\} = \frac{E}{(1-2\nu)} \begin{bmatrix} 1 \\ 1 \\ 1 \\ 0 \\ 0 \\ 0 \end{bmatrix} \text{ şeklinde dir.}$$

Isı ve dış yükler dahil olman durumunda elastikite bağıntıları;

$$\epsilon_x = \frac{1}{E} [\sigma_x - \nu (\sigma_y + \sigma_z)] + \alpha_t \cdot \Delta t$$

$$\epsilon_y = \frac{1}{E} [\sigma_y - \nu (\sigma_x + \sigma_z)] + \alpha_t \cdot \Delta t$$

$$\epsilon_z = \frac{1}{E} [\sigma_z - \nu (\sigma_x + \sigma_y)] + \alpha_t \cdot \Delta t$$

$$\gamma_{xy} = \frac{\tau_{xy}}{G} ; \gamma_{xz} = \frac{\tau_{xz}}{G} ; \gamma_{yz} = \frac{\tau_{yz}}{G} \text{ dir.}$$

5.3.4) Isı Değişimi ve Ortotrop Durum

Ortotrop durumda selül değişkenler ;

$$\frac{\nu_x}{E_x} = \frac{\nu_y}{E_y}$$

$$\epsilon_x = \alpha_x \cdot \Delta t$$

$$\epsilon_y = \alpha_y \cdot \Delta t$$

$$\gamma_{xy} = 0 \text{ 'dir.}$$

Elastikite bağıntıları matris halde ;

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \frac{1}{1-\nu_x \nu_y} \begin{bmatrix} E_x (\alpha_x + \nu_y \alpha_y) \\ E_y (\nu_x \alpha_x + \alpha_y) \\ 0 \end{bmatrix}$$

olarak bulunur.

5.4) LEVHA, PLAK VE KABUKLARIN ELASTİTE BAĞINTILAR

5.4.1) İzotrop Levhanın Bonye Denklemleri

Kesit kesiklerindeki şekil değişimleri arasındaki elastisite bağıntıları;

$$\begin{Bmatrix} N_x \\ N_y \\ N_{xy} \end{Bmatrix} = C \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

Buradaki katsayılar matrisi;

$$[C] = \begin{bmatrix} C & C\nu & 0 \\ C\nu & C & 0 \\ 0 & 0 & C\frac{1-\nu}{2} \end{bmatrix} \quad C = \frac{Et}{1-\nu^2}$$

5.4.2) İzotrop Plâğin Bonye Denklemleri

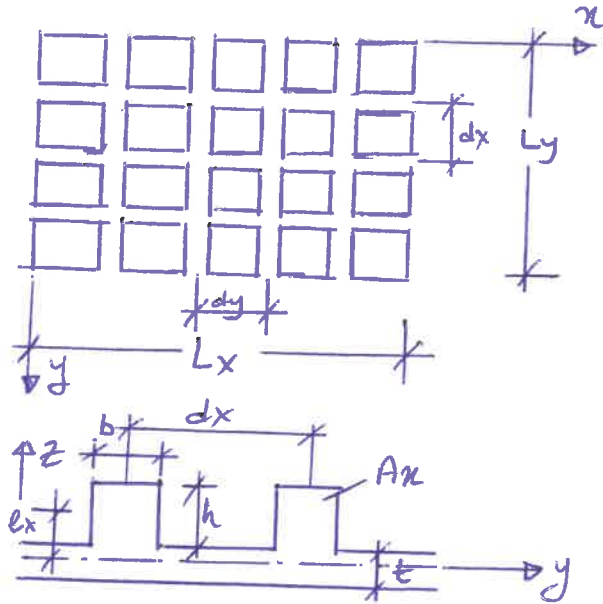
$$\begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix} = D \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \kappa_x \\ \kappa_y \\ \kappa_{xy} \end{Bmatrix}$$

Buradaki katsayılar matrisi;

$$[D] = \begin{bmatrix} D & D\nu & 0 \\ D\nu & D & 0 \\ 0 & 0 & \frac{1-\nu}{2} D \end{bmatrix} \quad D = \frac{Et^3}{12(1-\nu^2)} = \frac{EI}{(1-\nu^2)}$$

I = Kesit atalet momenti

5.4.3) Nervüelü Ortotrop Plâgin Bünge Denklemleri



$$\begin{bmatrix} N_x \\ N_y \\ N_{xy} \\ M_x \\ M_y \\ M_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & 0 & 0 & 0 & 0 \\ C_{21} & C_{22} & 0 & 0 & 0 & 0 \\ 0 & 0 & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{44} & C_{45} & 0 \\ 0 & 0 & 0 & C_{54} & C_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & C_{66} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ k_x \\ k_y \\ k_{xy} \end{bmatrix}$$

$$C_{11} = C + \frac{EA_x}{dx}$$

$$C_{44} = D + \frac{EI_x}{dx} = D + \frac{E(I_x + A_x \cdot e_x^2)}{dx}$$

$$C_{12} = \nu C = C_{21}$$

$$C_{45} = \nu D$$

$$C_{22} = C + \frac{EA_y}{dy}$$

$$C_{55} = D + \frac{EI_y}{dy} = D + \frac{E(I_y + A_y \cdot e_y^2)}{dy}$$

$$C_{33} = \frac{1-\nu}{2} C$$

$$C_{66} = (1-\nu)D + \frac{1}{2} \left[\frac{GJ_x}{dx} + \frac{GJ_y}{dy} \right]$$

Burada; $C = \frac{Et}{1-\nu^2}$, $D = \frac{Et^3}{12(1-\nu^2)}$, dx ve dy sırasıyla x ve y

doğru/luşundaki nervür aralığı, A_x ve A_y nervür alanları, I_x ve I_y de x ve y yönlerindeki atalet momentleridir.

5.4.4) İzotrop Kabukların Bınye Denklemleri

$$\begin{Bmatrix} N_x \\ N_y \\ N_{xy} \\ M_x \\ M_y \\ M_{xy} \end{Bmatrix} = \begin{bmatrix} C_{11} & C_{12} & 0 & 0 & 0 & 0 \\ C_{21} & C_{22} & 0 & 0 & 0 & 0 \\ 0 & 0 & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{44} & C_{45} & 0 \\ 0 & 0 & 0 & C_{54} & C_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & C_{66} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ k_x \\ k_y \\ k_{xy} \end{Bmatrix}$$

Burada izotrop durum için;

$$\begin{Bmatrix} N_x \\ N_y \\ N_{xy} \\ M_x \\ M_y \\ M_{xy} \end{Bmatrix} = \begin{bmatrix} C & \nu C & 0 & 0 & 0 & 0 \\ \nu C & C & 0 & 0 & 0 & 0 \\ 0 & 0 & \frac{1-\nu}{2}C & 0 & 0 & 0 \\ 0 & 0 & 0 & D & \nu D & 0 \\ 0 & 0 & 0 & \nu D & D & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1-\nu}{2}D \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ k_x \\ k_y \\ k_{xy} \end{Bmatrix}$$

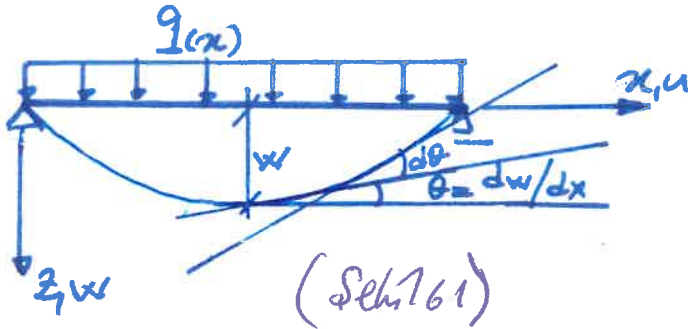
Burada, $C = \frac{Et}{1-\nu^2}$ $D = \frac{Et^3}{12(1-\nu^2)}$ 'dir.

BÖLÜM II

ENERJİ YÖNTEMLERİ

6) TOPLAM POTANSİYEL ENERJİ

6.1) Toplam Potansiyel Enerjinin 1. Varyasyonu (Denge Denklemleri)



w = (Çökme)

$$\frac{dw}{dx} = w' = \theta = (\text{Dönme})$$

$$-\frac{d^2w}{dx^2} = -\frac{d\theta}{dx} = -\frac{1}{\rho} = -\frac{M}{EI}$$

$$-EIw'' = M \text{ (moment)}$$

$$-\frac{d^3w}{dx^3} = \frac{d^2\theta}{dx^2} = -\frac{1}{EI} \frac{dM}{dx} = -\frac{V}{EI}$$

$$-EIw''' = V \text{ (Kesme Kuvveti)}$$

$$\frac{d^4w}{dx^4} = \frac{d^3\theta}{dx^3} = \frac{1}{EI} \frac{d^2M}{dx^2} = \frac{1}{EI} \frac{dV}{dx} = \frac{1}{EI} q(x)$$

$$EIw'''' = q \text{ (Yayılı yük)}$$

çubuğun diferansiyel denklemi de;

$$EI \frac{d^4w}{dx^4} = q(x) \rightarrow EIw'''' = q(x) \text{ olur.}$$

Sınır koşulları	}	$x=0$ 'da $w=0$	Geometrik şartlar
		$x=L$ 'de $w=0$	
		$x=0$ 'da $w''=0$	dinamik şartlar.
		$x=L$ 'de $w''=0$	

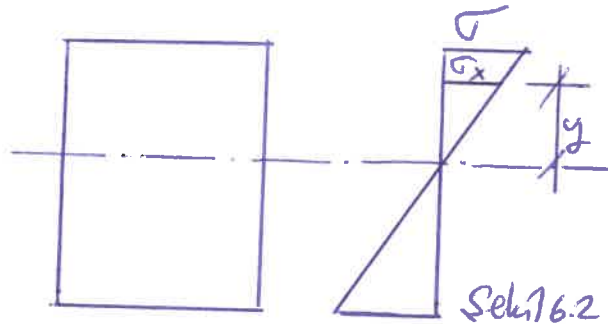
Bir çubuğun sınır koşullarını sağlayan pek çok yer değiştirme fonksiyonu $u=U_0$ iğinden yalnızca gerçek çözümü sağlayan bir $u=U_0(x)$ potansiyel enerjinin ifadesini (Π) minimum yapar. Başka bir deyişle toplam potansiyel enerji, sistemin gerçek denge konumunda bir ekstremumdan geçer. Böylece belirli bir integral ifadesi ekstremum yapan konumun bulunmasına problemi indirgenmiş olur. Bu ekstremum değeri bir minimumdur. Bu hesaba da Varyasyon Hesabı denir.

$$\Pi = U + W \quad (\text{Toplam potansiyel enerji})$$

Burada; U : Tg kuvvetlerin toplam potansiyel enerjisi
 W : dış

dış.

6.1.1.) Tg Kuvvetlerin Potansiyel Enerjisi



$$\sigma_x = \frac{M}{I} \cdot y$$

$$\epsilon_x = y \cdot \frac{d\theta}{dx} = y \cdot \frac{M}{EI}$$

$$U_1 = \int \frac{1}{2} \sigma_x \cdot \epsilon_x (dA) dx = \int \frac{1}{2} \cdot \frac{M}{I} \cdot y \cdot \frac{My}{EI} dA dx = \frac{1}{2} \int \frac{M^2}{EI} dx$$

$$U_1 = \frac{1}{2} \int EI \left(\frac{d^2 w}{dx^2} \right)^2 dx$$

$$\int y^2 dA = I$$

$$U = \int \frac{1}{2} \frac{M^2}{EI} dx + \int \frac{1}{2} \frac{N^2 L}{EA} dx + \int \frac{1}{2} \frac{V^2}{k'AG} dx$$

basit eğilme

basit uzama

basit kayma

$$\frac{1}{2} \int EI (w'')^2 dx$$

$$\frac{1}{2} \int EA (u')^2 dx$$

$$\frac{1}{2} \int GA' (\gamma')^2 dx$$

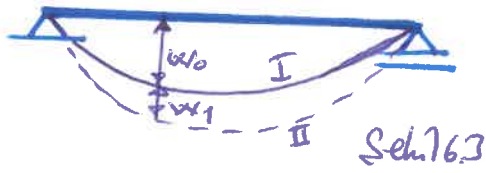
6.1.2.) Dış Kuvvetlerin Potansiyel Enerjisi

$$W = - \int q \cdot w dx$$

6.1.3.) Toplam Potansiyel Enerji

$$\begin{aligned} \Pi = U + W &= \int_0^L \frac{1}{2} \frac{M^2}{EI} dx - \int_0^L q \cdot w dx \\ &= \int_0^L \left(\frac{EI}{2} (w'')^2 - q \cdot w \right) dx \end{aligned}$$

6.1.4) Toplam Potansiyel Enerjinin Minimizasyonu



Değişim (varyasyon) : Gubuklatılacak yer
değiştikene $w \rightarrow w_0 + w_1$ olur. Skaler
bir büyüklük olan Π toplam potansiyel
enerji de $\Pi + \delta\Pi$ kadar artar.

$$\delta\Pi = \Pi_2 - \Pi_1$$

$$\delta\Pi = \Pi_2(w_0 + w_1) - \Pi_1(w_0)$$

I $\rightarrow$ Gececek konum (değiştirilmiş durumu)

II $\rightarrow$ Köprü konum

$$\delta\Pi = \left[\int_0^l \frac{EI}{2} (w_0'' + w_1'')^2 dx - \int_0^l q (w_0 + w_1) dx \right] - \left[\int_0^l \frac{EI}{2} (w_0'')^2 dx - \int_0^l q w_0 dx \right]$$

$$\delta\Pi = \int_0^l EI w_0'' w_1'' dx - \int_0^l q w_1 dx + \int_0^l \frac{EI}{2} (w_1'')^2 dx$$

$$\int_0^l \frac{EI}{2} (w_1'')^2 dx = \alpha > 0$$

İkinci mertebe terim, her zaman
sifirdan büyük ve pozitif

Bu denkleme kısmi integral uygularsak;

$$\int u dv = uv - \int v du$$

$$\int_0^l EI w_0'' w_1'' dx = \left[EI w_0'' w_1' \right]_0^l - \left[EI w_0''' w_1 \right]_0^l + \int_0^l EI w_0'''' w_1 dx$$

Dinamik $\left\{ \begin{array}{l} x=0 \rightarrow w''=0 \\ x=L \rightarrow w''=0 \end{array} \right.$

Sartlar $\left\{ \begin{array}{l} x=0 \rightarrow w_1=0 \\ x=L \rightarrow w_1=0 \end{array} \right.$

$x=0 \rightarrow w_1=0$

$x=L \rightarrow w_1=0$

Sinir

Sartları

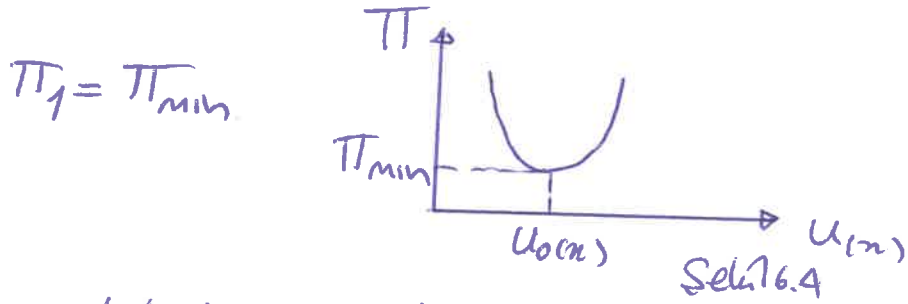
$$\delta\Pi = \int_0^l EI w_0'''' w_1 dx - \int_0^l q w_1 dx + \alpha = \left[\int_0^l (EI w_0'''' - q) dx \right] w_1 + \alpha$$

Hatırlatma $\rightarrow EI \frac{d^4 w}{dx^4} = q(x)$

$EI w_0'''' - q = 0$

0 olduğundan parantez içindeki kısım sifirdır.

Bu durumda $\delta\pi = \alpha > 0$ kalir. Bunun anlamı dege konumuna komzu olan bütün konumlardaki toplam potansiyel enerji; dege konumundakinden daha fazladir. O halde "1" konumundaki (dege konumundaki) toplam potansiyel enerji minimumdur.



İkinci' mertebeden bir büyüklük olan α birinci' mertebeden büyüklüklerin yanında ihmal edilirse $\alpha = 0$ yazılır ve dege hali için denge denklemleri elde edilir.

$$\delta\pi = 0 \quad (\text{denge denklemleri})$$

Ayrıca $\delta\pi$ 'den birinci' dereceden ve ikinci' dereceden terimlerin toplamına π 'nin birinci' ve ikinci' varyasyonları denir. Bunlar $\delta\pi$ ve $\frac{\delta^2\pi}{2!}$ ile gösterilir.

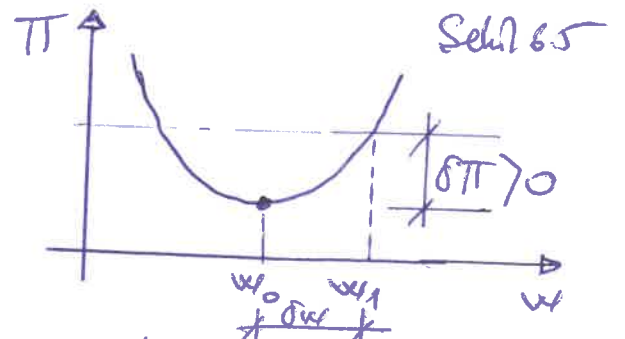
$$\delta\pi = \delta\pi + \frac{1}{2!} \delta^2\pi$$

$$\delta\pi = \int_0^L (\bar{E} w_0'' w_1'' - q \cdot w_1) dx = 0 \quad \text{dege denklemleri}$$

$$\frac{1}{2!} \delta^2\pi = \int_0^L \frac{\bar{E}}{2} (w_1'')^2 dx = \alpha > 0$$

Bu durumda dege hali için;

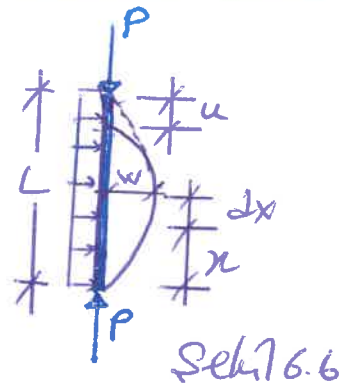
$$\left. \begin{array}{l} \delta\pi = 0 \\ \frac{1}{2!} \delta^2\pi > 0 \end{array} \right\} \text{de } \pi_{\min} \text{ minimumdur.}$$



Bu durumda elastik bir sistemde, geometrik sınır şartlarını sağlayan bütün yer değiştirmeye konumlar arasında; gerçek konum toplam potansiyel enerjisi minimum yapandır.

6.2) Toplam Potansiyel Enerjinin 2. Varyasyonu (Stabilite Durumu)

Eksenel yüklü bir elemanda baya yer değiştikmeye ek olarak eğilme ve yer değiştikmeler meydana gelirse bu elemanda burkulma problemi var demektir (Şekil 6.6). Cukuklatılı toplam potansiyel enerji;



$$\Pi = (U_m + U_b) + W \quad \text{'dir. Burada;}$$

U_m : Membran şekil değışikme enerjisi

U_b : Eğilme " " "

W : Dış yük potansiyel enerjisi 'dir.

$$\Pi = \left[\frac{EA}{2} \int_0^L \epsilon^2 dx + \frac{EI}{2} \int_0^L (w'')^2 dx \right] + P[u(L) - u(0)]$$

$$\Pi = \int_0^L \frac{EA}{2} \epsilon^2 dx + \int_0^L \frac{EI}{2} (w'')^2 dx + \int_0^L P u' dx$$

Burada;

u' : eksenel kuvvete bağlı doğrusal şekil değışikme

w'' : eğilmeye bağlı doğrusal şekil değışikmedir.

$$\epsilon = u' + \frac{1}{2} (w')^2 \quad \text{'dönme merkezi' ekseninde}$$

uzama şekil değışimidir Burada;

w' = büyük sapmaya bağlı doğrusal olmayan şekil değışikmedir.

ϵ değeri şeklinde yerine yazarak;

$$\leftarrow \Pi = \int_0^L \left[\frac{EA}{2} (u' + \frac{1}{2} w'^2)^2 + \frac{EI}{2} (w'')^2 + Pu' \right] dx$$

u_0 ve w_0 deger konumunu, u_1 ve w_1 ise sonuz kucuk artimin obdugu ikinci konumu gosternek icin ve buna gore;

$$\left. \begin{aligned} u &\rightarrow u_0 + u_1 \\ w &\rightarrow w_0 + w_1 \end{aligned} \right\} \text{artimlar}$$

$$\leftarrow u_0 = -\frac{P}{EA} x \rightarrow u_0' = -\frac{P}{EA} ; w_0 = 0$$

$$\left[\varepsilon = -\frac{P}{EA} , \frac{du}{dx} = -\frac{P}{EA} ; \rightarrow u = -\frac{P}{EA} x \right] \text{Kuvvetten dolgi}$$

Artimlar degerleri deklunde yerine yazariz;

$$\rightarrow \Pi + \Delta \Pi = \int_0^L \left\{ \frac{EA}{2} \left[(u_0' + u_1') + \frac{1}{2} (w_0' + w_1')^2 \right]^2 + \frac{EI}{2} (w_0'' + w_1'')^2 + P(u_0' + u_1') \right\} dx$$

$$\Pi = \int_0^L \left[\frac{EA}{2} (u_0' + \frac{1}{2} w_0'^2)^2 + \frac{EI}{2} (w_0'')^2 + Pu_0' \right] dx$$

$$\Delta \Pi = (\Pi + \Delta \Pi) - \Pi$$

Boylece potansiyel enerjideki degisimi seetlere cikarsak;

$$\Delta \Pi = \delta \Pi + \frac{1}{2!} \delta^2 \Pi + \frac{1}{3!} \delta^3 \Pi + \dots$$

$$\frac{1}{2!} \delta^2 \Pi = \frac{1}{2} \int_0^L \left[EA (u_1')^2 + EI (w_1'')^2 - P (w_1')^2 \right] dx$$

olur.

Yukarıda verilen $\frac{1}{2!} \delta^2 \pi$ bağıntısı π hâz dereceden terimlerin toplamını vermektedir. Üçüncü ve dördüncü dereceden olup terimleri ihmal edilmiştir.

Burada kritik yük olarak tanımlanan P 'nin kritik değerleri için; u_1 ve w_1 'in bütün sıfır olmayan varyasyonları için $\delta^2 \pi > 0$ 'dır.

$$\delta(\delta^2 \pi) = 0, \quad \delta \pi = 0$$

verilen bağıntıların u_1 ve w_1 'e göre kriteri de sıfırdır.

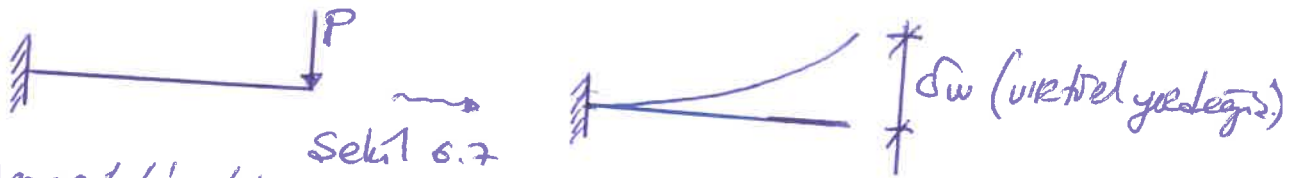
$$\frac{\partial(\delta^2 \pi)}{\partial u_1} = 0 \quad \frac{\partial(\delta^2 \pi)}{\partial w_1} = 0$$



6.3) VİKTÜL İS PRİNSİBİ

Viktüel, gerçekte nasıl olduğunu bilmediğimiz bir fiziksel büyüklüğün mevcut sınır koşulları ile uyumlu olacak birim varsayacağımız durumunu ifade ederken kullandığımız bir terimdir. Viktüel kuvvetler ve viktüel deplasmanlar metodu olarak ifade edilir.

Örneğin, aşağıdaki şekilde kiriş ucuna P yükü etliriz. Kiriş ucundaki yer değiştirmeyi viktüel olarak ele aldığımızda yer değiştirmeyi yukarıya veya aşağıya doğru alabiliriz. Ancak yer değiştirme eğrisi kirişin sınır koşullarını sağlamalıdır.



Şekil 6.7

Dengedeki bir cisim, sınır koşullarıyla uyumlu bir viktüel şekil değiştirmeye uğradığında, iç kuvvetlerin işi (δU), dış kuvvetlerin viktüel işine (δW) eşit olur.

$$\delta U = \delta W$$

U gerçekte yer değiştirmeye, δU viktüel yer değiştirmeye, ϵ gerçekte deplasman, $\delta \epsilon$ viktüel deplasman ile;

$$U = \frac{1}{2} \int_V \sigma \cdot \epsilon \cdot dV \quad \text{İçerik}$$

$$\{\sigma\} = [C] \{\epsilon\}$$

$$\delta U = \int_V \{\delta \epsilon\}^T \{\sigma\} dV = \int_V \{\delta \epsilon\}^T [C] \{\epsilon\} dV$$

X kütleesel kuvveti, P yüzeyel kuvveti, q çizgisel kuvveti, P moment kuvveti gösterirse dış kuvvetlerin viktüel işi;

$$\delta W = \int_V \{\delta u\}^T \{X\} dV + \int_A \{\delta u\}^T \{P\} dA + \int_s \{\delta u\}^T \{q\} ds + P$$

$$\delta U = \delta W$$

$$\delta U = \int_V \{\delta \epsilon\}^T [D] \{\epsilon\} dV \quad \text{veya} \quad \delta U = \int_V \{\delta \epsilon\}^T \{\sigma\} dV$$

$$\{\sigma\} = [D] (\{\epsilon\} - \{\epsilon_0\}) + \{\sigma_0\} - \alpha \Delta t \{\sigma_T\}$$

Burada;

$[D]$ = malzemenin elastisite sabiti'

$\{\epsilon_0\}$ = başlangıç şekil değişimi

$\{\sigma_0\}$ = " gerilmesi

α = genleşme katsayısı

Δt = sıcaklık değişimi

$\{\sigma_T\}$ = sıcaklık elastik malzeme ' dir.

$$\delta U = \int_V \{\delta \epsilon\}_i^T [D] \{\epsilon\}_i dV - \int_V \{\delta \epsilon\}_i^T [D] \{\epsilon_0\}_i dV + \int_V \{\delta \epsilon\}_i^T \{\sigma_0\}_i dV$$

$$- \int_V \{\delta \epsilon\}_i^T \cdot \alpha \cdot \Delta t \{\sigma_T\}_i dV$$

$$\delta W = \int_V \{\delta u\}_i^T \{X\} dV + \int_A \{\delta u\}_i^T \{P\} dA + \int_S \{\delta u\}_i^T \{q\} dS + \{d\}^T P$$

$$- \int_A c \{\delta w\}_i^T \{w\} dA$$

Burada;

u = yer değişimi

X : kütleli kuvvet

P : yüzeyel yük

q : çizgisel "

P : tekil yük

c : elastik yataklanma katsayısı ' dir.

$$\delta U = \delta W$$

BÖLÜM III

7.) SONLU ELEMANLAR YÖNTEMİ

7.1) GİRİŞ

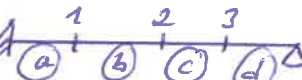
Sonlu Elemanlar Yöntemi; sürekli bir sistemi problemin karakterine uygun sonlu elemanlara ayırarak elle edilen elemanlar üzerinde iç ve dış kuvvetlerin enerjisinin minimize edilmesi ve sonra bu elemanların birleştirilmesi şeklinde bir uygulama getirmek. Bunun sonucu olarak nesnet şartları, sistemin üst özellikleri dış yüklerin sürekli ya da ani değişimleri kolayca gözönüne alınabilir. Dolayısıyla sonlu elemanlar yöntemi analitik yöntemlerle çözülemeyen karmaşık problemlere uygulanabilir. Yüzeyel sistemin diğer bölgeleğinde eleman bağlantıları kuvvetler o bölgenin daha hassas incelenmesi mümkün olur. Diğer bir avantajda, sınır şartlarının problemin çözüm sürecine göre en son adımda hesaplara dahil edilmesidir. Böylelikle sürekli sınır şartlarını probleme uygularken birtakım uygun hesaplara girilmesi.

Sonlu elemanlar yönteminin kabuk sistemlere uygulanışına üst çalışmaların bir kısmında, sistem düzlemsel sonlu elemanlara ayrılmaktadır. Bu tip hesaplarda eleman özel elemanlarının sistem elemanlarına dönüştürülmesinde, sistem rijitlik matrisinin hesabında bazı güçlüklerle karşılaşılacaktır. Kabuk formuna uygun eğrisel sonlu eleman kullanılması, bu güçlüğü ortadan kaldırmaktadır. Yüzeyin geometrisine bağlı olarak elemanın, silindirik bir, küresel veya üç boyutlu olabilir. Eleman sınırları düz veya eğri çizgi olabilir.

Sonlu elemanlar yönteminde sisten sonlu sayıda elemana ayrılmaktadır. Eleman boyutları küçük olursa problemin hata oranı azalmakta, fakat çözüm süresi uzamaktadır. Sistemi oluşturan elemanların her birine sonlu eleman denir ve birleştirilerek köşe noktaları da düğüm noktaları olarak adlandırılır (Şekil 7-1).

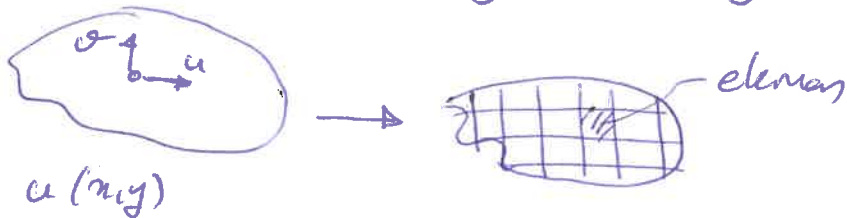
1) Çözüm yapılacak eleman 1 boyutlu ise; 

(1 boyutlu ise)

sonlu elemanlar ayrışması; 

eleman seçimi; 

2) Çözüm yapılacak eleman 2 boyutlu ise (2 boyutlu ise);

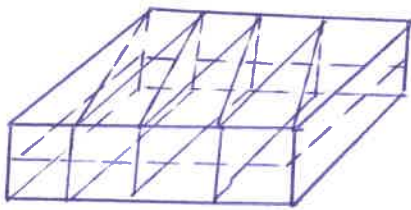


$u(x,y)$

$v(x,y)$

sonlu eleman seçimi; 

3) Çözüm yapılacak eleman 3 boyutlu ise;



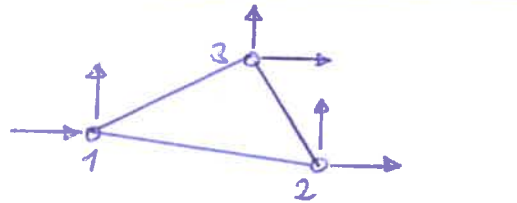
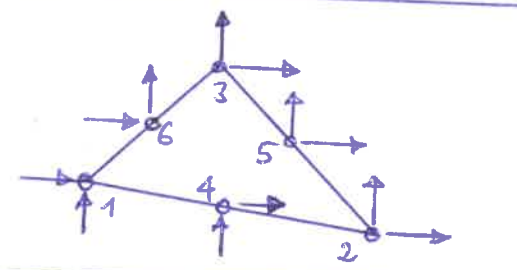
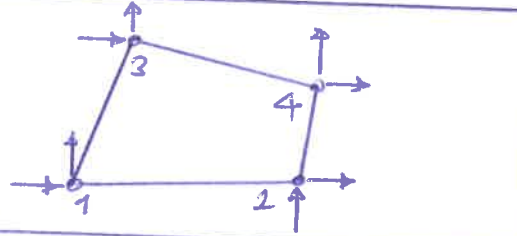
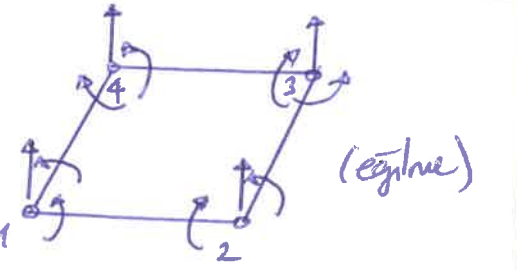
sonlu eleman seçimi;

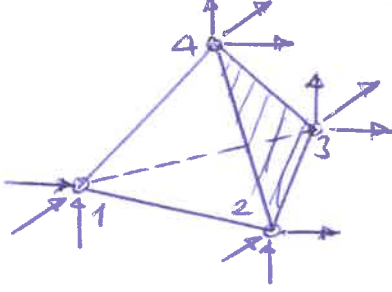
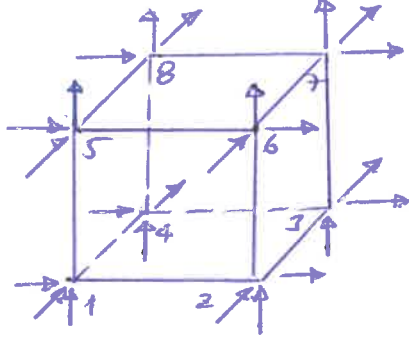
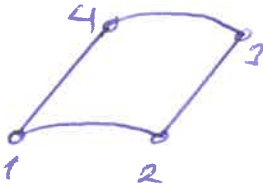
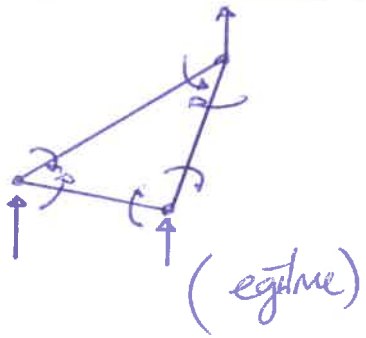


Sonlu eleman yüzeyinin şekli değiştirilmesi; düğüm noktalarının deplasman parametrelerine bağlı olarak ifade edilebilir. Deplasman parametreleri; deplasman bileşenleri, dönme ve bükülme eğitimi gibi deplasman vektörlerini içermektedir. Eğilme hesaplarında düğüm noktalarının deplasman

parametrelerinin belirlenmesi, sistemin deplasman yüzeyinin ve her düğüm noktasındaki kesit tesirlerinin bulunması için yeterlidir. Stabilité hesabında ise, bu deplasman parametrelerini göre kurulan denklemler takımının (Δ) katsayıları determinanının sıfır ya da yük, yani kritik yük (P_{kr}) belirlenir.

Bazı Sık Karşılaşılan Tipleri ve Serbestlik Dereceleri

Eleman	Şekil	Uygulama	Düğüm noktaları	Düğüm noktaları serbestlik dereceleri
Kafes (Bar) Çubuğu		Kafes elemanı	2	1
Kiriş (Beam)		Geçer	2	3
Üçgen (Triangular)		Düzlem Geometre	3	2
Üçgen (Triangular)		"	6	2
Dörtgen (Quadrilateral)		"	4	2
Dikdörtgen (Rectangular)		Plak (eğilme)	4	3

Eleman	Sebat	Uygulama	Düğümlerin Serbestlik Derecesi	Düğümlerin Sayısı
Dört üçgen gövde (Tetrahedron)		Üç eksenli geçirme	4	3
Prizma (Prism)			8	3
Dörtgen kabuk eleman (Rectangular shell element)		Silindirik kabuk, fanon	4	5-6
Üçgen eleman		plak	3	3

7.2) Deplasman Fonksiyonlarının Seçimi

Deplasman fonksiyonları; rijit cisim hareketi'ne sabit deformeasyon şartını sağlayacak şekilde seçilmelidir. Koordinat eksenleri değişince çözüm farklı olmalıdır. Bunun için deplasman fonksiyonları ya tam polinom veya sabit koordinatlarının fonksiyonu şeklinde olmalıdır. Elemanın içinde ve kenarında sürekli olmalıdır. Ayrıca iç ve dış kuvvetlerin içindeki türevlerde de sürekli olmalıdır.

Herhangi bir (e_i) elemanına ait ve bu elemanın içinde ya da sınırları üzerinde bir (i) noktasındaki deplasman (ya deforme) vektörü $\{u\}$ olsun;

$$\{u\}^e = \begin{Bmatrix} u(x,y) \\ v(x,y) \\ w(x,y) \end{Bmatrix} = [\phi(x,y)] \{a\}$$

Burada $[\phi(x,y)]$ seçilen deplasman fonksiyonudur. Herp kopylığı bakımından genellikle polinom seçilir. Pascal üssü polinomların seçilmesine yardımcı olur. Polinomlarda türev ve integral alınabilir daha kolaydır. Gerçek çözüme istenilen yaklaşımları sağlamak mümkündür. Pascal polinomları "seçim fonksiyonları" olarak kullanılabilir. Seçilen deplasman fonksiyonları tam bir polinom ise geometrik izotropi sağlar. Eğer polinom tam değil; fakat simetrik varsa, yine geometrik izotropi vardır. Bağıntıda verilen $\{a\}$ bilinmeyen katsayılarıdır. Bu $\{a\}$ katsayılarının sayısı, bir elemandaki düğüm noktalarının deplasman parametrelerinin toplam sayısına eşit olmalıdır.

ÖRNEK : Yer değıştirme vektörö bir polinom olsun;

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

Bu durumda deplasman fonksiyonu ;

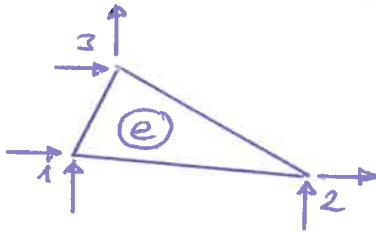
$$\{\phi(x,y)\} = [1, x, x^2, x^3]$$

ve bilinmeyen kat sayılar matrisi;

$$\{a\} = \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{Bmatrix} \text{ olacaktır.}$$

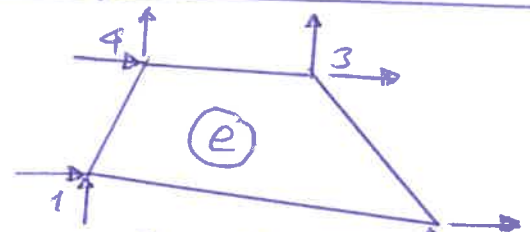
Seçilen elemanın düğüm noktaları parametreleri ise;

Üç düğüm noktalı elemanda



$$\{d\} = \begin{Bmatrix} [d]_1 \\ [d]_2 \\ [d]_3 \end{Bmatrix} = \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \\ u_3 \\ v_3 \end{Bmatrix}$$

Dört düğüm noktalı elemanda



$$\{d\} = \begin{Bmatrix} [d]_1 \\ [d]_2 \\ [d]_3 \\ [d]_4 \end{Bmatrix} = \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \\ u_3 \\ v_3 \\ u_4 \\ v_4 \end{Bmatrix}$$

şeklinde dir. Elemanın herhangi bir (i) düğüm noktasında tanımlanan deplasman parametreleri en genel halde ;

$$\{d\}_i = \{u, v, w, \theta_x, \theta_y, \theta_z, \tau, \dots\}$$

$$i = 1, 2, 3, 4, \dots$$

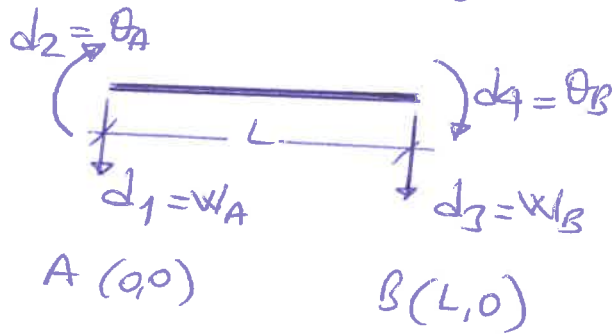
şeklinde dir.

Eleman düğüm noktaları deplasmanları $\{d\}$ ile, orijin sabitleri $\{a\}$ arasındaki bağı veren $[A]$ bağ matrisi ile, elemanın düğüm noktalarının deplasman parametrelerinin $\phi(x,y)$ ve $\phi'(x,y)$ 'nin türevleri' anında yazılmış değerlerin düğüm noktası koordinatlarının yerlerine konulması ile bulunur.

Örneğin; Yer değiştirme vektörü,

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3 \text{ olsun.}$$

$$w = [\phi(x)] \{a\} = [1, x, x^2, x^3] \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{Bmatrix}$$



$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

$$\theta = \frac{dw}{dx} = 0 + a_2 + 2a_3 x + 3a_4 x^2$$

A(0,0) noktasında x yerine 0 yazılırsa; $w_A = a_1 + 0 + 0 + 0$

$$\theta_A = 0 + a_2 + 0 + 0$$

B(L,0) " " " " L " "

$$w_B = a_1 + a_2 L + a_3 L^2 + a_4 L^3$$

$\theta_B = 0 + a_2 + 2a_3 L + 3a_4 L^2$ olur. Böylece $[A]$ matrisi,

$$[A] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 1 & L & L^2 & L^3 \\ 0 & 1 & 2L & 3L^2 \end{bmatrix}$$

elde edilir.

Burada $\{d\}$ ile $\{a\}$ matrisleri bilinmeyenlerdir. Bağ matrisi veritisiyle bilinmeyen yalnız $\{d\}$ deplasman parametreleri kalır ve $\{a\}$ kat sayılar $\{d\}$ deplasman parametreleri açısından hesaplanır.

$$\{d\} = [A]\{a\}$$

$$\{a\} = [A]^{-1}\{d\}$$

$$[B] = [A]^{-1} \text{ desek,}$$

$\{a\} = [B]\{d\}$ olur. Bu bağıntıyı $\{u\} = \begin{Bmatrix} u(m,y) \\ v(m,y) \\ w(m,y) \end{Bmatrix} = [\phi(m,y)]\{a\}$ bağıntısında yerine koyarsak;

$$\{u\} = \begin{Bmatrix} u(m,y) \\ v(m,y) \\ w(m,y) \end{Bmatrix} = [\phi(m,y)][B]\{d\} \quad \begin{array}{l} \text{elde edilir ki:} \\ \text{bu durumda elemanın} \end{array}$$

deplasman vektörü, elemanın diğer noktalarının deplasman parametreleri açısından belirlenmiştir.

Bağıntıdaki $[\phi(m,y)][B]$ ifadesine şekil fonksiyonu denir.

$$[\phi(m,y)][B] = [N]$$

Bağıntıdan deplasman parametresi kadar şekil fonksiyonu elde edilir.

$$\{u\} = [N]\{d\}$$

Selül değişkenler de;

$$\begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix} = \underbrace{\begin{bmatrix} \partial/\partial x & 0 \\ 0 & \partial/\partial y \\ \partial/\partial y & \partial/\partial x \end{bmatrix}}_{[\Delta]} \begin{Bmatrix} u \\ v \end{Bmatrix}$$

$[\Delta]$ = operatör matrisi

$$\{\varepsilon\} = [\Delta] \{u\}$$

$$\{u\} = [N] \{d\}$$

$$\{\varepsilon\} = \underbrace{[\Delta][N]}_{[G]} \{d\} = [G] \{d\}$$

$[G]$: selül değişken matrisi

(1) düğüm noktasındaki d_1 deplasman parametresine ait N_1 selül fonksiyonu öyle hesaplanır ki; (1) düğüm noktasının (x, y, z) koordinatları olduğu zaman,

$$N_1 = 1 ; N_2 = N_3 = \dots = N_n = 0$$

dur. Belirlenen bu selül fonksiyonları, eleman şekli ve boyutu değişmediği sürece her elemanda aynıdır. Yalnız deplasman parametreleri elemandan elemana değişir. Probleme bu deplasman parametreleri bilinmeyen olarak hesaplanır.

7.3) Genel Düz Yüz Halinde Sorlu Elemanlar Yöntemiyle Eğilme Hesabı

Dengede olan bir cisim, herhangi bir yer ve selül değişimmesinde T_i kuvvetlerin R_i , d_i kuvvetlerin R_i ne eşittir. T_i kuvvetlerin vektörel R_i ni σ_u , d_i kuvvetlerin vektörel R_i ni σ_v olarak göstererek;

$$\sigma_u = \sigma_v = \dots \text{ dir.}$$

Tabii durum gercek yutulene, $\bar{\epsilon}$ birim durumu ve viktrel yer degistirme ve viktrel sekil degistirme durumu olarak alinip viktrel tg kuvvetlere, viktrel ds kuvvetler isini yazarsak;

$$\delta U = \int_V \{ \delta \epsilon \}^T \sigma dV$$

$$\{ \sigma \} = [D] (\{ \epsilon \} - \{ \epsilon_0 \}) + \{ \sigma_0 \} - \alpha \cdot \Delta t + \{ \sigma_T \}$$

Burada;

$[D]$ = malzeme elastisite matrisini

$\{ \epsilon_0 \}$ = bazlangi sekil degistirmesini

$\{ \sigma_0 \}$ = bazlangi gerilmesini

α : genlesme katsayisini

Δt : sıcaklik degisimini

$\{ \sigma_T \}$: sıcaklik elastisite matrisini gostermektedir.

$\{ \sigma \}$ ifadesini δU bagintisinda yerine yazarsak;

$$\delta U = \int_V \{ \delta \epsilon \}^T [D] \{ \epsilon \} dV - \int_V \{ \delta \epsilon \}^T [D] \{ \epsilon_0 \} dV + \int_V \{ \delta \epsilon \}^T \{ \sigma_0 \} dV - \int_V \{ \delta \epsilon \}^T \alpha \Delta t dV + \int_V \{ \delta \epsilon \}^T \{ \sigma_T \} dV$$

$$\delta K = \int_V \{ \delta u \}^T \{ x \} dV + \int_A \{ \delta u \}^T \{ p \} dA + \int_S \{ \delta u \}^T \{ q \} dS + \int_A \{ \delta \}^T \{ p \} dA - \int_A \{ \delta w \}^T \{ c(w) \} dA$$

seklinde bagintiler elde edilir. Burada;

u : yer degistirmeyi

x : kitle kuvvetini

p : yüzeyel gkds

q : iletgenel "

p : telki "

c = elastik yataklanma katsayisini gostermektedir.

74) Sonlu Elemanlar Yöntemi ile Stabilité Hesabı

Kesit kalınlıklarının sisten boyutlarına göre çok küçük olması halinde yörünün narinliği söz konusu olur. Narinliğin artması büyük deplasmanlara sebep olacağından, birinci mertebe teorisi yerine, ikinci mertebe teorisiyle hesap gerekmektedir. Dolayısıyla sistemin stabilité hesabı önem kazanmaktadır. Stabilité hesabında dergi konumu bellidir. Dergi durumunun kararlı olup olmadığı araştırılır ve kararlı dergi konumunu oluşturan şartlar incelenir.

Stabilité hesabında yapılan kabuller;

a) Dergi çıkışları için değışiklik sisten içinde yapıldığı için elastik stabilité etkisi olmayan bir teorik yük-ye değışiklik arasında orantı bir orantılilik bulunmaz. Bu nedenle etkiler dış yüklerin oranındaki oran sabit kalacak şekilde arttırlar kabul edilir.

b) Sisten lineer olduğundan süperpozisyon kuralı, Betti-Karşılık Teoremi geçerli değışir.

c) Mamber kesit kesiklerinin büyük değışiklik için göz önüne alınarak yalnızca birinci [No] ikinci mertebe etkileri hesaba katılarak, eğilme ve burulma momentlerinin ikinci mertebe etkileri ihmal edilecektir.

Toplam Potansiyel Enerji Değişikliği;

$$\Pi = U + W$$

selül değışiklik enerji U ; eğilme ve mamber selül değışiklik enerji olarak ifade alınır,

$$\Pi = U_m + U_e + W \text{ olur.}$$

$$U_M = \frac{C}{2} \iint (\epsilon_x^2 + \epsilon_y^2 + 2\nu \epsilon_x \epsilon_y + \frac{1-\nu}{2} \gamma_{xy}^2) dx dy$$

$$U_b = \frac{D}{2} \iint (k_x^2 + k_y^2 + 2\nu k_x k_y + 2(1-\nu) k_{xy}^2) dx dy$$

$$U = \frac{C}{2} \iint (\epsilon_x^2 + \epsilon_y^2 + 2\nu \epsilon_x \epsilon_y + \frac{1-\nu}{2} \gamma_{xy}^2) dx dy + \frac{D}{2} \iint (k_x^2 + k_y^2 + 2\nu k_x k_y + 2(1-\nu) k_{xy}^2) dx dy$$

Uzayal bir sistemin yüzeyi üzerine etkiyen yüklerin x, y, z doğrultularındaki bileşenleri P_x, P_y, P_z ve elemanın ortalama yüzeyi üzerindeki bir noktanın yer değiştirmeye bileşenleri u, v ve w ve tatbik edilen yüklerin potansiyel enerjisi işin bağıntısı;

$$V = - \iint (P_x \cdot u + P_y \cdot v + P_z \cdot w) dx dy \text{ olur}$$

Sistem konservatif ve hız kuvvetlerin potansiyel enerjisi negatiflik bu bağıntıda yer değiştirmeler ve kuvvetler aynı yöndedir. Yukarıda verdiğimiz membran ve eğilme şekil değiştirmeye enerji bağıntıları ise, elastik bir dairesel kabuğa aittir olup ϵ_x, ϵ_y ve γ_{xy} kabuk orta yüzeyinin uzama ve kayma şekil değiştirmeye bileşenleri; k_x, k_y ve k_{xy} de ortalama yüzeyin eğrilik değişimi ve burulma eğriligidir. Bunlara kabuğun depremasyon - deplaman bağıntıları veya kinematik münasebetleri veya uyumluluk denklemleri deriz. Ortalama yüzeyin u, v, w deplaman bileşenlerinin fonksiyonlarıdır.

Navozhilov teorisine göre, bariik olmayan dairesel silindiklerin nonlineer deformasyon - deplaman bağıntıları ortalama yüzeyin u, v, w deplaman bileşenleri açısından ifadeleri aşağıdaki şekildedir. Burada r silindirin yarıçapını göstermektedir.

$$\varepsilon_x = \frac{\partial u}{\partial x} + \frac{1}{2} \beta_x^2 = \frac{\partial u}{\partial x} + \frac{1}{2} \left(-\frac{\partial w}{\partial x} \right)^2$$

$$\varepsilon_y = \frac{\partial u}{\partial y} + \frac{w}{r} + \frac{1}{2} \beta_y^2 = \frac{\partial v}{\partial y} + \frac{w}{r} + \frac{1}{2} \left(\frac{v}{r} - \frac{\partial w}{\partial y} \right)^2$$

$$\delta_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} + \beta_x \beta_y = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \left(-\frac{\partial w}{\partial x} \right) \left(\frac{v}{r} - \frac{\partial w}{\partial y} \right)$$

$$k_x = -\frac{\partial^2 w}{\partial x^2}, \quad k_y = \frac{\partial v}{r \partial y} - \frac{\partial^2 w}{\partial y^2}, \quad 2k_{xy} = \frac{2\partial v}{r \partial x} - 2\frac{\partial^2 w}{\partial x \partial y}$$

$$\beta_x = -\frac{\partial w}{\partial x} \quad \beta_y = \frac{v}{r} - \frac{\partial w}{\partial y}$$

7.4.1) Stabilitéde Minimum Potansiyel Enerji Yöntemi

Gerekli deger konumuna yaklas. Ilkisi bir konum ele alirsak;

$$u \rightarrow u_0 + u_1$$

$$v \rightarrow v_0 + v_1$$

$$w \rightarrow w_0 + w_1 \text{ olacaktir}$$

Burada u_0, v_0 ve w_0 deger konumuna karshi gelen yer degistirme-lerdir. u_1, v_1 ve w_1 de ilkin konumu ekle etmek tam verilen sorunu kusuk yaklasik aciklayabilir. u, v ve w de ilkin konuma karshi gelen yer degistirmelerdir. Ilke birim selul degistirme bagintilari;

$$\varepsilon_x = \frac{\partial u}{\partial x} + \frac{1}{2} \left(\frac{\partial w}{\partial x} \right)^2$$

$$\varepsilon_x^2 = \left(\frac{\partial u}{\partial x} \right)^2 + \frac{\partial u}{\partial x} \left(\frac{\partial w}{\partial x} \right)^2 + \frac{1}{4} \left(\frac{\partial w}{\partial x} \right)^4$$

Potansiyel enerji ile ilgili degisimi de;

$$\Delta u = \delta u + \frac{1}{2!} \delta^2 u + \frac{1}{3!} \delta^3 u \dots \text{seklindedir.}$$

Burada $\frac{1}{2!} \delta^2 u$ degisi; ($v_0 = 0$ tam)

$$\frac{1}{2!} \delta^2 (\varepsilon_x^2) = \left(\frac{\partial u}{\partial x} \right)^2 + \frac{\partial u_0}{\partial x} \left(\frac{\partial w_1}{\partial x} \right)^2 \text{ dir.}$$

$\epsilon_y, \delta_{xy}, k_x, k_y$ ve k_{xy} için benzer ifadeleri yazarsak membranın eğilme selül deformasyon enerjisinin İkinci varyasyonu aşağıdaki şekli alır;

$$\frac{1}{2!} \delta^2 U_m = \frac{C}{2} \iint \left\{ (\delta \epsilon_x)^2 + (\delta \epsilon_y)^2 + 2\nu (\delta \epsilon_x)(\delta \epsilon_y) + \frac{1-\nu}{2} (\delta \delta_{xy})^2 \right. \\ \left. + [(\epsilon_{x0} + \nu \epsilon_{y0})(\delta^2 \epsilon_x) + (\epsilon_{y0} + \nu \epsilon_{x0})(\delta^2 \epsilon_y) + \frac{1-\nu}{2} \delta_{xy0}(\delta^2 \delta_{xy})] \right\} dx dy$$

$$\frac{1}{2!} \delta^2 U_b = \frac{D}{2} \iint \left[(\delta k_x)^2 + (\delta k_y)^2 + 2\nu (\delta k_x)(\delta k_y) + 2(1-\nu)(\delta k_{xy})^2 \right] dx dy$$

Bu durumda toplam enerjinin İkinci varyasyonu;

$$\frac{1}{2!} \delta^2 U = \frac{C}{2} \iint (\epsilon_{x1}^2 + \epsilon_{y1}^2 + 2\nu \epsilon_{x1} \epsilon_{y1} + \frac{1-\nu}{2} \delta_{xy1}^2) dx dy \\ + \frac{1}{2} \iint (N_{x0} \beta_{x1}^2 + N_{y0} \beta_{y1}^2 + 2N_{xy0} \beta_{x1} \beta_{y1}) dx dy \\ + \frac{D}{2} \iint [k_{x1}^2 + k_{y1}^2 + 2\nu k_{x1} k_{y1} + 2(1-\nu)k_{xy1}^2] dx dy$$

Bu bağıntıdaki deplasman-deformasyon ilişkisi ise;

$$\epsilon_{x1} = \frac{\partial u_1}{\partial x} \quad \epsilon_{y1} = \frac{\partial v_1}{\partial y} + \frac{w_1}{r} \quad \delta_{xy1} = \frac{\partial u_1}{\partial y} + \frac{\partial v_1}{\partial x} \\ k_{x1} = -\frac{\partial^2 w_1}{\partial x^2} \quad k_{y1} = \frac{\partial v_1}{r \partial y} - \frac{\partial^2 w_1}{\partial y^2} \quad 2k_{xy1} = \frac{2\partial v_1}{r \partial x} - \frac{2\partial^2 w_1}{\partial x \partial y} \\ \beta_{x1} = -\frac{\partial w_1}{\partial x} \quad \beta_{y1} = \frac{v_1}{r} - \frac{\partial w_1}{\partial y}$$

Buradaki N_{x0} , N_{y0} ve N_{xy0} değerleri membran dengesizliklerinden elde edilen membran kuvvetlerdir.

$$N_{x0} = C (\epsilon_{x0} + \nu \epsilon_{y0})$$

$$N_{y0} = C (\epsilon_{y0} + \nu \epsilon_{x0})$$

$$N_{xy0} = C \frac{1-\nu}{2} \delta_{xy0}$$

Die yüklerin potansiyel enerjilerini veren bağıntı;

$$V = - \iint (P_x u + P_y v + P_z w) dx dy \quad \text{di. Bu bağıntının}$$

İkinci varyasyonunda İkinci dereceden terimler ihmal etme-
diğinden $\delta^2 W = 0$ olur. Bu durumda;

$$\delta^2 \Pi = \delta^2 U_m + \delta^2 U_b \quad \text{olur.}$$

$\frac{1}{2!} \delta^2 U$ bağıntısındaki (1) indisi atılıp matris formda yazılırsa;

$$\frac{1}{2!} \delta^2 \Pi = \frac{1}{2} \iint \{ \epsilon \}^T [C] \{ \epsilon \} dx dy + \frac{1}{2} \iint \{ \beta \}^T [N_b] \{ \beta \} dx dy$$

Bağıntı açık şekilde yazılırsa;

$$\frac{1}{2!} \delta^2 \Pi = \frac{1}{2} \iint \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ k_x \\ k_y \\ k_{xy} \end{Bmatrix} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ k_x \\ k_y \\ k_{xy} \end{Bmatrix} dx dy + \frac{1}{2} \iint \begin{Bmatrix} -\frac{\partial w}{\partial x} \\ \frac{v}{r} - \frac{\partial w}{\partial y} \end{Bmatrix} \begin{bmatrix} N_{x0} & N_{y0} \\ N_{r0} & N_{\theta} \end{bmatrix} \begin{Bmatrix} \frac{\partial w}{\partial x} \\ \frac{v}{r} - \frac{\partial w}{\partial y} \end{Bmatrix} dx dy$$

$$\{ \epsilon \} = [F] \{ a \} ; \quad \{ \beta \} = [G] \{ a \} \quad [F]; [G]: \text{Taas matrislerdir}$$

$$\{ \epsilon \} = [F] [B] \{ d \}$$

$$\{ \beta \} = [G] [B] \{ d \}$$

Bu durumda;

$$\frac{1}{2!} \delta^2 \Pi = \frac{1}{2} \{ d \}^T \left(\iint [B]^T [F]^T [C] [F] [B] dx dy + \right. \\ \left. + \iint [B]^T [G]^T [N_b] [G] [B] dx dy \right) \{ d \}$$

olacaktır.

Bağıntının kapalı çözümü;

$$\delta^2\pi = \{\delta\}^T ([K_e] + \lambda [N_e]) \{\delta\} \text{ olur.}$$

Burada; $[K_e]$ = eleman rijitlik matrisi

λ : yük parametresi

$[N_e]$: eleman geometrik yük matrisidir.

$[N_e]$ ilinci mertebe etkilerinden meydana gelen ilave yükler. Sisteme geçen çukukma matrisleri veya biriktirme metodu ile yapılarak $[K]$ sistem rijitlik matrisi ve $[N]$ sistem geometrik yük matrisi elde edilir. Elemanın deplasman parametreleri $\{\delta_{ei}\}$; sistemin deplasman parametreleri $\{\delta_s\}$ ise; bütün sistemin potansiyel enerjisinin minimizasyonu yaparsak;

$$\frac{\partial (\delta^2\pi)_{\text{sistem}}}{\partial \delta_s} = \sum \frac{\partial (\delta^2\pi)_{\text{eleman}}}{\partial \delta_{ei}} = 0 \quad (\delta = \delta_1, \delta_2, \delta_3, \dots, \delta_n)$$

elde edilir.

7.4.2) Stabilité Probleminin Çözümü

Burkulma yükü, sistemin ilk durumunu kararsız hale getiren en küçük yük parametresi olarak tanımlanabilir. Dış yüklerden oluşan mabean kesit tesirleri bir yük parametresine bağlı olarak ifade edilebilir. ilinci' mertebe hesaplarında süperpozisyon prensibi geçerli olmadığı için yük belli bir değerin katları şeklinde gösterebiliriz. $[N_0] = \lambda [N_0]$ gibi. Burada λ yük parametresidir. Stabilité denklemi;

$$([K] + \lambda [N]) \{\delta_s\} = 0 \text{ şeklinde dir.}$$

Bu denklem takımına problemin sınır şartları uygulanır. Sistemin ilk konumunun karaesiz hale gelmesi ve $\{\Delta_s\} = 0$ farklı karaesiz başka bir dege konumunun bulunması için yukarıdaki denklem takımının katsayıları (Δ) determinantını sıfır olmasını gerektirir. (Δ) determinantını sıfır yapan yük P_{cr} şeklinde bulunur.

1) Sıfırdan başlayarak P 'ye artan degeleler verilir ve (Δ) determinantı hesaplanır. Pozitiften negatife geçtiği bölgede bu aralıklar sıklaştırılarak (Δ) determinantını sıfır yapan P_{cr} değeri bulunmuş olur.



2) Ardızık yaklaşım yöntemi ile de P_{cr} yük hesaplanır.

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## 7.5) Dinamik Düz Etkiler Halinde Sorlu Elemanlar Yöntemi

İnşaat mühendisliğinde deprem, darbe kuvvetleri, malzeme titreşim kuvvetleri ve ani patlama kuvvetleri zamanla değişen kuvvetlerdir. Düz etkilerin zamana bağlı olarak sorlu bir hâlde değişmesi halinde atalet kuvvetlerinin de gözönüne alınması gerekmektedir. Sisteme etkileyen kuvvetler zamanın bir fonksiyonu ise, bu kuvvetlerin etkilediği yapıların tepkisi'nde zamanın bir fonksiyonudur ve zamana bağlı olarak meydana gelecek yer değiştirmelerin incelenmesi, atalet kuvvetlerini meydana getirebilir. Bu durumda sistem şu tip yörün etkisi altında düşünülebilir; harekete neden olan dış yük ve hareketin incelenmesine karşılıklı olarak atalet kuvvetleri. Yapı özellikleri ve etkileyen kuvvetlerden yola çıkılarak, titreşim sisteminde art mekanik bir yay-kütle modeli oluşturulur ve kütlelere art titreşim denklemleri kurulurak çözüme ulaşırlar.

Eleman üzerine yayılı kütleler doğan noktasalara toplanabilir. Eleman boyutlarının büyük olması halinde gerçek sonucu iyi yaklaştırmak için nokta kütle sayısını arttırmak ve mümkün olduğu kadar yayılı kütle haline yaklaştırmak gereklidir. Yüzeyel kütlelerden doğan atalet kuvvetlerinin gerçek yayılılıkları ile hesaba katılması uygundur. Yayılı kütlelerin ortalama yüzeye indirildiği kabulü yapılacaktır. Zamana bağlı yer değiştirmeye, ortalama yüzeyin yer değiştirmesi'ni cinsinden belirlendiğinde, yayılı atalet kuvvetleri yalnız yer değiştirmeye biterkenlere bağlı olacaktır. Aynı biterkenlere bağlı olarak ayrıca yüzeyel yayılı momentler meydana gelmeyecektir. Herhangi bir  $t$  anındaki



yüzeyel elemanın hareket denklemi D'Alembert prensibine göre; sisteme etki eden atalet kuvvetleri, sönüm etkileri ve yay kuvvetleriyle beraber dış kuvvetler derjce halinde olmalıdır. Bilindiği gibi sönüm tesiri de,  $\mu$  sisteme katılan olmak üzere viskoz bir ortamda hızla orantılı bir sönüm kuvveti gibi düşünülmektedir. Birim alana gelen kütle yoğunluğu  $\rho$  olduğuna göre; titreşime maruz kütlenin etresi altında kaldığı atalet kuvveti, sönüm kuvveti ve yay kuvvetlerinin ifade dış yükler gibi düşünülmeleriyle vektörel iz teoremi uygulanabilir.

$\delta U$   $T_0$  kuvvetleri,  $\delta W$  dış kuvvetlerin işi ise;

$$\delta U = \int \{\delta \varepsilon\}^T \sigma dV$$

$$\{\sigma\} = [D] (\{\varepsilon\} - \{\varepsilon_0\}) + \{\sigma_0\} - \alpha \cdot \Delta t \{\dot{\sigma}_T\}$$

$\varepsilon_0$  başlangıç selül değişimliliğini ve  $\sigma_0$  başlangıç gerilmesini ihmal edersek;

$$\{\sigma\} = [D] \{\varepsilon\} - \alpha \cdot \Delta t \{\dot{\sigma}_T\} \quad \text{elde edilir.}$$

$$\delta U = \int_V \{\delta \varepsilon\}_i^T [D] \{\varepsilon\} dV - \int_V \{\delta \varepsilon\}_i^T \alpha \cdot \Delta t \{\dot{\sigma}_T\} dV$$

$$\delta W = - \rho \int_V \{\delta u\}_i^T \{\ddot{u}\} dV - \mu \int_V \{\delta u\}_i^T \{\ddot{u}\} dV - c \int_A \{\delta u\}_i \{u\} dA + \int_A \{\delta u\}_i^T \{p\} dA$$

Tabiri durum gerek yükler,  $\ddot{u}$  birim durumu ise vektörel deplasman ve vektörel selül değişimliliği durumu olarak alınıp; vektörel iz teoremi uygulanca dış kuvvetler,  $T_0$  kuvvetlerin eşitliğini yazarak;

$\delta U = \delta W$  dir. Virial yasa ve self dengelenmesi d

$$\delta u = [N] \{ \delta d \}$$

$$\delta \dot{u} = [N] \{ \delta \dot{d} \}$$

$$\delta \ddot{u} = [N] \{ \delta \ddot{d} \}$$

$\delta \varepsilon = [G] \{ \delta d \} = [\Delta N] \{ \delta d \}$  şeklinde. Bu durumda;  
 $\delta u = \delta W$

$$\begin{aligned} \{ \delta d \}^T & \left[ \int_V [\Delta N]^T [\sigma] [\Delta N] dV \right] \{ d \} - \{ \delta d \}^T [\alpha \Delta t] \int_V [\Delta N]^T \{ \sigma_T \} dV = \\ & - \{ \delta d \}^T \left[ \rho \int_V [N]^T [N] dV \right] \{ \ddot{d} \} - \{ \delta d \}^T \left[ \mu \int_V [N]^T [N] dV \right] \{ \ddot{d} \} \\ & - \{ \delta d \}^T \left[ c \int_A [N(w)]^T [N(w)] dA \right] \{ d \} + \{ \delta d \}^T \int_A [N]^T p(t) dA \end{aligned}$$

Buradan ;

$$[k_e] = \int_V [\Delta N]^T [\sigma] [\Delta N] dV \quad : \text{element rijitlik matrisi}$$

$$[c_e] = \mu \int_V [N]^T [N] dV \quad : \text{" sönüm "}$$

$$[m_e] = \rho \int_V [N]^T [N] dV \quad : \text{" kütle "}$$

$$[s_e] = c \int_A [N(w)]^T [N(w)] dA \quad : \text{" elastik yataklama matrisi "}$$

$$\{ q_e(t) \}_T = \alpha \Delta t \int_V [\Delta N]^T \{ \sigma_T \} dV \quad : \text{Elementin zamana bağlı sıcaklık değişimi}$$

$$\{ q_e(t) \}_0 = \int_A [N]^T p(t) dA \quad : \text{" " " dış yük}$$

kuatlımaları yavaşca bağlı;

$$[k_e] \{ d(t) \} + [c_e] \{ \dot{d}(t) \} + [m_e] \{ \ddot{d}(t) \} + [s_e] \{ d(t) \} = \{ q_e(t) \}_0 + \{ q_e(t) \}_T$$

halini alır. Buradan sisteme geçiş;

$[K]\{d(t)\} + [C]\{\dot{d}(t)\} + [M]\{\ddot{d}(t)\} + [S]\{d(t)\} = \{Q(t)\} + \{Q(t)\}_0$   
halini alır.

### 7.5.1) Sönümsüz Serbest Titreşim

Sönümsüz serbet titreşimde sönüm matrisi, zamanı bağıdır ve sıfır olacaktır. Elastik yataklama etkileri de yoksa yukarıdaki bağıntı;

$$[K]\{d(t)\} + [M]\{\ddot{d}(t)\} = 0 \text{ olur.}$$

Titreşim basit harmonik hareket olduğundan bu homojen diferansiyel denklemin çözümü;

$$\{d\} = \{\bar{d}\} \sin(\omega t + \theta) \text{ 'dır.}$$

Burada  $\{\bar{d}\}$  kütlelerin öteleme genlikleridir. Bu ifadenin türevi alınıp bağıntıdaki yerine yazılırsa titreşim bağıntısı;

$$([K] - \omega^2 [M])\{\bar{d}\} = 0 \text{ şeklini alır. } \{\bar{d}\} \neq 0$$

çözümü için katsayılar determinantı sıfır olmalıdır.

$$|[K] - \omega^2 [M]| = 0$$

Bu determinanta frekans determinantı denir. Frekans determinantını sıfır yapan  $\omega$  değerlerine özel frekans denir.  $\lambda = \omega^2$  yazılır ve bağıntı  $[M]^{-1}$  ile çarpılırsa;

$$|[K][M]^{-1} - \lambda [M][M]^{-1}| = 0$$

$$[A] - \lambda [I] = 0 \text{ olur.}$$

Bu durumda problem özdeğer problemine dönüşür ve

$n$ -merkebeden sabit kabaylı bir polinomun köklerini bulun  
maına indirgenmiş olur.  $n$  adet kök olan bu denklemden  
 $n$  adet  $\bar{\omega}_i$ ; dolayısıyla  $\omega_i$  özel doğal frekansları bulun  
Her  $\omega_i$  özel değerine karşılık  $([K] - \omega^2[M])\{\bar{d}\} = 0$   
homojen denkleminde  $\{\bar{d}\}_i$  değerleri bulunur.  $\{\bar{d}\}_i$   
vektörüne,  $\omega_i$  serbest titreşim frekansına karşı gelen  
serbest titreşim mod zekî denir.  $\{\bar{d}\}_i$  vektörü yer de-  
ğiştirmelere karşı gelmektedir.  $\{\bar{d}\}_i$  kolon matrisi yan yana  
yerleştirilerek kare  $[\Phi]$  modal matrisi elde edilir.

$$[\Phi] = [\{\bar{d}\}_1, \{\bar{d}\}_2, \dots, \{\bar{d}\}_n]$$

Matrisin  $\{\bar{d}\}_{ij}$  elemanı;  $j$ -modda,  $i$ . serbestlik derecesi doğrudan-  
sındaki yer değıştirmeyi göstermektedir.

$\{d\} = \{\bar{d}\}_1 D_1 + \{\bar{d}\}_2 D_2 + \dots + \{\bar{d}\}_n D_n \rightarrow \{d\} = [\Phi]\{D\}$   
Buradaki  $D_i$  değerlerine genelleştirmiş koordinat denir.  $\{\bar{d}\}_i$  ve  
 $\{\bar{d}\}_i$  mod vektörleri hem kütle matrisine, hem de rijitlik matrisine  
göre ortogonaldır.

### 7.5.2) Sönümlü Zorlanmış Titreşim

$$[K]\{d(t)\} + [M]\{\ddot{d}(t)\} = \{Q(t)\}_0$$

### 7.5.3) Sönümlü Serbest Titreşim

$$[K]\{d(t)\} + [C]\{\dot{d}(t)\} + [M]\{\ddot{d}(t)\} = 0$$

### 7.5.4) Sönümlü Zorlanmış Titreşim

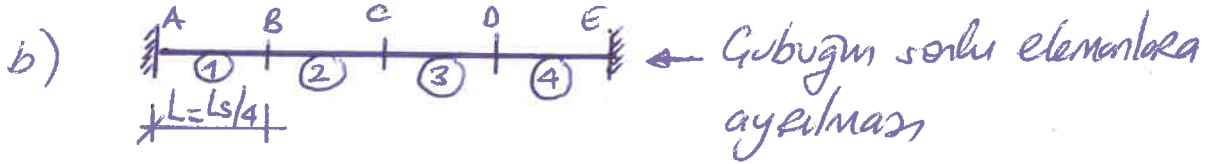
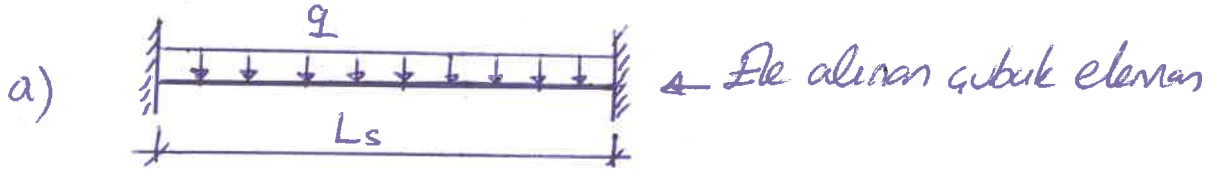
$$[K]\{d(t)\} + [C]\{\dot{d}(t)\} + [M]\{\ddot{d}(t)\} = \{Q(t)\}_0$$

## BÖLÜM V

### 9) SONLU ELEMANLAR YÖNTEMİ İLE ÇUBUK HESABI

#### 9.1) Çubuk Eğilme Hesabı

Çubuk eğilme hesabı için  $L_s$  uzunluğunda, düzgün yaygınlı yükle yüklü bir elemanı düşünelim ve bu elemanı 4 parçaya bölelim.



e)  $\{d\} = \begin{Bmatrix} w_A \\ \theta_A \\ w_B \\ \theta_B \end{Bmatrix} = \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \end{Bmatrix}$  ← Elemanın düğüm noktaları yer değiştirmeye parametreleri (matrisi)



g)  $\{D\} = \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} = \begin{Bmatrix} w_A \\ \theta_A \\ w_B \\ \theta_B \\ w_C \\ \theta_C \end{Bmatrix}$  ← Sistem yer değiştirmeye matrisi



### 9.1.1) Deplasman Fonksiyonlarının Seçimi

Deplasman fonksiyonları rijit cisim hareketi ve sabit deformasyon şartını sağlayacak şekilde seçilmelidir. Koordinat eksenini değiştirilince çözüm feali olmamalıdır. Bunun için deplasman fonksiyonu ya tam polinom veya tabii koordinatların fonksiyonu şeklinde olmalıdır. Ayrıca 1 ve 2 kuvvetlerin izinde türevlerde de sürekli olmalıdır. Bu çalışmada deplasman fonksiyonu polinom olarak seçilmiştir. Elemanda bilinmeyen dört deplasman parametresi olduğu için, dört bilinmeyenli deplasman fonksiyonu seçilmiştir.

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$
$$w = [\phi(x)] \{a\} = [1, x, x^2, x^3] \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{Bmatrix}$$

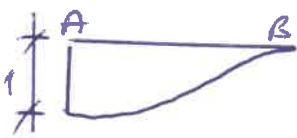
### 9.1.2) Şekil Fonksiyonlarının Tayini

Şekil fonksiyonlarını 1ki şekilde hesaplamak mümkündür. Birincisi çubuk uç deplasmanlarının hesabı ile, ikincisi de bağ matrisleri yardımıyla.

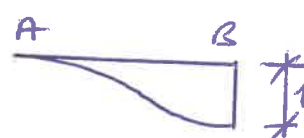
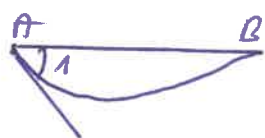
a) Çubuk uç deplasmanları yardımı ile şekil fonksiyonlarının tayini :

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

$$\theta = \frac{dw}{dx} = a_2 + 2a_3 x + 3a_4 x^2$$



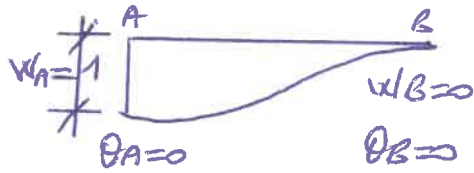
A ucunun çökmesi ve dönmesi



B ucunun çökmesi ve dönmesi



-  $N_1$  şekil fonksiyonunun tayini ( $w_A=1, w_B=\theta_A=\theta_B=0$ )



Koordinatlar  $A(0,0); B(L,0)$

$$w = a_1 + a_2x + a_3x^2 + a_4x^3$$

$$\theta = \frac{\partial w}{\partial x} = 0 + a_2 + 2a_3x + 3a_4x^2$$

A ve B ucunun koordinat değerlerini  $w_A, \theta_A, w_B$  ve  $\theta_B$  bağıştlarında yerleere yazarsak;

$$w_A = 1 = a_1 + 0 + 0 + 0$$

$$\theta_A = 0 = 0 + a_2 + 0 + 0$$

$$w_B = 0 = a_1 + a_2L + a_3L^2 + a_4L^3$$

$$\theta_B = 0 = 0 + a_2 + 2a_3L + 3a_4L^2$$

Bu değerlere göre;

$$a_1 = 1; a_3 = -\frac{3}{L^2}$$

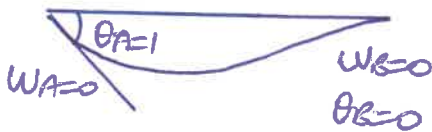
$$a_2 = 0; a_4 = \frac{2}{L^3}$$

bulunur. Bu durumda da;

$N_1 = 1 - \frac{3}{L^2}x^2 + \frac{2}{L^3}x^3$  olarak  $N_1$  şekil fonksiyonu bulunur.

$$\xi = \frac{x}{L} \text{ denilirse } \rightarrow N_1 = 1 - 3\xi^2 + 2\xi^3 \text{ olur.}$$

-  $N_2$  şekil fonksiyonunun tayini ( $\theta_A=1, w_A=w_B=\theta_B=0$ )



$$w_A = 0 = a_1 + 0 + 0 + 0$$

$$\theta_A = 1 = 0 + a_2 + 0 + 0$$

$$w_B = 0 = a_1 + a_2L + a_3L^2 + a_4L^3$$

$$\theta_B = 0 = 0 + a_2 + 2a_3L + 3a_4L^2$$

Bu gözünden;

$$a_1 = 0; a_2 = 1$$

$$a_3 = -\frac{2}{L}; a_4 = \frac{1}{L^2}$$

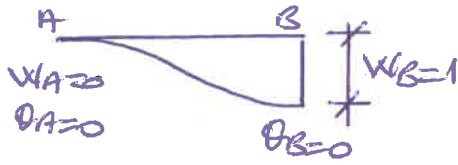
elde edilir.

Bu durumda  $N_2$  şekil fonksiyonu;

$$N_2 = L \left( \frac{x}{L} - 2\frac{x^2}{L^2} + \frac{x^3}{L^3} \right) \text{ olarak bulunur. } \xi = \frac{x}{L} \text{ ise}$$

$$N_2 = L \left( \xi - 2\xi^2 + \xi^3 \right) \text{ olur}$$

-  $N_3$  şekli fonksiyonunun tayini ( $w_B=1, w_A=\theta_A=\theta_B=0$ )

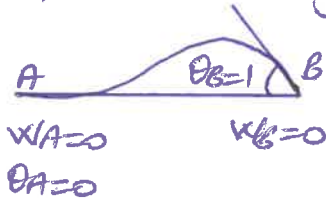


$$\left. \begin{aligned} w_A=0 &= a_1 + 0 + 0 + 0 \\ \theta_A=0 &= 0 + a_2 + 0 + 0 \\ w_B=1 &= a_1 + a_2 L + a_3 L^2 + a_4 L^3 \\ \theta_B=0 &= 0 + a_2 + 2a_3 L + 3a_4 L^2 \end{aligned} \right\} \begin{aligned} &\text{Bu gözünden;} \\ &a_1=0; a_2=0 \\ &a_3=\frac{3}{L^2}; a_4=-\frac{2}{L^3} \end{aligned}$$

$$N_3 = 3 \frac{x^2}{L^2} - 2 \frac{x^3}{L^3}$$

$$N_3 = 3 \xi^2 - 2 \xi^3$$

-  $N_4$  şekli fonksiyonunun tayini ( $\theta_B=1, w_A=w_B=\theta_A=0$ )



$$\left. \begin{aligned} w_A=0 &= a_1 + 0 + 0 + 0 \\ \theta_A=0 &= 0 + a_2 + 0 + 0 \\ w_B=0 &= a_1 + a_2 L + a_3 L^2 + a_4 L^3 \\ \theta_B=1 &= 0 + a_2 + 2a_3 L + 3a_4 L^2 \end{aligned} \right\} \begin{aligned} &a_1=0; a_2=0 \\ &a_3=-\frac{1}{L}; a_4=\frac{1}{L^2} \end{aligned}$$

$$N_4 = L \left( -\frac{x^2}{L^2} + \frac{x^3}{L^3} \right)$$

$$N_4 = L \left( -\xi^2 + \xi^3 \right)$$

b) Bağ matrisi yardımıyla şekli fonksiyonların tayini

Eleman düğüm noktaları deplasmanları  $\{d\}$  ile polinom sabitleri  $\{a\}$  arasındaki bağı veren  $[A]$  bağ matrisi de, elemanın düğüm noktaları deplasman parametrelerinin  $\phi(x)$  ve  $\phi(x)$ 'in türevleri açısından yazılmış bileşenlerinde düğüm noktaları koordinatlarının yerine konulmasıyla bulunur.

$$\{d\} = [A]\{a\}$$

$$\{a\} = [A]^{-1}\{d\}$$

$$[A]^{-1} = [B] \text{ dersek;}$$

$$\{a\} = [B]\{d\}$$

$w = [\phi(x)]\{a\} = [\phi(x)][B]\{d\}$  Bu ifadedeki  $[\phi(x)][B]$ 'ye  $[N]$  deniyoruz denir.

$$[N] = [\phi(x)][B]$$

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

$$\theta = \frac{dw}{dx} = a_2 + 2a_3 x + 3a_4 x^2$$

$A(0,0)$  ve  $B(L,0)$  düğüm noktaları koordinatları ve bu koordinatlar şu bağışlarla yazılabilir;

$$\left| \begin{array}{l} w_A = a_1 + a_2 x + a_3 x^2 + a_4 x^3 \\ \theta_A = a_2 + 2a_3 x + 3a_4 x^2 \end{array} \right|_{x=0}$$

$$\left| \begin{array}{l} w_B = a_1 + a_2 x + a_3 x^2 + a_4 x^3 \\ \theta_B = a_2 + 2a_3 x + 3a_4 x^2 \end{array} \right|_{x=L}$$

$$[A] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 1 & L & L^2 & L^3 \\ 0 & 1 & 2L & 3L^2 \end{bmatrix}$$

matrisi elde edilebilir. Buradan  $[A]^{-1} = [B]$  matrisi hesaplanır.

$$[B] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -3/L^2 & -2/L & 3/L^2 & -1/L \\ 2/L^3 & 1/L^2 & -2/L^3 & 1/L^2 \end{bmatrix}$$

$$w = [\phi(x)]\{a\} = [\phi(x)][B]\{d\}$$

$$[N_1, N_2, N_3, N_4] = \underbrace{[1, x, x^2, x^3]}_{[N]} [B]$$

$[A]^{-1} = [B]$  matrisinin matlab programı ile hesabı

```
>> syms L
>> syms A
>> syms B
>> syms N1
>> syms N2
>> syms N3
>> syms N4
>> syms x
>> syms x1
>> [A]=[1 0 0 0;0 1 0 0;1 L L^2 L^3;0 1 2*L 3*L^2]
```

A =

```
[ 1, 0, 0, 0]
[ 0, 1, 0, 0]
[ 1, L, L^2, L^3]
[ 0, 1, 2*L, 3*L^2]
```

```
>> [B]=[A]^(-1)
```

B =

```
[ 1, 0, 0, 0]
[ 0, 1, 0, 0]
[ -3/L^2, -2/L, 3/L^2, -1/L]
[ 2/L^3, 1/L^2, -2/L^3, 1/L^2]
```

```
>> [x1]=[N1 N2 N3 N4]
```

x1 =

```
[ N1, N2, N3, N4]
```

```
>> [x1]=[1 x x^2 x^3]*[B]
```

x1 =

```
[ (2*x^3)/L^3 - (3*x^2)/L^2 + 1, x - (2*x^2)/L + x^3/L^2, (3*x^2)/L^2 - (2*x^3)/L^3,
x^3/L^2 - x^2/L]
```

```
>>
```

$[x_1] = [N_1, N_2, N_3, N_4] * [B]$

$$[N_1, N_2, N_3, N_4] = [1, x, x^2, x^3] \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -3/L^2 & -2/L & 3/L^2 & -1/L \\ 2/L^3 & 1/L^2 & -2/L^3 & 1/L^2 \end{bmatrix}$$

$$N_1 = 1 - \frac{3}{L^2} x^2 + \frac{2}{L^3} x^3$$

$$N_2 = x - \frac{2}{L} x^2 + \frac{1}{L^2} x^3 = L \left( \frac{x}{L} - \frac{2}{L^2} x^2 + \frac{1}{L^3} x^3 \right)$$

$$N_3 = \frac{3}{L^2} x^2 - \frac{2}{L^3} x^3$$

$$N_4 = -\frac{1}{L} x^2 + \frac{1}{L^2} x^3 = L \left( -\frac{x^2}{L^2} + \frac{x^3}{L^3} \right)$$

$$\xi = \frac{x}{L} \quad dx;$$

$$N_1 = 1 - 3\xi^2 + 2\xi^3$$

$$N_2 = L \left( \xi - 2\xi^2 + \xi^3 \right)$$

$$N_3 = 3\xi^2 - 2\xi^3$$

$$N_4 = L \left( -\xi^2 + \xi^3 \right)$$

### 9.1.3) Eleman Rijitlik Matrisinin Bulunması

Toplam potansiyel enerjinin birinci' varyasyonu önceden de verildiği gibi;

$$\delta \Pi^e = \delta U^e + \delta W^e$$

$$\delta \Pi^e = \int_0^L [EI (w'')^2 - q \cdot w] dx \quad \text{şeklinde dir.}$$

Burada;

$$\delta U^e = \int_0^L [E(w'')^2] dx = \int_0^L \{\varepsilon\}^T [D] \{\varepsilon\} dx = \{d\}^T [k_e] \{d\}$$

$$\delta W^e = - \int_0^L q w dx = - \int_0^L [N_1, N_2, N_3, N_4]^T q dx = - \{d\}^T [q_e]$$

$$\delta \Pi^e = \{d\}^T ([k_e] \{d\} - [q_e])$$

Burada  $[k_e]$  eleman rijitlik matrisi,  $[q_e]$  dış yük matrisidir. Eleman rijitlik matrisi hesaplamak 2 yoldan uygulanır.

a) Integral matrisleri yardımıyla

b) Seçil fonksiyonlar "

### a) Integral Matrisleriyle Rijitlik Matrisinin Hesabı

$$\varepsilon = - \frac{\partial^2 w}{\partial x^2} = -w''$$

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

$$w' = a_2 + 2a_3 x + 3a_4 x^2$$

$$-w'' = -2a_3 - 6a_4 x$$

$\{\varepsilon\} = [F] \{a\}$  Buradaki  $F$  matrisi türev matrisidir.

$$\{\varepsilon\} = [0, 0, -2, -6x] \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{Bmatrix}$$

$$\{\varepsilon\} = [F][B] \{d\}$$

$$[F] = [0, 0, -2, -6x]$$

$$[k_e] = \int_0^L \{\varepsilon\}^T [D] \{\varepsilon\} dx$$

$$[k_e] = \int_0^L [B]^T [F]^T [E] [F] [B] dx \quad \text{olur.}$$

$[H] = EI \int_0^L [F]^T [F] dx$  integral matrisidir.



$$[k_e] = EI \int_0^L \begin{bmatrix} 1 & 0 & -3/L^2 & 2/L^3 \\ 0 & 1 & -2/L & 1/L^2 \\ 0 & 0 & 3/L^2 & -2/L^3 \\ 0 & 0 & -1/L & 1/L^2 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ -2 \\ -6x \end{bmatrix} \begin{bmatrix} 0 & 0 & -2 & -6x \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -3/L^2 & -2/L & 3/L^2 & -1/L \\ 2/L^3 & 1/L^2 & -2/L^3 & 1/L^2 \end{bmatrix}$$

Hesaplamadan sonra eleman rijitlik matrisi;

$$[k_e] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \quad \text{şeklinde belirlenir.}$$

b) Selül Fonksiyonlarından Faydalanarak Eleman Rijitlik Matrisinin Hesabı

$$[k_e] = \int_0^L \{\varepsilon\}^T [D] \{\varepsilon\} dx, \quad \{\varepsilon\} = -\frac{\partial^2 w}{\partial x^2}, \quad w = \underbrace{[\phi(x)][B]}_{[N]} \{d\}$$

$$\varepsilon = -\frac{\partial^2 w}{\partial x^2} = -\frac{\partial^2 [N]}{\partial x^2} \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \end{Bmatrix}$$

$$[k_e] = \int_0^L \left\{ \left( -\frac{\partial^2 [N]}{\partial x^2} \right)^T [D] \left( -\frac{\partial^2 [N]}{\partial x^2} \right) \right\} dx$$

$$[k_e] = \int_0^L \begin{bmatrix} -N_1'' & -N_2'' & -N_3'' & -N_4'' \end{bmatrix}^T [EI] \begin{bmatrix} -N_1'' & -N_2'' & -N_3'' & -N_4'' \end{bmatrix} dx$$

$$[k_e] = EI \int_0^L \begin{bmatrix} N_1'' N_1'' & N_1'' N_2'' & N_1'' N_3'' & N_1'' N_4'' \\ N_2'' N_1'' & N_2'' N_2'' & N_2'' N_3'' & N_2'' N_4'' \\ N_3'' N_1'' & N_3'' N_2'' & N_3'' N_3'' & N_3'' N_4'' \\ N_4'' N_1'' & N_4'' N_2'' & N_4'' N_3'' & N_4'' N_4'' \end{bmatrix} dx = \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{42} & k_{43} & k_{44} \end{bmatrix}$$

$$N_1 = 1 - \frac{3x^2}{L^2} + \frac{2x^3}{L^3}$$

$$N_2 = L \left( \frac{x}{L} - \frac{2x^2}{L^2} + \frac{x^3}{L^3} \right)$$

$$N_1' = -\frac{6x}{L^2} + \frac{6x^2}{L^3}$$

$$N_2' = L \left( \frac{1}{L} - \frac{4x}{L^2} + \frac{3x^2}{L^3} \right)$$

$$N_1'' = -\frac{6}{L^2} + \frac{12x}{L^3}$$

$$N_2'' = L \left( -\frac{4}{L^2} + \frac{6x}{L^3} \right)$$

Bu durumda;

$$k_{11} = EI \int_0^L N_1'' N_1'' dx = EI \int_0^L \left( -\frac{6}{L^2} + \frac{12x}{L^3} \right) \left( -\frac{6}{L^2} + \frac{12x}{L^3} \right) dx = \frac{12EI}{L^3}$$

$$k_{12} = EI \int_0^L N_1'' N_2'' dx = EI \int_0^L \left( -\frac{6}{L^2} + \frac{12x}{L^3} \right) \left( -\frac{4}{L^2} + \frac{6x}{L^3} \right) L dx = \frac{6EI}{L^2}$$

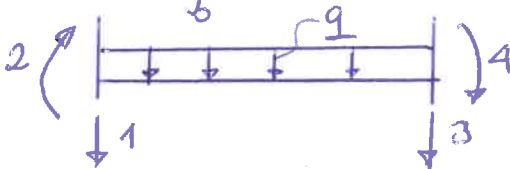
Bunun sonucunda;

$$[k_e] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \text{ elde edilir.}$$

#### 9.1.4) Eleman Yük Matrisinin Hesabı

Elemana etkiyen yayılı yükler, eşdeğer kuvvetler çevrilerek doğrudan noktasına yerel yük gibi etki ettirilir. Bu durumda yük matrisi elemanlarda;

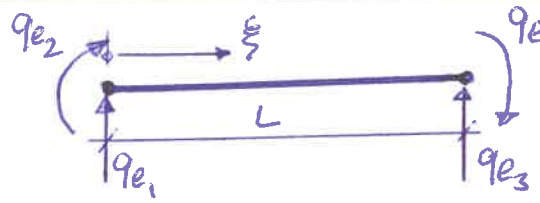
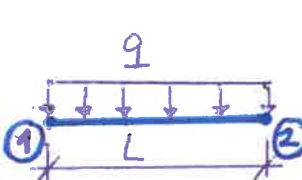
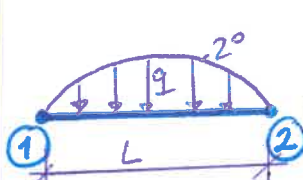
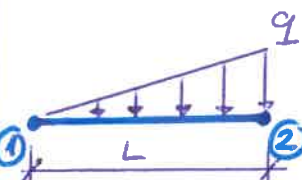
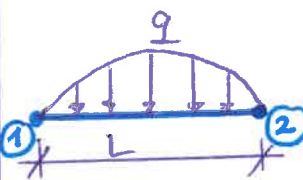
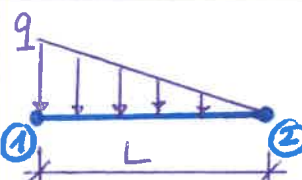
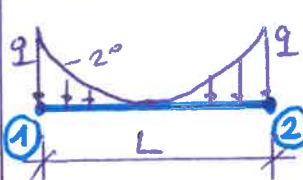
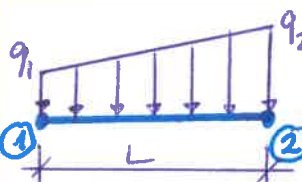
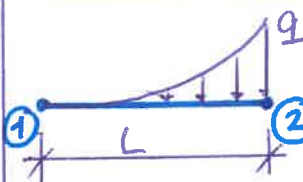
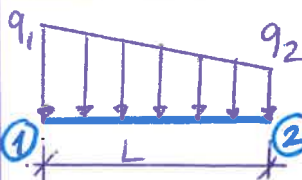
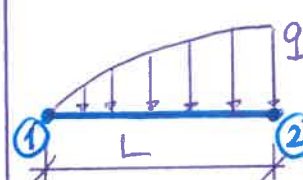
$$\{q_e\} = \int_0^L [N_1, N_2, N_3, N_4]^T q dx \text{ şeklinde olacaktır.}$$



$$\{q_e\} = \int_0^L \left[ \left( 1 - \frac{3x^2}{L^2} + \frac{2x^3}{L^3} \right) \left( L \left( \frac{x}{L} - \frac{2x^2}{L^2} + \frac{x^3}{L^3} \right) \right) \left( \frac{3x^2}{L^2} - \frac{2x^3}{L^3} \right) \left( L \left( -\frac{x^2}{L^2} + \frac{x^3}{L^3} \right) \right) \right]^T q dx$$

$$\{q_e\} = q \left[ \left( \frac{L}{2} \right), \left( \frac{L^2}{12} \right), \left( \frac{L}{2} \right), \left( -\frac{L^2}{12} \right) \right]^T$$

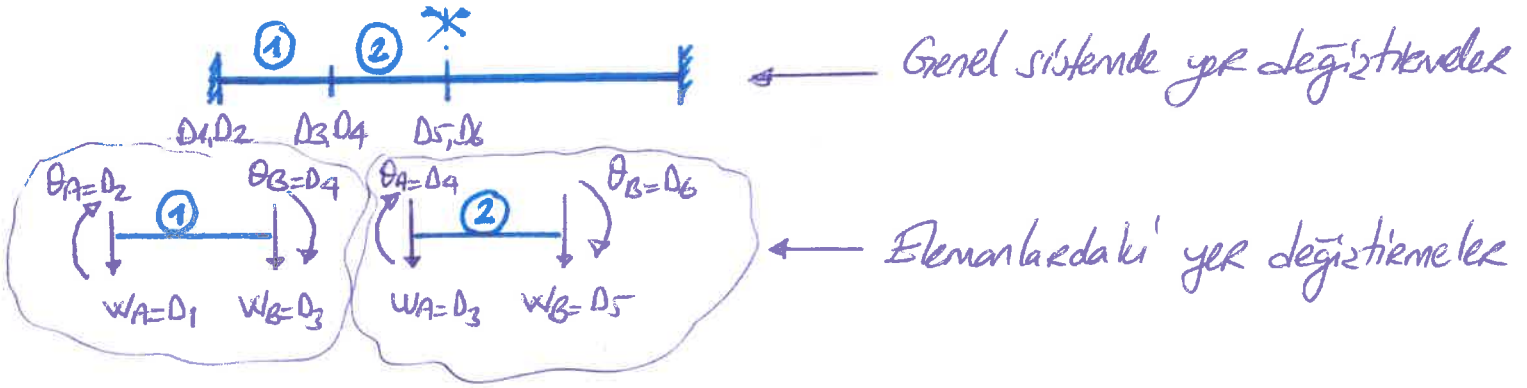
Gesitli Oluk Yüklemin Düzgün Abitlakasına Abitazılması  
 Kıdeğer Kuvvet ve Moment Değerleri

|                                                                                                                                                                                                                                                  |                                                                                                                                                                                                                                |
|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
|  $\{q_e\} = \begin{Bmatrix} q_{e1} \\ q_{e2} \\ q_{e3} \\ q_{e4} \end{Bmatrix}$                                                                                 | $[N] = \begin{Bmatrix} N_1 \\ N_2 \\ N_3 \\ N_4 \end{Bmatrix} = \begin{Bmatrix} 1 - 3\xi^2 + 2\xi^3 \\ L(-\xi + 2\xi^2 - \xi^3) \\ 3\xi^2 - 2\xi^3 \\ L(\xi^2 - \xi^3) \end{Bmatrix}$                                          |
|  $\{q_e\} = q \begin{Bmatrix} L/2 \\ L^2/12 \\ L/2 \\ -L^2/12 \end{Bmatrix}$ $q(\xi) = q$                                                                       |  $\{q_e\} = q \begin{Bmatrix} L/3 \\ L^2/15 \\ L/3 \\ -L^2/15 \end{Bmatrix}$ $q(\xi) = 4q(\xi - \xi^2)$                                      |
|  $\{q_e\} = q \begin{Bmatrix} \frac{3}{20}L \\ L^2/30 \\ \frac{7}{20}L \\ -L^2/20 \end{Bmatrix}$ $q(\xi) = q\xi$                                               |  $\{q_e\} = q \begin{Bmatrix} L/\pi \\ 2L^2/\pi^3 \\ L/\pi \\ -2L^2/\pi^3 \end{Bmatrix}$ $q(\xi) = q \sin(\pi\xi)$                          |
|  $\{q_e\} = q \begin{Bmatrix} \frac{7}{20}L \\ -L^2/20 \\ \frac{3}{20}L \\ -L^2/30 \end{Bmatrix}$ $q(\xi) = q(1 - \xi)$                                       |  $\{q_e\} = q \begin{Bmatrix} L/6 \\ L^2/60 \\ L/6 \\ -L^2/60 \end{Bmatrix}$ $q(\xi) = q - 4q(\xi - \xi^2)$                                |
|  $\{q_e\} = \frac{L}{60} \begin{Bmatrix} 21q_1 + 9q_2 \\ L(39q_1 + 29q_2) \\ 9q_1 + 21q_2 \\ -L(29q_1 + 39q_2) \end{Bmatrix}$ $q(\xi) = q_1 + (q_2 - q_1)\xi$ |  $\{q_e\} = q \begin{Bmatrix} L/15 \\ L^2/60 \\ 4L/15 \\ -L^2/30 \end{Bmatrix}$ $q(\xi) = q\xi^2$                                          |
|  $\{q_e\} = \frac{L}{60} \begin{Bmatrix} 21q_1 + 9q_2 \\ L(39q_1 + 29q_2) \\ 9q_1 + 21q_2 \\ -L(29q_1 + 39q_2) \end{Bmatrix}$ $q(\xi) = q_1 + (q_2 - q_1)\xi$ |  $\{q_e\} = q \begin{Bmatrix} \frac{80}{315}L \\ \frac{16}{315}L^2 \\ \frac{130}{315}L \\ -20/315L^2 \end{Bmatrix}$ $q(\xi) = q\sqrt{\xi}$ |

## 9.1.5) Sisteme Geçiş

Sisteme geçiş çevikme matrisleriyle ve birleştirme yöntemi ile yapılmaktadır.

a) Çevikme matrisleriyle sisteme geçiş



1. eleman için çevikme matrisi:

$$\begin{bmatrix} w_A \\ \theta_A \\ w_B \\ \theta_B \end{bmatrix}_{1\text{eleman}} = \underbrace{\begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}}_{C_{e1}} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{bmatrix}_{\text{sistem}}$$

\* 1. elemandaki yer değiştikmeler ( $w_A, \theta_A, w_B, \theta_B$ ), genel sistemin ( $D_1, D_2, D_3, D_4$ ) yer değiştikmelerle karşılaştırılmıştır.

2. eleman için çevikme matrisi:

$$\begin{bmatrix} w_A \\ \theta_A \\ w_B \\ \theta_B \end{bmatrix}_{2\text{eleman}} = \underbrace{\begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}}_{C_{e2}} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{bmatrix}_{\text{sistem}}$$

\* 2. elemandaki yer değiştikmeler ( $w_A, \theta_A, w_B, \theta_B$ ), genel sistemin ( $D_4, D_5, D_6$ ) yer değiştikmelerle karşılaştırılmıştır.

Not: Bu sistemde kullanılan 'sıralı eleman şekli' tek olduğundan 1 ve 2 nolu elemanlarda eleman yer değiştikmeleri aynı olmak anılmaktadır. Aksi durumda isimler değiştirilmelidir.



Toplam Sistem Rijitlik Matrisi;

$$[K]_s = [K_{e_1}] + [K_{e_2}]$$

$$[K] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L & 0 & 0 \\ 6L & 4L^2 & -6L & 2L^2 & 0 & 0 \\ -12 & -6L & 24 & 0 & -12 & 6L \\ 6L & 2L^2 & 0 & 8L^2 & -6L & 2L^2 \\ 0 & 0 & -12 & -6L & 12 & -6L \\ 0 & 0 & 6L & 2L^2 & -6L & 4L^2 \end{bmatrix}$$

Sistem yük vektörünün hesabı;

$$\{Q_{e_1}\} = [C_{e_1}]^T \{q_{e_1}\}$$

$$\{Q_{e_2}\} = [C_{e_2}]^T \{q_{e_2}\}$$

$$\{Q_{e_1}\} = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} q = \begin{bmatrix} 4/2 \\ L^2/12 \\ 4/2 \\ -L^2/12 \end{bmatrix} = q = \begin{bmatrix} 4/2 \\ L^2/12 \\ 4/2 \\ 0 \\ 0 \end{bmatrix}$$

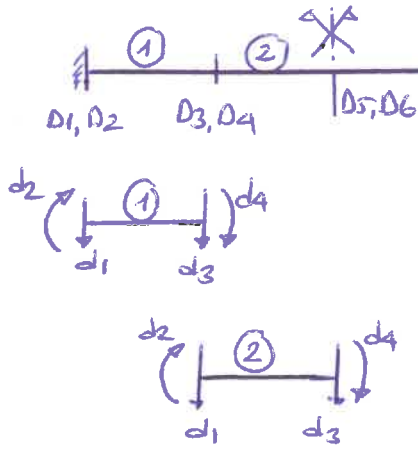
$$\{Q_{e_2}\} = \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} q = \begin{bmatrix} 4/2 \\ L^2/12 \\ 4/2 \\ -L^2/12 \end{bmatrix} = q = \begin{bmatrix} 0 \\ 0 \\ 4/2 \\ L^2/12 \\ 4/2 \\ -L^2/12 \end{bmatrix}$$

$$\{Q\} = \{Q_{e_1}\} + \{Q_{e_2}\}$$

$$\{Q\} = q = \begin{bmatrix} 4/2 \\ L^2/12 \\ L \\ 0 \\ 4/2 \\ -L^2/12 \end{bmatrix}$$



## b) Birleştirme yöntemiyle sisteme geçiş



| Eleman No           | Eleman deplasman parametreleri                               | Sisteme deplasman parametresi |
|---------------------|--------------------------------------------------------------|-------------------------------|
| ①                   | $d_1 = w_A ; d_3 = w_B$<br>$d_2 = \theta_A ; d_4 = \theta_B$ | $D_1 ; D_3$<br>$D_2 ; D_4$    |
| ②                   | $d_1 = w_A ; d_3 = w_B$<br>$d_2 = \theta_A ; d_4 = \theta_B$ | $D_3 ; D_5$<br>$D_4 ; D_6$    |
| Güçler ve momentler | $i'_{ucu} ; j'_{ucu}$                                        | $i_{ucu} ; j_{ucu}$           |

Eleman Rijitlik Matrisi,  $[k_e] =$

$$\begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{42} & k_{43} & k_{44} \end{bmatrix}$$

|       | $D_1$      | $D_2$      | $D_3$                 | $D_4$                 | $D_5$      | $D_6$      |
|-------|------------|------------|-----------------------|-----------------------|------------|------------|
| $D_1$ | $k_{11}^1$ | $k_{12}^1$ | $k_{13}^1$            | $k_{14}^1$            | 0          | 0          |
| $D_2$ | $k_{21}^1$ | $k_{22}^1$ | $k_{23}^1$            | $k_{24}^1$            | 0          | 0          |
| $D_3$ | $k_{31}^1$ | $k_{32}^1$ | $k_{33}^1 + k_{11}^2$ | $k_{34}^1 + k_{12}^2$ | $k_{13}^2$ | $k_{14}^2$ |
| $D_4$ | $k_{41}^1$ | $k_{42}^1$ | $k_{43}^1 + k_{21}^2$ | $k_{44}^1 + k_{22}^2$ | $k_{23}^2$ | $k_{24}^2$ |
| $D_5$ | 0          | 0          | $k_{31}^2$            | $k_{32}^2$            | $k_{33}^2$ | $k_{34}^2$ |
| $D_6$ | 0          | 0          | $k_{41}^2$            | $k_{42}^2$            | $k_{43}^2$ | $k_{44}^2$ |

$k_{ij}$   
↓  
ypr sebep

Eleman Rijitlik matrisi  $[k_e] = \frac{EI}{L^3}$

$$\begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix}$$

$K_{33} = k_{33}^1 + k_{11}^2 = \frac{EI}{L^3} (12 + 12) = \frac{24EI}{L^3}$   
 $K_{34} = k_{34}^1 + k_{12}^2 = \frac{EI}{L^3} (-6L + 6L) = 0$   
 $K_{43} = k_{43}^1 + k_{21}^2 = \frac{EI}{L^3} (-6L + 6L) = 0$   
 $K_{44} = k_{44}^1 + k_{22}^2 = \frac{EI}{L^3} (4L^2 + 4L^2) = \frac{8EI}{L^3} (8L^2)$

Sistem rijitlik matrisi;  $[K] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L & 0 & 0 \\ 6L & 4L^2 & -6L & 2L^2 & 0 & 0 \\ -12 & -6L & 24 & 0 & -12 & 6L \\ 6L & 2L^2 & 0 & 8L^2 & -6L & 2L^2 \\ 0 & 0 & -12 & -6L & 12 & -6L \\ 0 & 0 & 6L & 2L^2 & -6L & 4L^2 \end{bmatrix}$

Sistem rijitlik matrisi  $[K]$ , yük matrisi  $\{Q\}$  ve yer değiştirilme matrisi  $\{D_s\}$  ile;

$$[K]\{D_s\} = \{Q\} \text{ olur.}$$

$$\frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L & 0 & 0 \\ 6L & 4L^2 & -6L & 2L^2 & 0 & 0 \\ -12 & -6L & 24 & 0 & -12 & 6L \\ 6L & 2L^2 & 0 & 8L^2 & -6L & 2L^2 \\ 0 & 0 & -12 & -6L & 12 & -6L \\ 0 & 0 & 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{bmatrix} = \begin{bmatrix} 4/2 \\ L^2/12 \\ L \\ 0 \\ 4/2 \\ -L^2/12 \end{bmatrix}$$

### 9.1.6) Sınır Koşulları

Sonlu elemanlar yönteminin diğer yöntemlere göre avantajı sınır koşullarının probleme en sonunda uygulanmasıdır. Bu problemde simetriiden faydalanıldığından simetrii koşuluna göre  $D_6 = 0$ , mesnet koşullarına göre  $D_1 = D_2 = 0$  (Ankastre mesnette  $w = w' = 0$ ) olur. Bu durumda rijitlik, yer değiştirilme ve yük matrisinin ilgili satır ve sütunları sıfırlanır.

$$\frac{EI}{L^3} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 24 & 0 & -12 & 0 \\ 0 & 0 & 0 & 8L^2 & -6L & 0 \\ 0 & 0 & -12 & -6L & 12 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ D_3 \\ D_4 \\ D_5 \\ 0 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ L \\ 0 \\ L/2 \\ 0 \end{bmatrix}$$

$$\left. \begin{array}{l} 24 D_3 - 12 D_5 = 9L \left( \frac{L^3}{EI} \right) \text{ ---- 1} \\ 8L^2 D_4 - 6L D_5 = 0 \text{ ---- 2} \\ -12 D_3 - 6L D_4 + 12 D_5 = 9 \frac{L}{2} \frac{L^3}{EI} \text{ ---- 3} \end{array} \right\} \text{ Denklemleri gözünüzden;}$$

$$D_3 = \frac{3}{8} 9 \frac{L^4}{EI}; D_4 = \frac{1}{2} 9 \frac{L^3}{EI}$$

$$D_5 = \frac{2}{3} 9 \frac{L^4}{EI} \text{ bulunur.}$$

9.1.7) Sistem momentlerinin hesabı

$$M_i = -EI w'' \rightarrow M_1 = -EI [N_1' \ N_2' \ N_3' \ N_4'] \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{bmatrix}$$

1. elemanda ( $M_A$  ve  $M_B$ 'nin hesabı)

$$M_1 = -\frac{EI}{L^2} \left[ \left( -6 + \frac{12x}{L} \right) L \left( -4 + \frac{6x}{L} \right) \left( 6 - \frac{12x}{L} \right) L \left( -2 + \frac{6x}{L} \right) \right] \begin{bmatrix} 0 \\ 0 \\ \frac{3}{8} 9 \frac{L^4}{EI} \\ \frac{1}{2} 9 \frac{L^3}{EI} \end{bmatrix}$$

$$M_1 = -\left( \frac{5}{4} - \frac{6x}{4L} \right) 9L^2$$

$$x=0 \rightarrow M_1 = -\frac{5}{4} 9L^2 \rightarrow L = \frac{L_s}{4} \rightarrow M_A = -\frac{5}{64} 9L_s^2 = -\frac{1}{128} 9L_s^2$$

$$\left( \text{Kesin çözüm } M_1 = -\frac{9L_s^2}{12} \right)$$

$$x=L \rightarrow M_B = \frac{9L^2}{4}$$

2. elemanları ( $M_B^2$  ve  $M_C$  hesabı)

$$M_2 = -\frac{EI}{L^2} \left[ \left( -6 + \frac{12x}{L} \right) L \left( -4 + \frac{6x}{L} \right) \left( 6 - \frac{12x}{L} \right) L \left( 2 + \frac{6x}{L} \right) \right]$$

$$M_2 = \left( \frac{1}{4} + \frac{2x}{4L} \right) qL^2$$

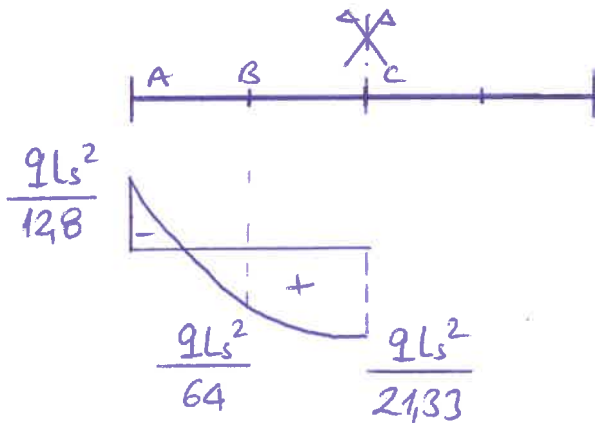
$$\begin{bmatrix} \frac{3}{8} q \frac{L^4}{EI} \\ \frac{1}{2} q \frac{L^3}{EI} \\ \frac{2}{3} q \frac{L^4}{EI} \\ 0 \end{bmatrix}$$

$$x=0 \rightarrow M_B^2 = \frac{qL^2}{4}$$

$$x=L \rightarrow M_C = \frac{3}{4} qL^2 \rightarrow L = \frac{L_3}{4} \rightarrow M_C = \frac{3}{64} qL_3^2 = \frac{1}{21,33} qL_3^2$$

(Keşif ~~görm~~  $M_C = \frac{qL_3^2}{24}$ )

$$M_B = (M_B^1 + M_B^2) / 2 = \frac{qL^2}{4} \rightarrow L = \frac{L_3}{4} \rightarrow M_B = \frac{1}{64} qL_3^2$$



# Rijlik Matematik Yardımıyla Kesit Tesirlerinin Hesabı

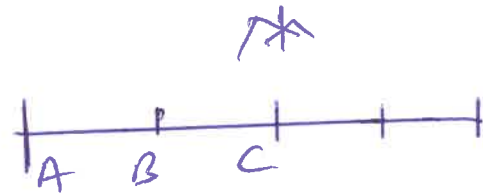
$$\{F\}^e = [K]^e \cdot \{D\}^e - \{B\}^e$$

1. nolu elemanın kesit tesirleri:

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^1 = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ \frac{3qL^4}{8EI} \\ \frac{1}{2}q\frac{L^3}{EI} \end{Bmatrix} - \begin{Bmatrix} \frac{qL}{2} \\ \frac{qL^2}{12} \\ \frac{qL}{2} \\ -\frac{qL^2}{12} \end{Bmatrix} = \begin{Bmatrix} -\frac{2qL}{3} \\ -\frac{4}{3}qL^2 \\ qL \\ -\frac{qL^2}{6} \end{Bmatrix}$$

$L = \frac{L_s}{4}$  olduğundan;

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^1 = \begin{Bmatrix} V_A \\ M_A \\ V_B \\ M_B \end{Bmatrix} = \begin{Bmatrix} -\frac{qL_s}{2} \\ -\frac{qL_s^2}{12} \\ \frac{qL_s}{4} \\ -\frac{qL_s^2}{96} \end{Bmatrix}$$

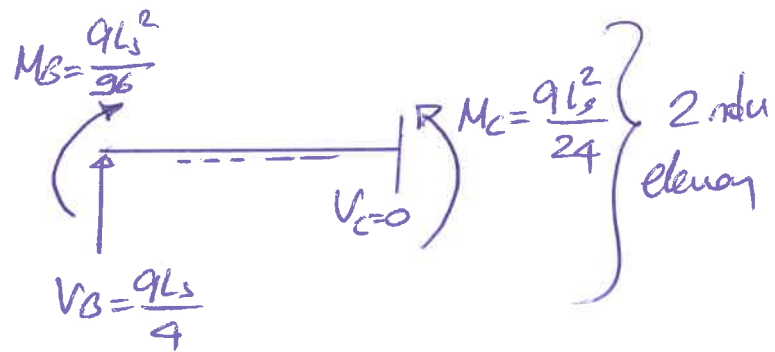
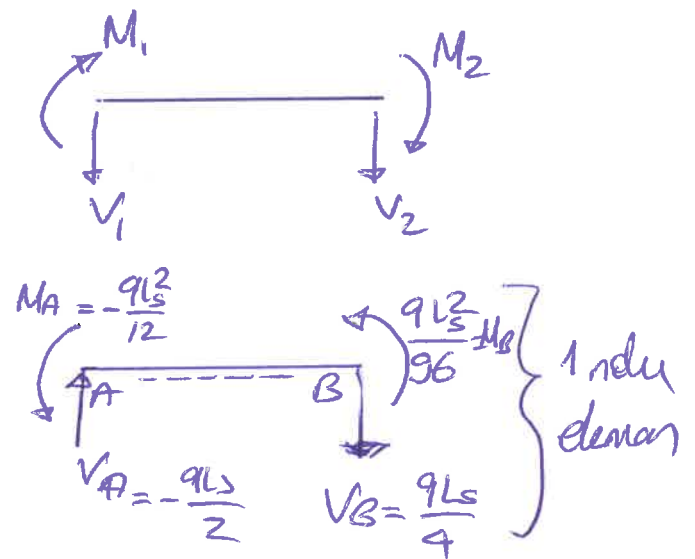
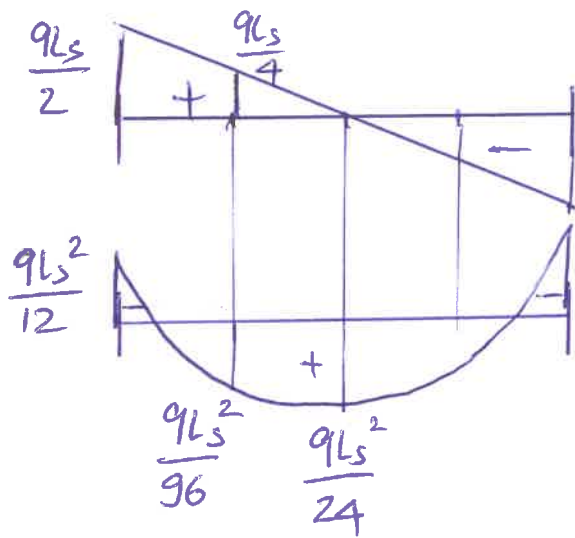


2. nolu elemanın kesit tesirleri:

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^2 = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} \frac{3}{8}q\frac{L^4}{EI} \\ \frac{1}{2}q\frac{L^3}{EI} \\ \frac{2}{3}q\frac{L^4}{EI} \\ 0 \end{Bmatrix} - \begin{Bmatrix} \frac{qL}{2} \\ \frac{qL^2}{12} \\ \frac{qL}{2} \\ -\frac{qL^2}{12} \end{Bmatrix} = \begin{Bmatrix} -qL \\ qL^2/6 \\ 0 \\ -\frac{2}{3}qL^2 \end{Bmatrix}$$

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^2 = \begin{Bmatrix} M_B \\ M_B \\ V_C \\ M_C \end{Bmatrix} = \begin{Bmatrix} -qL/4 \\ qL^2/96 \\ 0 \\ -qL^2/24 \end{Bmatrix}$$

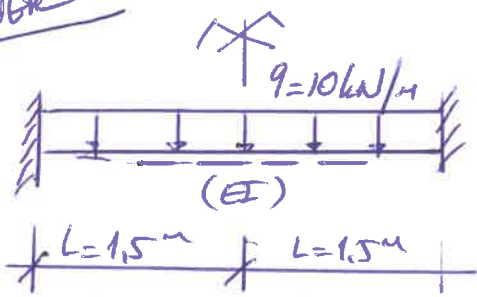
(85a)



Yukarıdaki hesaplardan da anlaşılabileceği gibi kesit tesirlerini varyasyon yöntemi ya da serüf fonksiyonları yardımıyla hesapladığımızda elde edilen sonuçlar yaklaşıklık olacaktır. Halbuki rijitlik matrisi yardımıyla bulunan kesit tesirleri kesin sonuçlardır.

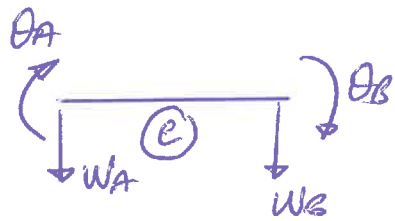
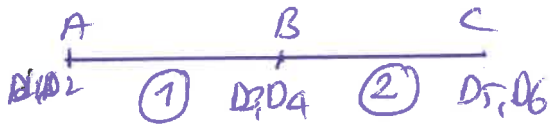


ORNAK



$$EI = 3 \times 10^4 \text{ kNm}^2$$

$$q = 10 \text{ kN/m}$$



$$[k_e] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix}$$

$$\{Q\}^e = \begin{Bmatrix} qL/2 \\ qL^2/12 \\ qL/2 \\ -qL^2/12 \end{Bmatrix}$$

$$[C_1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix} \quad [C_2] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$\left. \begin{aligned} [K_1]^e &= [C_1]^T [k_e] [C_1] \\ [K_2]^e &= [C_2]^T [k_e] [C_2] \end{aligned} \right\} [K]_s = [K_1]^e + [K_2]^e$$

$$\left. \begin{aligned} \{Q_1\}^e &= [C_1]^T \{Q\}^e \\ \{Q_2\}^e &= [C_2]^T \{Q\}^e \end{aligned} \right\} [Q]_s = \{Q_1\}^e + \{Q_2\}^e$$

$$\frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L & 0 & 0 \\ 6L & 4L^2 & -6L & 2L^2 & 0 & 0 \\ -12 & -6L & 24 & 0 & -12 & 6L \\ 6L & 2L^2 & 0 & 8L^2 & -6L & 2L^2 \\ 0 & 0 & 12 & -6L & 12 & -6L \\ 0 & 0 & 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} = \begin{Bmatrix} qL/2 \\ qL^2/12 \\ 9L \\ 0 \\ qL/2 \\ -qL^2/12 \end{Bmatrix}$$

$$D_1 = D_2 = D_4 = D_5 = D_6 = 0$$

$$D_3 = w_B = \frac{qL^4}{24EI} = \frac{10 \cdot 15^4}{24 \cdot 3 \cdot 10^4} = 303 \times 10^{-5} \text{ m.}$$

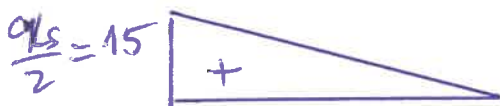
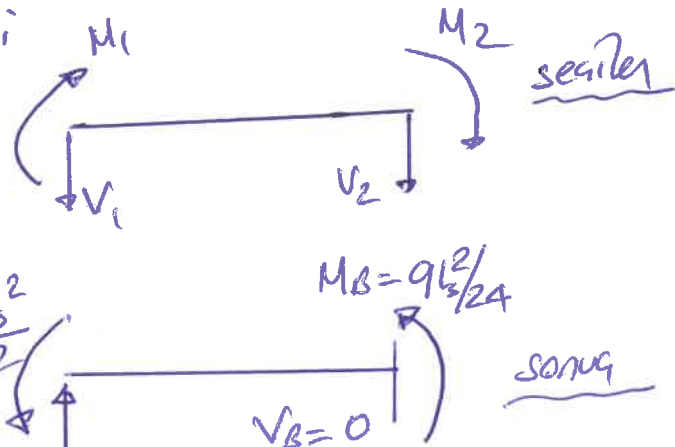
Eleman Kiri7 Tesitleri

1 nolu eleman

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^1 = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ \frac{qL^4}{24EI} \\ 0 \end{Bmatrix} - \begin{Bmatrix} qL/2 \\ qL^2/12 \\ qL/2 \\ -qL^2/12 \end{Bmatrix} = \begin{Bmatrix} -qL \\ -qL^2/3 \\ 0 \\ -qL^2/6 \end{Bmatrix}$$

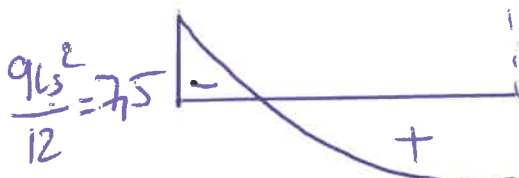
$L = \frac{L_s}{2}$  olduğundan;

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^1 = \begin{Bmatrix} V_A \\ M_A \\ V_B \\ M_B \end{Bmatrix}^1 = \begin{Bmatrix} -\frac{qL_s}{2} \\ -\frac{qL_s^2}{12} \\ 0 \\ -\frac{qL_s^2}{24} \end{Bmatrix}$$



$V^1 (\text{kN})$

$$V_A = \frac{qL_s}{2}$$



$M^1 (\text{kNm})$   $L_s = 3 \text{ m}$

$$\frac{qL_s^2}{24} = 3.75$$

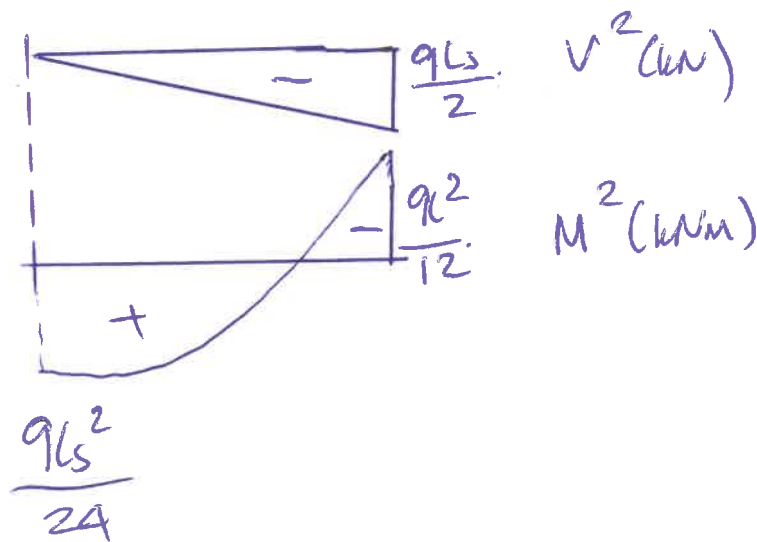
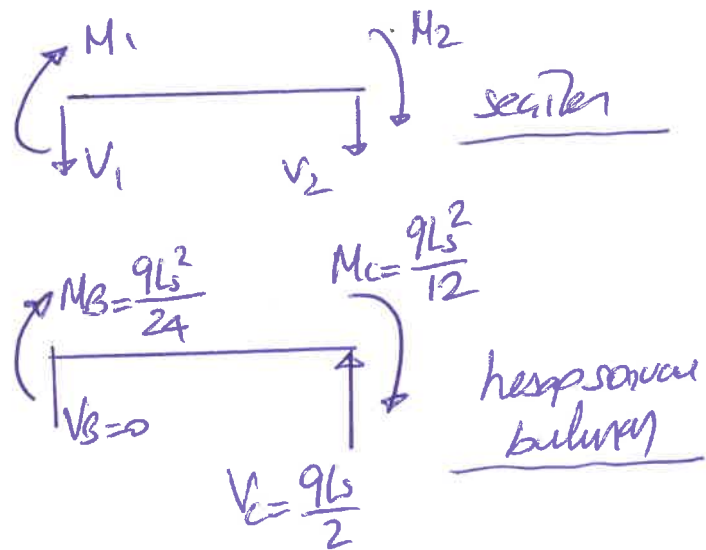
85 d

2 node element

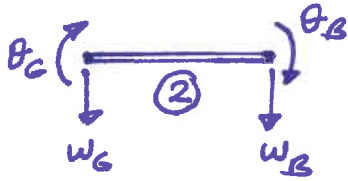
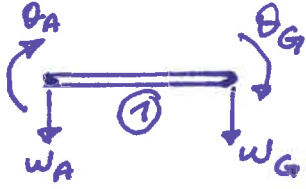
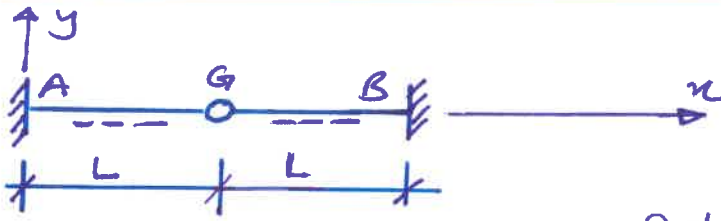
$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix}^2 = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} qL^4/24EI \\ 0 \\ 0 \\ 0 \end{Bmatrix} - \begin{Bmatrix} qL/2 \\ qL^2/12 \\ qL/2 \\ -qL^2/12 \end{Bmatrix} = \begin{Bmatrix} 0 \\ qL^2/6 \\ -qL \\ qL^2/3 \end{Bmatrix}$$

$L = \frac{L_s}{2}$  o haligundan;

$$\begin{Bmatrix} V_1 \\ M_1 \\ V_2 \\ M_2 \end{Bmatrix} = \begin{Bmatrix} V_B \\ M_B \\ V_C \\ M_C \end{Bmatrix} = \begin{Bmatrix} 0 \\ qL_s^2/24 \\ -qL_s/2 \\ qL_s^2/12 \end{Bmatrix}$$



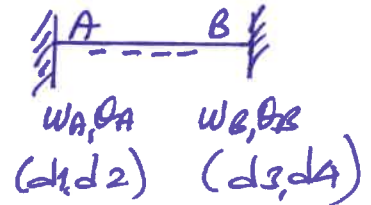
## ÇUBUKTA MAFSAL OLMAYI DURUMU



Şekilde göeüldüğü gibi iki ucu rijit çubukun orta noktasında moment mafsalı bulunmaktadır. Çubukta ki yerdeğıstikmeler şekle vermiştir.

a) İki ucu rijit çubukun rijitlik matrisi

$$[k] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{matrix} w_A \\ \theta_A \\ w_B \\ \theta_B \end{matrix}$$



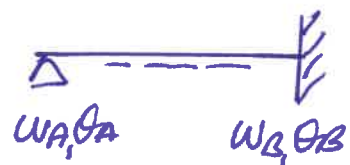
b) Sağ ucu mafsalı çubukun rijitlik matrisi

$$[k] = \frac{EI}{L^3} \begin{bmatrix} 3 & 3L & -3 & 0 \\ 3L & 3L^2 & -3L & 0 \\ -3 & -3L & 3 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{matrix} w_A \\ \theta_A \\ w_B \\ \theta_B \end{matrix}$$

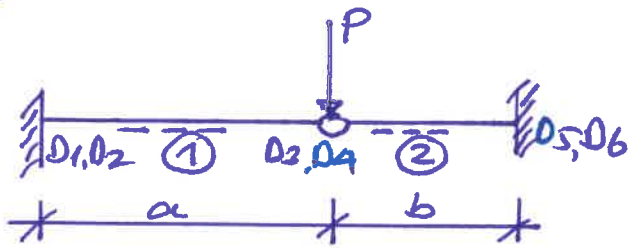


c) Sol ucu mafsalı çubukun rijitlik matrisi

$$[k] = \frac{EI}{L^3} \begin{bmatrix} 3 & 0 & -3 & 3L \\ 0 & 0 & 0 & 0 \\ -3 & 0 & 3 & -3L \\ 3L & 0 & -3L & 3L^2 \end{bmatrix} \begin{matrix} w_A \\ \theta_A \\ w_B \\ \theta_B \end{matrix}$$

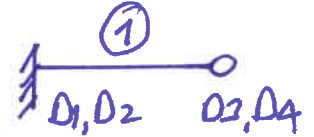


4. ÖRNEK

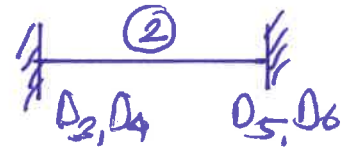


Matris 1 nolu gıbuğun sağında seklinde hesaplayacağız.

$$[k_1]_e = \frac{EI}{a^3} \begin{bmatrix} D_1 & D_2 & D_3 & D_4 \\ 3 & 3a & -3 & 0 \\ 3a & 3a^2 & -3a & 0 \\ -3 & -3a & 3 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{matrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{matrix}$$



$$[k_2]_e = \frac{EI}{b^3} \begin{bmatrix} D_3 & D_4 & D_5 & D_6 \\ 12 & 6b & -12 & 6b \\ 6b & 4b^2 & -6b & 2b^2 \\ -12 & -6b & 12 & -6b \\ 6b & 2b^2 & -6b & 4b^2 \end{bmatrix} \begin{matrix} D_3 \\ D_4 \\ D_5 \\ D_6 \end{matrix}$$



Geçirme Matrisleri

$$[C_1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

$$[C_2] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[K_1]^e = [C_1]^T [k_1]_e [C_1]$$

$$[K_2]^e = [C_2]^T [k_2]_e [C_2]$$

$$[K_s] = [K_1]^e + [K_2]^e$$

$$\{Q\} = \begin{Bmatrix} 0 \\ 0 \\ P \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

$$D_1 = D_2 = D_5 = D_6 = 0$$

$$EI \begin{bmatrix} 3/a^3 & 3/a^2 & -3/a^3 & 0 & 0 & 0 \\ 3/a^2 & 3/a & -3/a^2 & 0 & 0 & 0 \\ -3/a^3 & -3/a^2 & 3/a^3 + 12/b^3 & 6/b^2 & -12/b^3 & 6/b^2 \\ 0 & 0 & 6/b^2 & 4/b & -6/b^2 & 2/b \\ 0 & 0 & -12/b^3 & -6/b^2 & 12/b^3 & -6/b^2 \\ 0 & 0 & 6/b^2 & 2/b & -6/b^2 & 4/b \end{bmatrix} \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ P \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

$$EI \begin{bmatrix} 3/a^3 + 12/b^3 & 6/b^2 \\ 6/b^2 & 4/b \end{bmatrix} \begin{Bmatrix} D_3 \\ D_4 \end{Bmatrix} = \begin{Bmatrix} P \\ 0 \end{Bmatrix}$$

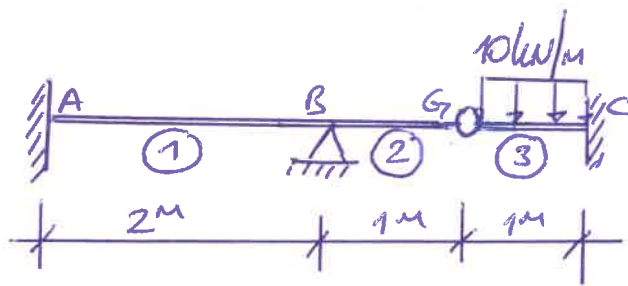
$$D_3 = w_3 = \frac{-a^3 b^3 P}{3(b^3 + a^3)EI}$$

$$D_4 = \theta_3 = \frac{a^3 b^2 P}{2(b^3 + a^3)EI}$$

ödev : Göbek kuvvetlerini (kesit tesitleri) bularak M, V diyagramlarını çiziniz.



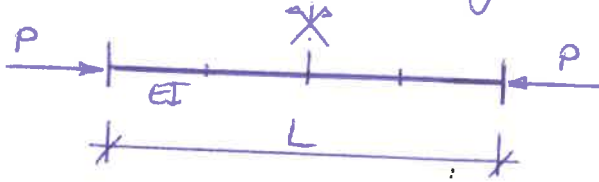
ÖDEV



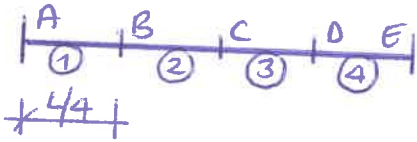
$$E = 210 \text{ GPa} , \quad I = 2 \cdot 10^4 \text{ m}^4$$

## 9.2) Gubugun Burkulma Hesabı

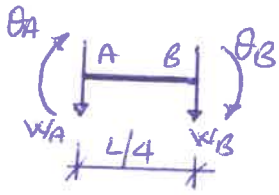
Eksereel  $P$  basınç kuvvetine maruz  $L$  uzunluğunda bir gubugu ele alalım. Ve gubugu 4 eşit parçaya bölelim;



← Ele alınan gubuk elemanı



← Gubugun sonlu elemanlara ayrılması



← Sonlu eleman parçası ve yer değişkenler

$$\{d\}^T = [W_A, \theta_A, W_B, \theta_B] = [d_1, d_2, d_3, d_4] \quad \leftarrow \text{yer değişkenler}$$

$$A(0,0); B(L,0) \quad \leftarrow \text{Eleman koordinatları}$$

### 9.2.1) Deplasman Fonksiyonlarının Seçimi

Deplasman fonksiyonları rijit cisim hareketi ve sabit deplasman şartını sağlayacak şekilde seçilmelidir. Koordinat eksenini değiştirilse çözüm farklı olmamalıdır. Bunun için deplasman fonksiyonu ya tam polinom veya tabii koordinatların fonksiyonu şeklinde olmalıdır. Elemanın içinde ve kenarlarında sürekli olmalıdır. Ayrıca iç ve dış kuvvetlerin içindeki türevlerde de sürekli olmalıdır. Burada deplasman fonksiyonu polinom olarak seçtiğimizde Elemanda bilinmeyen dört deplasman parametresi olduğundan, dört bilinmeyenli deplasman fonksiyonu seçtiğimizde

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

$$w = [\phi(x)] \{a\} = [1, x, x^2, x^3] \{a\}$$

### 9.2.2) Sekül Fonksiyonlarının Tayini

Eleman düğüm noktası deplasmanları  $\{d\}$  ile polinom sabitleri arasındaki bağı veren  $[A]$  matrisi de; elemanın düğüm noktası deplasman parametrelerinin  $\phi(x)$  ve  $\phi'(x)$ 'in türevleri altında yazılır bileşenlerinde, düğüm noktası koordinatlarının yerlerine konulmasıyla bulunur.

$$\{d\} = [A]\{a\}$$

$$\{a\} = [A]^{-1}\{d\} \quad [A]^{-1} = [B] \text{ de;}$$

$$\{a\} = [B]\{d\}$$

$$w = \underbrace{[\phi(x)]}_{[N]} [B] \{d\} = [N] \{d\} \quad | [N]: \text{sekül fonksiyonu}$$

$$w = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

$$w' = \theta = a_2 + 2a_3 x + 3a_4 x^2$$

$$\{d\} = [A]\{a\}$$

$$d_1 = (w_A) = w(x=0) = a_1$$

$$d_2 = \theta_A = w'(x=0) = a_2$$

$$d_3 = w_B = w(x=L/4) = a_1 + a_2 \frac{L}{4} + a_3 \frac{L^2}{16} + a_4 \frac{L^3}{64}$$

$$d_4 = \theta_B = w'(x=L/4) = a_2 + 2a_3 \frac{L}{4} + 3a_4 \frac{L^2}{16}$$

$$\begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 1 & L/4 & L^2/16 & L^3/64 \\ 0 & 1 & L/2 & 3L^2/16 \end{bmatrix} \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{Bmatrix}$$

$$[B] = [A]^{-1} = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ \frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^2} & \frac{16}{L^2} \end{bmatrix}$$

$$[N] = [\phi(x)] [B] = [1, x, x^2, x^3] [B]$$

$$[N_1, N_2, N_3, N_4] = [1, x, x^2, x^3] \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix}$$

$$N_1 = 1 - \frac{48x^2}{L^2} + \frac{128x^3}{L^3}$$

$$N_2 = x - \frac{8x^2}{L} + \frac{16x^3}{L^2}$$

$$N_3 = \frac{48x^2}{L^2} - \frac{128x^3}{L^3}$$

$$N_4 = -\frac{4x^2}{L} + \frac{16}{L^2} x^3$$

### 9.2.3) Eleman Rijitlik Matrisinin Hesabı

Uçlarından elastik P basıncı yüküne maruz bir çubuğun toplam potansiyel enerjisinin türevi varyasyonunu yazarsak;

$$\delta^2 \Pi^e = \int_0^{L/4} [EI(w'')^2 - P(w')^2] dx$$

$$\delta^2 \Pi^e = \int_0^{L/4} \{\epsilon\}^T [0] \{\epsilon\} dx + \int_0^{L/4} \{\beta\}^T [N_0] \{\beta\} dx$$

$$\delta^2 \Pi^e = \{d\}^T [k_e] \{d\} - P \{d\}^T [n_e] \{d\}$$

$$\{d\}^T [k_e] \{d\} = EI \int_0^{L/4} (w'')^2 dx$$

$$2 \{d\}^T [n_e] \{d\} = -P \int_0^{L/4} (w')^2 dx$$

$$\delta^2 \Pi^e = \{d\}^T ([k_e] + 2[n_e]) \{d\}$$

Burada  $N_0 = -P$ ,  $[k_e]$  element rijitlik matrisi, 2 yök parametres,  $[n_e]$  geometrik yök matrisidir. Element rijitlik ve geometrik yök matrislerini integral matrisleri ile hesaplasak;

$$w'' = -2a_3 - 6a_4x$$

$$\{\varepsilon\} = [F]\{a\} \quad ; \quad [F] = [0, 0, -2, -6x]$$

$$\{\varepsilon\} = [F][B]\{d\}$$

$$[k_e] = EI \int_0^{L/4} [B]^T [F]^T [F] [B] dx$$

$$[k_e] = EI \int_0^{L/4} \begin{bmatrix} 1 & 0 & -\frac{48}{L^2} & \frac{128}{L^3} \\ 0 & 1 & -\frac{8}{L} & \frac{16}{L^2} \\ 0 & 0 & \frac{48}{L^2} & -\frac{128}{L^3} \\ 0 & 0 & -\frac{4}{L} & \frac{16}{L^2} \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ -2 \\ -6x \end{bmatrix} \begin{bmatrix} 0 & 0 & -2 & -6x \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix} dx$$

$$[k_e] = EI \int_0^{L/4} \begin{bmatrix} 0 & 0 & -\frac{192}{L^2} + \frac{1536x}{L^3} & -\frac{576x}{L^2} + \frac{4608x^2}{L^3} \\ 0 & 0 & -\frac{32}{L} + \frac{192x}{L^2} & -\frac{96x}{L} + \frac{576x^2}{L^2} \\ 0 & 0 & \frac{192}{L^2} - \frac{1536x}{L^3} & \frac{576x^2}{L^2} - \frac{4608x^2}{L^3} \\ 0 & 0 & -\frac{16}{L} + \frac{192x}{L^2} & -\frac{48x}{L} + \frac{576x^2}{L^2} \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix} dx$$

$$[k_e] = \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{42} & k_{43} & k_{44} \end{bmatrix}$$

$$\begin{aligned} k_{11} &= EI \int_0^{L/4} \left[ -\frac{48}{L^2} \left( -\frac{192}{L^2} + \frac{1536x}{L^3} \right) + \frac{128}{L^3} \left( -\frac{576x}{L^2} + \frac{4608x^2}{L^3} \right) \right] dx \\ &= EI \left[ -\frac{48}{L^2} \left( -\frac{192x}{L^2} + \frac{1536x^2}{2L^3} \right) + \frac{128}{L^3} \left( -\frac{576x^2}{2L^2} + \frac{4608x^3}{3L^3} \right) \right]_0^{L/4} \\ &= \frac{768EI}{L^3} \end{aligned}$$

$$[k_e] = \frac{EI}{L^3} \begin{bmatrix} 768 & 96L & -768 & 96L \\ 96L & 16L^2 & -96L & 8L^2 \\ -768 & -96L & 768 & -96L \\ 96L & 8L^2 & -96L & 16L^2 \end{bmatrix}$$

#### 9.2.4) Eleman Geometrik Yük Matrisi'

$$w' = a_2 + 2a_3x + 3a_4x^2 \quad \text{or } w,$$

$$\{\beta\} = [G]\{a\} \longrightarrow [G] = [0, 1, 2x, 3x^2]$$

$$\{\beta\} = [G][B]\{d\}$$

$$[k_e] = \int_0^{L/4} [B]^T [G]^T [G] [B] dx \quad \text{dir.}$$



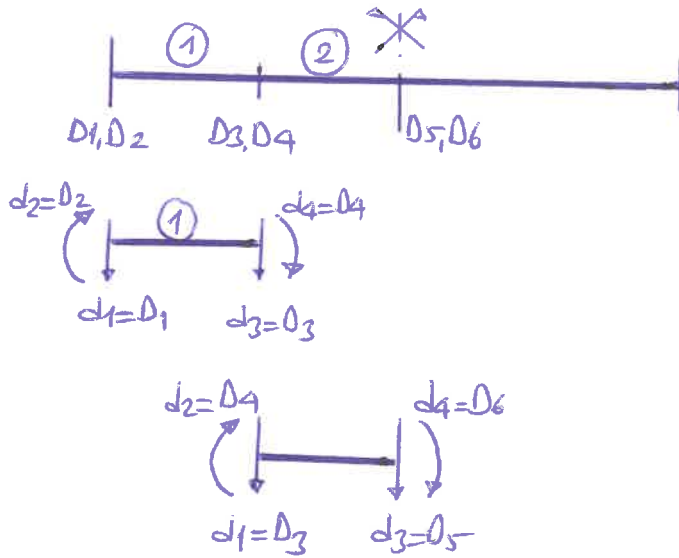
$$[n_e] = \int_0^{L/4} \begin{bmatrix} 1 & 0 & -\frac{48}{L^2} & \frac{128}{L^3} \\ 0 & 1 & -\frac{8}{L} & \frac{16}{L^2} \\ 0 & 0 & \frac{48}{L^2} & -\frac{128}{L^3} \\ 0 & 0 & -\frac{4}{L} & \frac{16}{L^2} \end{bmatrix} \begin{bmatrix} 0 \\ 1 \\ 2x \\ 3x^2 \end{bmatrix} \begin{bmatrix} 0 & 1 & 2x & 3x^2 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix} dx$$

$$[n_e] = \int_0^{L/4} \begin{bmatrix} 0 & \left(-\frac{96x}{L^2} + \frac{384}{L^3}x^2\right) & \left(-\frac{192}{L^2}x^2 + \frac{768}{L^3}x^3\right) & \left(-\frac{288}{L^2}x^3 + \frac{1552}{L^3}x^4\right) \\ 0 & \left(1 - \frac{16x}{L} + \frac{48}{L^2}x^2\right) & \left(2x - \frac{32}{L^2}x^2 + \frac{96}{L^2}x^3\right) & \left(3x^2 - \frac{48}{L}x^3 + \frac{144}{L^2}x^4\right) \\ 0 & \left(\frac{96x}{L^2} - \frac{384}{L^3}x^2\right) & \left(\frac{192}{L^2}x^2 - \frac{768}{L^3}x^3\right) & \left(\frac{288}{L^2}x^3 - \frac{1552}{L^3}x^4\right) \\ 0 & \left(-\frac{8x}{L} + \frac{48}{L^2}x^2\right) & \left(-\frac{16x^2}{L} + \frac{96}{L^2}x^3\right) & \left(-\frac{24}{L}x^3 + \frac{144}{L^2}x^4\right) \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix} dx$$

$$[n_e] = \begin{bmatrix} n_{11} & n_{12} & n_{13} & n_{14} \\ n_{21} & n_{22} & n_{23} & n_{24} \\ n_{31} & n_{32} & n_{33} & n_{34} \\ n_{41} & n_{42} & n_{43} & n_{44} \end{bmatrix} = \frac{1}{L} \begin{bmatrix} \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} \\ \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} \\ -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} \\ \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} \end{bmatrix}$$

$$\begin{aligned} n_{11} &= \int_0^{L/4} \left[ -\frac{48}{L^2} \left( -\frac{192}{L^2}x^2 + \frac{768}{L^3}x^3 \right) + \frac{128}{L^3} \left( -\frac{288}{L^2}x^3 + \frac{1552}{L^3}x^4 \right) \right] dx \\ &= \left[ -\frac{48}{L^2} \left| -\frac{192}{L^2} \frac{x^3}{3} + \frac{768}{L^3} \frac{x^4}{4} \right|_0^{L/4} + \frac{128}{L^3} \left| -\frac{288}{L^2} \frac{x^4}{4} + \frac{1552}{L^3} \frac{x^5}{5} \right|_0^{L/4} \right] \\ &= \frac{24}{5L} \end{aligned}$$

### 9.2.5) Çevirme Matrisleri



Birinci' eleman için çevirme matrisi;

$$\begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \end{bmatrix} = \underbrace{\begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}}_{[C_{e1}]} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{bmatrix}$$

İkinci' eleman için çevirme matrisi;

$$\begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \end{bmatrix} = \underbrace{\begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}}_{[C_{e2}]} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{bmatrix}$$

### 9.2.6) Sistemin Rijitlik ve Geometrik Yük Matrislerinin Hesabı

$$[K_{e1}] = [C_{e1}]^T [k_e] [C_{e1}]$$

$$[K_{e1}] = \frac{EI}{L^3} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 768 & 96L & -768 & 96L \\ 96L & 16L^2 & -96L & 8L^2 \\ -768 & -96L & 768 & -96L \\ 96L & 8L^2 & -96L & 16L^2 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

$$[K_{e1}] = \frac{EI}{L^3} \begin{bmatrix} 768 & 96L & -768 & 96L & 0 & 0 \\ 96L & 16L^2 & -96L & 8L^2 & 0 & 0 \\ -768 & -96L & 768 & -96L & 0 & 0 \\ 96L & 8L^2 & -96L & 16L^2 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[K_{e2}] = \frac{EI}{L^3} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 768 & 96L & -768 & 96L \\ 96L & 16L^2 & -96L & 8L^2 \\ -768 & -96L & 768 & -96L \\ 96L & 8L^2 & -96L & 16L^2 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

$$[N_{e1}] = [C_{e1}]^T [N_e] [C_{e1}]$$

$$[N_{e1}] = \frac{1}{L} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} \\ \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} \\ -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} \\ \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

$$[N_{e2}] = [C_{e2}]^T [N_e] [C_{e2}]$$

$$[N_{e2}] = \frac{1}{L} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} \\ \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} \\ -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} \\ \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} \end{bmatrix} \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[N_{e1}] = \frac{1}{L} \begin{bmatrix} \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} & 0 & 0 \\ \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} & 0 & 0 \\ -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} & 0 & 0 \\ \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \quad [N_{e2}] = \frac{1}{L} \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} \\ 0 & 0 & \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} \\ 0 & 0 & -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} \\ 0 & 0 & \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} \end{bmatrix}$$

Sistem rijitlik matrisi  $[K] = [K_1] + [K_2]$

$$[K] = \frac{EI}{L^3} \begin{bmatrix} 768 & 96L & -768 & 96L & 0 & 0 \\ 96L & 16L^2 & -96L & 8L^2 & 0 & 0 \\ -768 & -96L & 768 & -96L & 0 & 0 \\ 96L & 8L^2 & -96L & 16L^2 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} + \frac{EI}{L^3} \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 768 & 96L & -768 & 96L \\ 0 & 0 & 96L & 16L^2 & -96L & 8L^2 \\ 0 & 0 & -768 & -96L & 768 & -96L \\ 0 & 0 & 96L & 8L^2 & -96L & 16L^2 \end{bmatrix}$$

$$[K] = \frac{EI}{L^3} \begin{bmatrix} 768 & 96L & -768 & 96L & 0 & 0 \\ 96L & 16L^2 & -96L & 8L^2 & 0 & 0 \\ -768 & -96L & 1536 & 0 & -768 & 96L \\ 96L & 8L^2 & 0 & 32L^2 & -96L & 8L^2 \\ 0 & 0 & -768 & -96L & 768 & -96L \\ 0 & 0 & 96L & 8L^2 & -96L & 16L^2 \end{bmatrix}$$

Sistem geometrik yük matrisi;  $[N] = [N_1] + [N_2]$

$$[N] = \frac{1}{L} \begin{bmatrix} \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} & 0 & 0 \\ \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} & 0 & 0 \\ -\frac{24}{5} & -\frac{L}{10} & \frac{48}{5} & 0 & -\frac{24}{5} & \frac{L}{10} \\ \frac{L}{10} & -\frac{L^2}{120} & 0 & \frac{L^2}{15} & -\frac{L}{10} & -\frac{L^2}{120} \\ 0 & 0 & -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} \\ 0 & 0 & \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} \end{bmatrix}$$

$$\frac{\partial(\delta^2 \Pi)}{\partial D_s} = 0 \longrightarrow ([K] - P[N]) \{D_s\} = 0$$

$$\frac{EI}{L^3} \begin{bmatrix} 768 & 96L & -768 & 96L & 0 & 0 \\ 96L & 16L^2 & -96L & 8L^2 & 0 & 0 \\ -768 & 96L & 1536 & 0 & -768 & 96L \\ 96L & 8L^2 & 0 & 32L^2 & -96L & 8L^2 \\ 0 & 0 & -768 & -96L & 768 & -96L \\ 0 & 0 & 96L & 8L^2 & -96L & 16L^2 \end{bmatrix} \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} - P \frac{1}{L} \begin{bmatrix} \frac{24}{5} & \frac{L}{10} & -\frac{24}{5} & \frac{L}{10} & 0 & 0 \\ \frac{L}{10} & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} & 0 & 0 \\ -\frac{24}{5} & -\frac{L}{10} & \frac{48}{5} & 0 & -\frac{24}{5} & \frac{L}{10} \\ \frac{L}{10} & -\frac{L^2}{120} & 0 & \frac{L^2}{15} & -\frac{L}{10} & -\frac{L^2}{120} \\ 0 & 0 & -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & -\frac{L}{10} \\ 0 & 0 & \frac{L}{10} & -\frac{L^2}{120} & -\frac{L}{10} & \frac{L^2}{30} \end{bmatrix} \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} = 0$$

Symmetrien

$$x=0 \rightarrow \begin{cases} w=0; D_1=0 \\ w'=0; D_2=0 \end{cases} \text{ Antikaste mesnet}$$

$$x=\frac{L}{2} \rightarrow \begin{cases} w'=0; D_6=0 \end{cases} \text{ symmetrisch}$$

$$\frac{EI}{L^3} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1536 & 0 & -768 & 0 \\ 0 & 0 & 0 & 32L^2 & -96L & 0 \\ 0 & 0 & -768 & -96L & 768 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} - P \frac{1}{L} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & \frac{48}{5} & 0 & -\frac{24}{5} & 0 \\ 0 & 0 & 0 & \frac{L^2}{15} & -\frac{L}{10} & 0 \\ 0 & 0 & -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \end{Bmatrix} = 0$$

$$\frac{EI}{L^3} (1536D_3 - 768D_5) - P \frac{1}{L} \left( \frac{48}{5} D_3 - \frac{24}{5} D_5 \right) = 0$$

$$\frac{EI}{L^3} (32L^2 D_4 - 96L D_5) - P \frac{1}{L} \left( \frac{L^2}{15} D_4 - \frac{L}{10} D_5 \right) = 0$$

$$\frac{EI}{L^3} (-768D_3 - 96L D_4 + 768D_5) - P \frac{1}{L} \left( -\frac{24}{5} D_3 - \frac{L}{10} D_4 + \frac{24}{5} D_5 \right) = 0$$



$$\left( \frac{EI}{L^3} 1536 - \frac{P}{L} \frac{48}{5} \right) D_3 - \left( 768 \frac{EI}{L^3} - \frac{24}{5} \frac{P}{L} \right) D_5 = 0$$

$$\left( 32L^2 \frac{EI}{L^3} - \frac{P}{L} \frac{L^2}{15} \right) D_4 - \left( 96L \frac{EI}{L^3} - \frac{L}{10} \frac{P}{L} \right) D_5 = 0$$

$$-\left( 768 \frac{EI}{L^3} - \frac{P}{L} \frac{24}{5} \right) D_3 - \left( 96L \frac{EI}{L^3} - \frac{L}{10} \frac{P}{L} \right) D_4 + \left( 768 \frac{EI}{L^3} - \frac{24P}{5L} \right) D_5 = 0$$

Sistemin ilk konumunun kararlı hale gelmesi ve  $\{D_5\}=0$ 'den farklı kararlı bir başka dege konumunun bulunması için yukarıdaki denklemler takımının kareyılar determinanının sıfır olması gerekir.  $(\Delta)$  determinantını sıfır yapan yük kritik yükler.

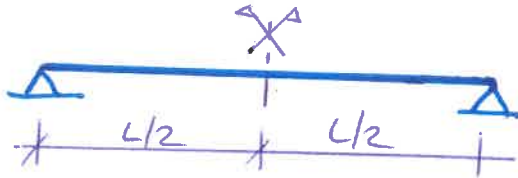
$$\Delta = \begin{vmatrix} \left( \frac{EI}{L^2} 1536 - P \frac{48}{5} \right) & 0 & -\left( 768 \frac{EI}{L^2} - P \frac{24}{5} \right) \\ 0 & \left( 32EI - \frac{PL^2}{15} \right) & -\left( 96 \frac{EI}{L} - P \frac{L}{10} \right) \\ -\left( 768 \frac{EI}{L^2} - P \frac{24}{5} \right) & -\left( 96 \frac{EI}{L} - P \frac{L}{10} \right) & \left( 768 \frac{EI}{L^2} - P \frac{24}{5} \right) \end{vmatrix} = 0$$

$$0,3P^2L^2 - 1664PEI + \frac{6144(EI)^2}{L^2} = 0$$

$$P_{kr} = \frac{39.766EI}{L^2} \quad ; \quad P_{kr} = \frac{4\pi^2 EI}{L^2} \approx \frac{39.5EI}{L^2}$$

sonlu eleman                      kesin çözüm

Sistemin 2 ucunun basit mesnetli olması halinde;



$$x=0 \rightarrow w=0 \quad (D_1=0)$$

$$x=\frac{L}{2} \rightarrow w'=0 \quad (D_6=0)$$

$$\frac{EI}{L^3} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 16L^2 & -96L & 8L^2 & 0 & 0 \\ 0 & -96L & 1536 & 0 & -768 & 0 \\ 0 & 8L^2 & 0 & 32L^2 & -96L & 0 \\ 0 & 0 & -768 & -96L & 768 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ 0 \end{bmatrix} - \frac{P}{L} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & \frac{L^2}{30} & -\frac{L}{10} & -\frac{L^2}{120} & 0 & 0 \\ 0 & -\frac{L}{10} & \frac{48}{5} & 0 & -\frac{24}{5} & 0 \\ 0 & -\frac{L^2}{120} & 0 & \frac{L^2}{15} & -\frac{L}{10} & 0 \\ 0 & 0 & -\frac{24}{5} & -\frac{L}{10} & \frac{24}{5} & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ 0 \end{bmatrix} = 0$$

$$\begin{aligned} \left(16L^2 \frac{EI}{L^2} - \frac{PL^2}{30}\right) D_2 - \left(96L \frac{EI}{L^2} - P \frac{L}{10}\right) D_3 + \left(8L^2 \frac{EI}{L^2} + P \frac{L^2}{120}\right) D_4 &= 0 \\ -\left(96L \frac{EI}{L^2} - \frac{PL}{10}\right) D_2 + \left(1536 \frac{EI}{L^2} - P \frac{48}{5}\right) D_3 - \left(768 \frac{EI}{L^2} - P \frac{24}{5}\right) D_5 &= 0 \\ \left(8L^2 \frac{EI}{L^2} + \frac{PL^2}{120}\right) D_2 + \left(32L^2 \frac{EI}{L^2} - P \frac{L^2}{15}\right) D_4 - \left(96L \frac{EI}{L^2} - P \frac{L}{10}\right) D_5 &= 0 \\ -\left(768 \frac{EI}{L^2} - \frac{P \cdot 24}{5}\right) D_3 - \left(96L \frac{EI}{L^2} - P \frac{L}{10}\right) D_4 + \left(768 \frac{EI}{L^2} - P \frac{24}{5}\right) D_5 &= 0 \end{aligned}$$

$$\begin{vmatrix} \left(16EI - \frac{PL^2}{30}\right) & -\left(96 \frac{EI}{L} - P \frac{L}{10}\right) & \left(8EI + P \frac{L^2}{120}\right) & 0 \\ -\left(96 \frac{EI}{L} - \frac{PL}{10}\right) & \left(1536 \frac{EI}{L^2} - P \frac{48}{5}\right) & 0 & -\left(768 \frac{EI}{L^2} - P \frac{24}{5}\right) \\ \left(8EI + \frac{PL^2}{120}\right) & 0 & \left(32EI - P \frac{L^2}{15}\right) - \left(96 \frac{EI}{L^2} - P \frac{L}{10}\right) & 0 \\ 0 & -\left(768 \frac{EI}{L^2} - \frac{P \cdot 24}{5}\right) & -\left(96 \frac{EI}{L} - P \frac{L}{10}\right) & \left(768 \frac{EI}{L^2} - P \frac{24}{5}\right) \end{vmatrix} = 0$$

$$P_{kr} = \frac{987EI}{L^2} \quad ; \quad P_{kr} = \frac{\pi^2 EI}{L^2} \approx \frac{986EI}{L^2}$$

sonlu eleman      kesin çözüm

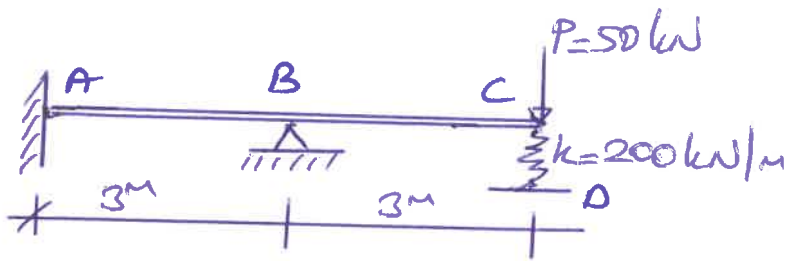
4

**ÇUBUKTA ELASTİK YAY OLMASI  
DURUMUNDA**

# Gubakta elastik yay olma durumu

E41

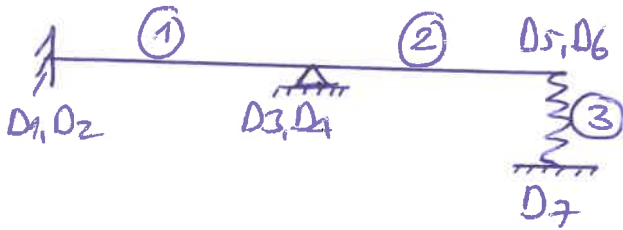
ÖRNEK



$$E = 210 \text{ GPa} = 21 \cdot 10^7 \text{ kN/m}^2$$

$$I = 2 \cdot 10^{-4} \text{ m}^4$$

$$k = 200 \text{ kN/m}$$



$$[k_1] = [k_2] = \begin{bmatrix} \frac{12EI}{L^3} & \frac{6EI}{L^2} & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ \frac{6EI}{L^2} & \frac{4EI}{L} & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{12EI}{L^3} & -\frac{6EI}{L^2} & \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ \frac{6EI}{L^2} & \frac{2EI}{L} & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix}$$

$$[k_3] = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix}$$

$$[C_1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 \end{bmatrix}$$

$$[C_3] = \begin{bmatrix} 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[C_2] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 \end{bmatrix}$$

$$[K_{e1}] = [C_1]^T [k_1] [C_1]$$

$$[K_{e2}] = [C_2]^T [k_2] [C_2]$$

$$[K_{e3}] = [C_3]^T [k_3] [C_3]$$

$$[K]_s = [K_{e1}] + [K_{e2}] + [K_{e3}]$$

| $D_1$<br>( $\psi_1$ ) | $D_2$<br>( $\psi_1$ ) | $D_3$<br>( $\psi_2$ ) | $D_4$<br>( $\psi_2$ ) | $D_5$<br>( $\psi_3$ )  | $D_6$<br>( $\psi_3$ )  | $D_7$<br>( $\psi_4$ ) |       |    |
|-----------------------|-----------------------|-----------------------|-----------------------|------------------------|------------------------|-----------------------|-------|----|
| $\frac{12EI}{L^3}$    | $\frac{6EI}{L^2}$     | $-\frac{12EI}{L^3}$   | $\frac{6EI}{L^2}$     | 0                      | 0                      | 0                     | $D_1$ | 0  |
| $\frac{6EI}{L^2}$     | $\frac{4EI}{L}$       | $-\frac{6EI}{L^2}$    | $\frac{2EI}{L}$       | 0                      | 0                      | 0                     | $D_2$ | 0  |
| $-\frac{12EI}{L^3}$   | $-\frac{6EI}{L^2}$    | $\frac{24EI}{L^3}$    | 0                     | $-\frac{12EI}{L^3}$    | $\frac{6EI}{L^2}$      | 0                     | $D_3$ | 0  |
| $\frac{6EI}{L^2}$     | $\frac{2EI}{L}$       | 0                     | $\frac{8EI}{L}$       | $-\frac{6EI}{L^2}$     | $\frac{2EI}{L}$        | 0                     | $D_4$ | 0  |
| 0                     | 0                     | $-\frac{12EI}{L^3}$   | $-\frac{6EI}{L^2}$    | $\frac{12EI}{L^3} + k$ | $-\frac{6EI}{L^2} - k$ | 0                     | $D_5$ | -P |
| 0                     | 0                     | $\frac{6EI}{L^2}$     | $\frac{2EI}{L}$       | $-\frac{6EI}{L^2}$     | $\frac{4EI}{L}$        | 0                     | $D_6$ | 0  |
| 0                     | 0                     | 0                     | 0                     | -k                     | 0                      | k                     | $D_7$ | 0  |

$$\left\{ \begin{matrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \end{matrix} \right\} = \left\{ \begin{matrix} 0 \\ 0 \\ 0 \\ 0 \\ -P \\ 0 \\ 0 \end{matrix} \right\}$$

$$\begin{bmatrix} \frac{8EI}{L} & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{6EI}{L^2} & \frac{12EI}{L^3} + k & -\frac{6EI}{L^2} \\ \frac{2EI}{L} & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} \begin{Bmatrix} D_4(\psi_2) \\ D_5(\psi_3) \\ D_6(\psi_3) \end{Bmatrix} = \begin{Bmatrix} 0 \\ -P \\ 0 \end{Bmatrix}$$

$$\frac{EI}{L^3} \begin{bmatrix} 8L^2 & -6L & 2L^2 \\ -6L & 12 + k' & -6L \\ 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} D_4 \\ D_5 \\ D_6 \end{Bmatrix} = \begin{Bmatrix} 0 \\ -P \\ 0 \end{Bmatrix} \quad k' = \frac{kL^3}{EI}$$

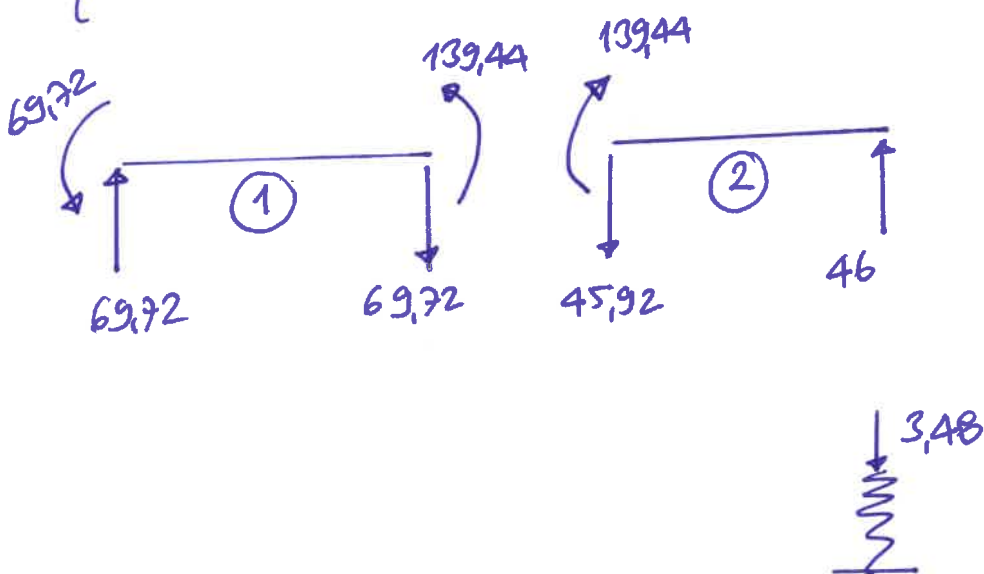
$$D_4(\psi_2) = -\frac{3PL^2}{EI} \left( \frac{1}{12 + 7k'} \right); \quad D_5(\psi_3) = -\frac{7PL^3}{EI} \left( \frac{1}{12 + 7k'} \right)$$

$$D_6(\psi_3) = -\frac{9PL^2}{EI} \left( \frac{1}{12 + 7k'} \right) \quad D_4 = -0,00249 \text{ rad.}; \quad D_5 = -0,0724 \text{ m}; \quad D_6 = -0,00247 \text{ rad.}$$

$$\begin{Bmatrix} V_A \\ M_A \\ V_{B_1} \\ M_{B_1} \end{Bmatrix} = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -0,00249 \end{Bmatrix} = \begin{Bmatrix} -69,72 \\ -69,72 \\ 69,72 \\ -139,44 \end{Bmatrix}$$

$$\begin{Bmatrix} V_{B_2} \\ M_{B_2} \\ V_C \\ M_C \end{Bmatrix} = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \begin{Bmatrix} 0 \\ -0,00249 \\ -0,0174 \\ -0,00747 \end{Bmatrix} = \begin{Bmatrix} 45,92 \\ 139,44 \\ -46 \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} V_{CD} \\ V_{DC} \end{Bmatrix} = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} -0,0174 \\ 0 \end{Bmatrix} = \begin{Bmatrix} -3,48 \\ 3,48 \end{Bmatrix}$$





EK:1A

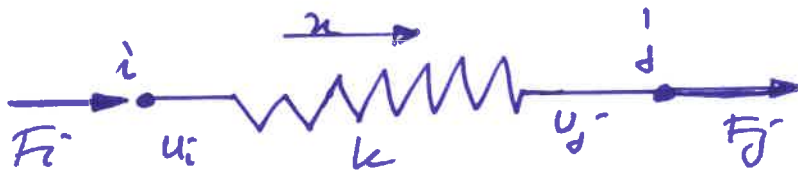
# **KAFES SİSTEMLERİN SONLU ELEMANLAR YÖNTEMİ İLE ÇÖZÜMÜ**

1. Giriş

Kafes Sistemlerin çözümünde yay elemanlarından faydalanmak sorlu elemanlarda basıtlığı yönünden tercih edilmektedir. Eleman ve sistem rijitliğini tanımlamak son derece kolay olmaktadır.

2. Yay Elemanı

Dogrusal elastik davranış gösteren yay elemanında yer değiştikmeler  $u$ , yay kuvveti  $F$ , yay rijitliği veya yay katırlığı  $k$  ise;



$$F = k(u_j - u_i) = k \cdot \Delta u$$

$$F_i + F_j = 0 \longrightarrow F_i = -F_j$$

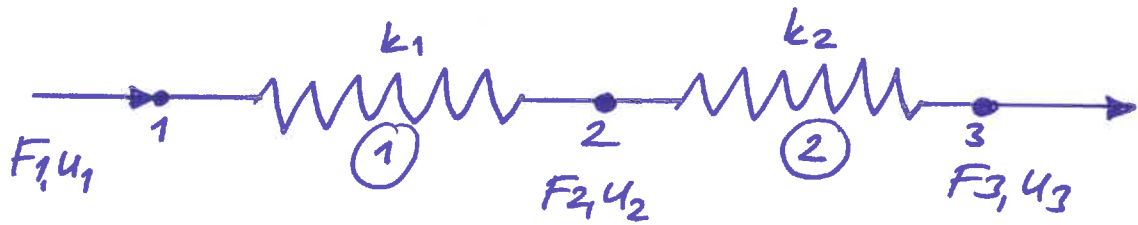
$$\left. \begin{aligned} F_i + k(u_j - u_i) &= 0 \\ F_j - k(u_j - u_i) &= 0 \end{aligned} \right\} \begin{aligned} F_i &= k u_i - k u_j \\ F_j &= -k u_i + k u_j \end{aligned}$$

veya  $[k]\{u\} = \{F\}$  dir.

$$\begin{Bmatrix} F_i \\ F_j \end{Bmatrix} = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} u_i \\ u_j \end{Bmatrix} \text{ olur.}$$

Thiti' yay duumunda de;

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$$\{u^1\} = \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} \quad \{u^2\} = \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} \quad \begin{array}{l} \text{element} \\ \text{yerdegiz} \end{array}$$

$$\{F\}^1 = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix} \quad \{F\}^2 = \begin{Bmatrix} F_2 \\ F_3 \end{Bmatrix} \quad \text{element kuvvetler}$$

$$\begin{bmatrix} k_1 & -k_1 \\ -k_1 & k_1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix}; \quad \begin{bmatrix} k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} F_2 \\ F_3 \end{Bmatrix}$$

Gesam matriskei de;

$$[C_1] = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \end{bmatrix} \quad C_2 = \begin{bmatrix} 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$[k_1] = \begin{bmatrix} 1 & 0 \\ 0 & 1 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} k_1 & -k_1 \\ -k_1 & k_1 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$[k_2] = \begin{bmatrix} 0 & 0 \\ 1 & 0 \\ 0 & 1 \end{bmatrix} \begin{bmatrix} k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{bmatrix} 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} = \begin{bmatrix} 0 & 0 & 0 \\ 0 & k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix}$$

$$[K]_s = [k_1] + [k_2] = \begin{bmatrix} k_1 & -k_1 & 0 \\ 0 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix}$$

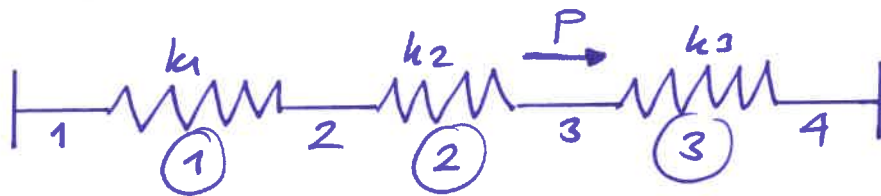
$$\{u\}_s = \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \end{Bmatrix} \quad \{F\}_s = \begin{Bmatrix} F_1 \\ F_1 + F_2 \\ F_2 \end{Bmatrix}$$

$$\{k\}_s \{u\}_s = \{F\}_s$$



### ÖRNEK:1

Şekilde verilen sistemin 2 ve 3 nolu düğüm noktalarının yerdeğiştirme miktarı, mesnet tepkilerini ve yay kuvvetlerini bulunuz.



$$k = 100 \text{ kN/m}$$

$$P = 0,5 \text{ kN}$$

$$k_1 = k_3 = \begin{bmatrix} 100 & -100 \\ -100 & 100 \end{bmatrix}$$

$$k_2 = \begin{bmatrix} 200 & -200 \\ -200 & 200 \end{bmatrix}$$

$$[C_{e1}] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \end{bmatrix} \quad [C_{e2}] = \begin{bmatrix} 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \end{bmatrix} \quad [C_{e3}] = \begin{bmatrix} 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[k]_s = \begin{bmatrix} 100 & -100 & 0 & 0 \\ -100 & 100+200 & -200 & 0 \\ 0 & -200 & 200+100 & -100 \\ 0 & 0 & -100 & 100 \end{bmatrix} = \begin{bmatrix} 100 & -100 & 0 & 0 \\ -100 & 300 & -200 & 0 \\ 0 & -200 & 300 & -100 \\ 0 & 0 & -100 & 100 \end{bmatrix}$$

$$\begin{bmatrix} 100 & -100 & 0 & 0 \\ -100 & 300 & -20 & 0 \\ 0 & -200 & 300 & -100 \\ 0 & 0 & -100 & 100 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} R_1 \\ 0 \\ P \\ R_2 \end{Bmatrix} \quad \left| \begin{array}{l} u_1 = 0 \\ u_4 = 0 \end{array} \right. \quad 4/17$$

$$\begin{bmatrix} 300 & -20 \\ -20 & 300 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 0 \\ P \end{Bmatrix} \rightarrow \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 0,002 \text{ m} \\ 0,003 \text{ m} \end{Bmatrix}$$

$$F_1 = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} \quad F_2 = \begin{bmatrix} 2k & -2k \\ -2k & 2k \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} \quad F_3 = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} u_3 \\ u_4 \end{Bmatrix}$$

$$F_1 = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix}^1 = \begin{bmatrix} 100 & -100 \\ -100 & 100 \end{bmatrix} \begin{Bmatrix} 0 \\ 0,002 \end{Bmatrix} = \begin{Bmatrix} -0,2 \\ 0,2 \end{Bmatrix} \text{ kN}$$

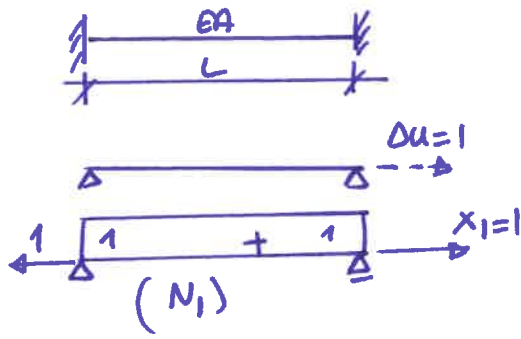
$$F_2 = \begin{Bmatrix} F_2 \\ F_3 \end{Bmatrix}^2 = \begin{bmatrix} 200 & -20 \\ -20 & 200 \end{bmatrix} \begin{Bmatrix} 0,002 \\ 0,003 \end{Bmatrix} = \begin{Bmatrix} -0,2 \\ 0,2 \end{Bmatrix} \text{ kN}$$

$$F_3 = \begin{Bmatrix} F_3 \\ F_4 \end{Bmatrix}^3 = \begin{bmatrix} 100 & -100 \\ -100 & 100 \end{bmatrix} \begin{Bmatrix} 0,003 \\ 0 \end{Bmatrix} = \begin{Bmatrix} 0,3 \\ -0,3 \end{Bmatrix} \text{ kN}$$

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# Eksenel Kuvvet Elemanı

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$$\left. \begin{aligned} \delta_{1w} &= -1.1 = -1 \\ \delta_{11} &= \frac{L.1.1}{EA} \end{aligned} \right\} \begin{aligned} \delta_{1w} + \delta_{11}.u_1 &= 0 \\ u_1 &= \frac{EA}{L} \end{aligned}$$

$F = \frac{EA}{L}$

$$\sigma = \frac{F}{A} \rightarrow F = \sigma \cdot A = E \cdot \frac{\Delta u}{L} \cdot A = \frac{EA}{L} \cdot \Delta u$$

$$\sigma = E \cdot \epsilon = E \cdot \frac{\Delta u}{L}$$

şeklinde olacaktır ve  $k = \frac{EA}{L}$  değerinin sabit bir eksenel rijitliğini temsil ettiği şekilde alınır;

$$F_i = \frac{EA}{L} (u_i - u_j) = \overset{k}{\frac{EA}{L}} u_i - \overset{k}{\frac{EA}{L}} u_j$$

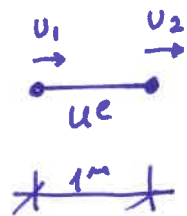
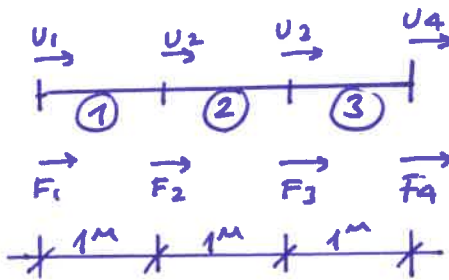
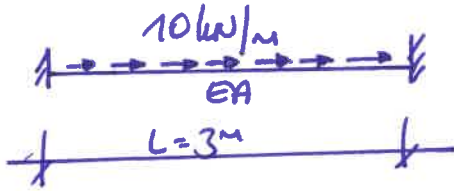
$$F_j = -\frac{EA}{L} (u_i - u_j) = -\frac{EA}{L} u_i + \frac{EA}{L} u_j$$

$$\begin{Bmatrix} F_i \\ F_j \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} u_i \\ u_j \end{Bmatrix}$$

 olur.



Sekilde gösterilen iki ucu ankastee normal kuvvet gubugun mesnet tepkilerini ve gubuk kuvvetlerini hesaplayiniz.



$$k_1 = k_2 = k_3 = EA \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[C_{e1}] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \end{bmatrix} \quad [C_{e2}] = \begin{bmatrix} 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \end{bmatrix} \quad [C_{e3}] = \begin{bmatrix} 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[K_1]^e = [C_{e1}]^T [K_1] [C_{e1}] = \frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[K_2]^e = [C_{e2}]^T [K_2] [C_{e2}] = \frac{EA}{L} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 1 & -1 & 0 \\ 0 & -1 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[K_3]^e = [C_{e3}]^T [K_3] [C_{e3}] = \frac{EA}{L} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix}$$

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$$[K]_s = \frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 2 & -1 & 0 \\ 0 & -1 & 2 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{Bmatrix} \quad \left| \begin{array}{l} u_1 = 0 \\ u_4 = 0 \end{array} \right.$$

Yerdeğiştirme Hesabı

$$\frac{EA}{L} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix} \quad \left| \begin{array}{l} F_1 = 5 \text{ kN} \\ F_2 = 10 \text{ kN} \\ F_3 = 10 \text{ kN} \\ F_4 = 5 \text{ kN} \end{array} \right.$$

$$\frac{EA}{L} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 10 \\ 10 \end{Bmatrix}$$

$$\begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 10/EA \\ 10/EA \end{Bmatrix}$$

mesnet Tepkilerinin Hesabı

$$\frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 2 & -1 & 0 \\ 0 & -1 & 2 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} 0 \\ 10/EA \\ 10/EA \\ 0 \end{Bmatrix} = \begin{Bmatrix} R_A + 5 \\ 10 \\ 10 \\ R_B + 5 \end{Bmatrix}$$

$$\frac{EA}{L} (-10/EA) = R_A + 5 \quad \left| \quad \frac{EA}{L} (-10/EA) = R_B + 5 \right.$$

$$\underline{R_A = -15 \text{ kN}} \quad \left| \quad \underline{R_B = -15 \text{ kN}} \right.$$

## Güçlük kuvvetlerinin hesabı

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$$F_1 = ku = F_e - F_q$$

$$F_1^e = \begin{Bmatrix} N_1^e \\ N_2^e \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} - \begin{Bmatrix} 5 \\ 5 \end{Bmatrix}$$

$$= \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} 0 \\ 10/EA \end{Bmatrix} - \begin{Bmatrix} 5 \\ 5 \end{Bmatrix} = \begin{Bmatrix} -15 \\ 5 \end{Bmatrix} \text{ kN}$$

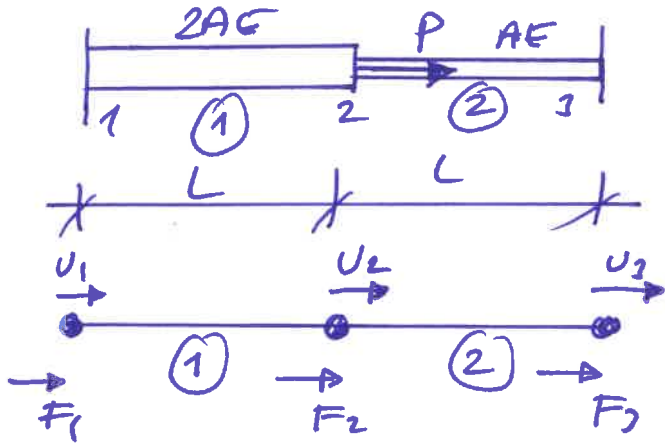
$$F_2^e = \begin{Bmatrix} N_1^e \\ N_2^e \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} 10/EA \\ 10/EA \end{Bmatrix} - \begin{Bmatrix} 5 \\ 5 \end{Bmatrix} = \begin{Bmatrix} -5 \\ -5 \end{Bmatrix}$$

$$F_3^e = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} 10/EA \\ 0 \end{Bmatrix} - \begin{Bmatrix} 5 \\ 5 \end{Bmatrix} = \begin{Bmatrix} 5 \\ -15 \end{Bmatrix}$$

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ÖRNEK 2: Yerdeğiştirmeleri hesaplayınız.

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$$k_1 = \frac{2EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$k_2 = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[C_1] = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \end{bmatrix} \quad [C_2] = \begin{bmatrix} 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$\left. \begin{aligned} [K_1]^e &= [C_1]^T [k_1] [C_1] \\ [K_2]^e &= [C_2]^T [k_2] [C_2] \end{aligned} \right\} [K]_s = [K_1]^e + [K_2]^e$$

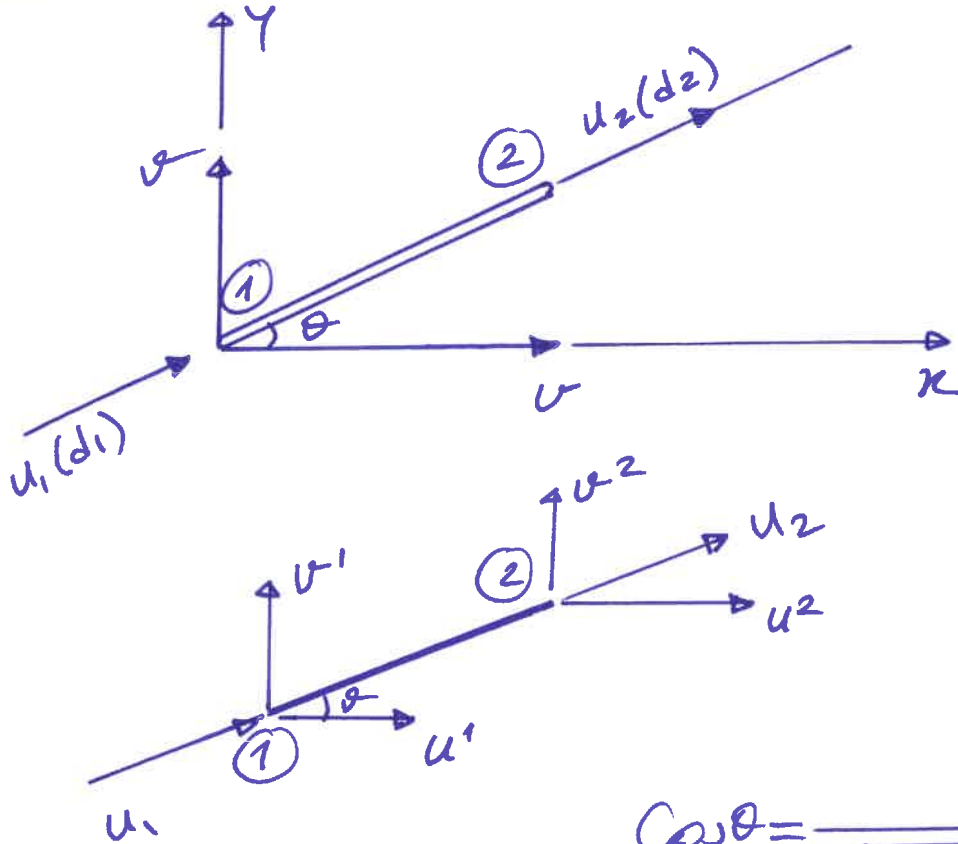
$$[K]_s = \frac{EA}{L} \begin{bmatrix} 2 & -2 & 0 \\ -2 & 3 & -1 \\ 0 & -1 & 1 \end{bmatrix}$$

Yerdeğiştirmelerin Hesabı

$$\frac{EA}{L} \begin{bmatrix} 2 & -2 & 0 \\ -2 & 3 & -1 \\ 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \end{Bmatrix} \quad \left\| \begin{aligned} u_1 &= 0 \\ u_3 &= 0 \\ F_2 &= P \end{aligned} \right.$$

$$\frac{3EA}{L} u_2 = P \rightarrow u_2 = \frac{PL}{3EA}$$

Dönüşüm Matrisi'



$$\cos \theta = \frac{x_2 - x_1}{\sqrt{(x_2 - x_1)^2 + (y_2 - y_1)^2}}$$

$$\cos \theta = \frac{x_2 - x_1}{L}$$

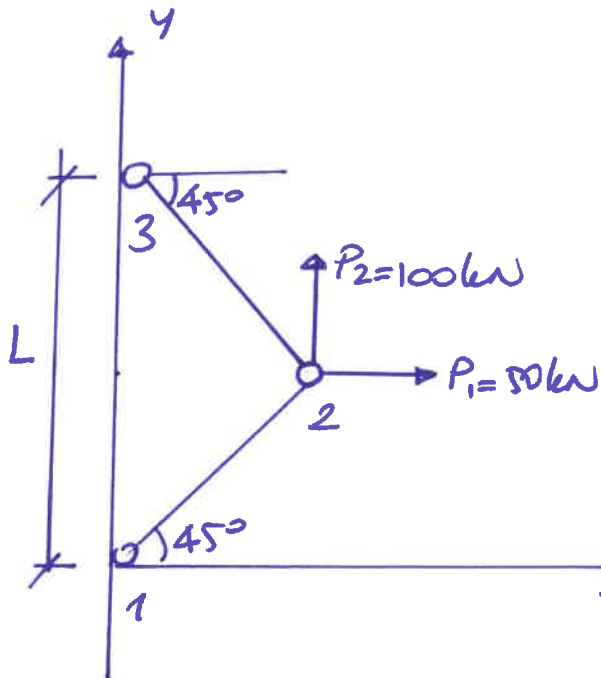
$$\sin \theta = \frac{y_2 - y_1}{\sqrt{(x_2 - x_1)^2 + (y_2 - y_1)^2}}$$

$$\sin \theta = \frac{y_2 - y_1}{L}$$

$$u_1 = u^1 \cos \theta + v^1 \sin \theta$$

$$u_2 = u^2 \cos \theta + v^2 \sin \theta$$

$$[T] = \begin{bmatrix} c & s & 0 & 0 \\ 0 & 0 & c & s \end{bmatrix}$$



$$[k_1] = [k_2] = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

| DN | X   | Y   |
|----|-----|-----|
| 1  | 0   | 0   |
| 2  | L/2 | L/2 |
| 3  | 0   | L   |

| EN | i | j | L                     | C                     | S                    |
|----|---|---|-----------------------|-----------------------|----------------------|
| 1  | 1 | 2 | $\frac{L\sqrt{2}}{2}$ | $\frac{\sqrt{2}}{2}$  | $\frac{\sqrt{2}}{2}$ |
| 2  | 2 | 3 | $\frac{L\sqrt{2}}{2}$ | $-\frac{\sqrt{2}}{2}$ | $\frac{\sqrt{2}}{2}$ |

$$[T_1] = \begin{bmatrix} \sqrt{2}/2 & \sqrt{2}/2 & 0 & 0 \\ 0 & 0 & \sqrt{2}/2 & \sqrt{2}/2 \end{bmatrix}$$

$$[T_2] = \begin{bmatrix} -\sqrt{2}/2 & \sqrt{2}/2 & 0 & 0 \\ 0 & 0 & -\sqrt{2}/2 & \sqrt{2}/2 \end{bmatrix}$$

$$k_1 = [T_1]^T [k_1] [T_1] = \frac{EA}{2} \begin{bmatrix} 1 & 1 & -1 & -1 \\ 1 & 1 & -1 & -1 \\ -1 & -1 & 1 & 1 \\ -1 & -1 & 1 & 1 \end{bmatrix}$$



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$$k_2 = [\tau_2]^T [k_2] [\tau_2] = \frac{EA}{2} \begin{bmatrix} 1 & -1 & -1 & 1 \\ -1 & 1 & 1 & -1 \\ -1 & 1 & 1 & -1 \\ 1 & -1 & -1 & 1 \end{bmatrix}$$

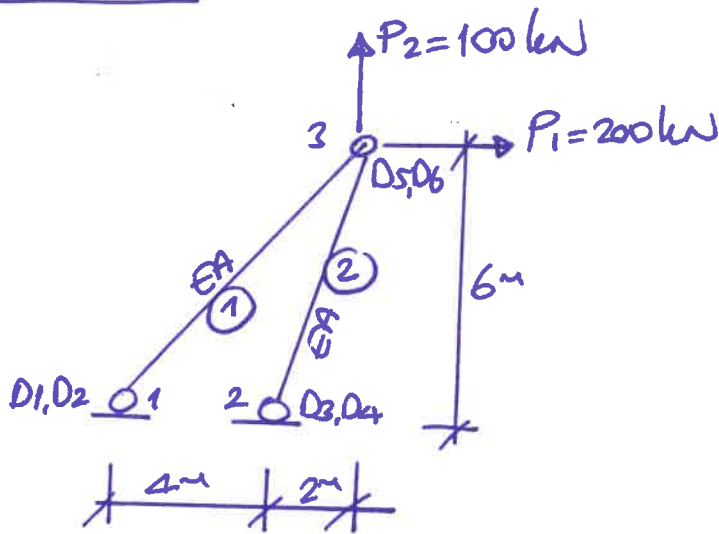
$$\frac{EA}{2L} \begin{bmatrix} 1 & 1 & -1 & -1 & 0 & 0 \\ 1 & 1 & -1 & -1 & 0 & 0 \\ -1 & -1 & 2 & 0 & -1 & 1 \\ -1 & -1 & 0 & 2 & 1 & -1 \\ 0 & 0 & -1 & 1 & 1 & -1 \\ 0 & 0 & 1 & -1 & -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \\ u_3 \\ v_3 \end{Bmatrix} = \begin{Bmatrix} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \\ F_{3x} \\ F_{3y} \end{Bmatrix}$$

$$u_1 = 0, v_1 = 0, u_3 = 0, v_3 = 0$$

$$\frac{EA}{2L} \begin{bmatrix} 2 & 0 \\ 0 & 2 \end{bmatrix} \begin{Bmatrix} u_2 \\ v_2 \end{Bmatrix} = \begin{Bmatrix} F_{2x} \\ F_{2y} \end{Bmatrix}$$

$$\begin{Bmatrix} u_2 \\ v_2 \end{Bmatrix} = \frac{L}{EA} \begin{Bmatrix} 500 \\ 100 \end{Bmatrix}$$

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$$E = 200 \text{ GPa}$$

$$A = 0,01 \text{ m}^2$$

$$L_1 = \sqrt{6^2 + 6^2} = 8,49 \text{ m.}$$

$$L_2 = \sqrt{(6-4)^2 + 6^2} = 6,32 \text{ m.}$$

| DN | X(m) | Y(m) |
|----|------|------|
| 1  | 0    | 0    |
| 2  | 4    | 0    |
| 3  | 6    | 6    |

| EN | $\bar{i}$ | $\bar{j}$ | L    | C     | S     |
|----|-----------|-----------|------|-------|-------|
| 1  | 1         | 3         | 8,49 | 0,707 | 0,707 |
| 2  | 2         | 3         | 6,32 | 0,316 | 0,949 |

$$C_1 = \cos \theta_1 = \frac{x_3 - x_1}{L_1} = \frac{6}{8,49} = 0,707$$

$$S_1 = \sin \theta_1 = \frac{y_3 - y_1}{L_1} = \frac{6}{8,49} = 0,707$$

$$C_2 = \cos \theta_2 = \frac{x_3 - x_2}{L_2} = \frac{2}{6,32} = 0,316$$

$$S_2 = \sin \theta_2 = \frac{y_3 - y_2}{L_2} = \frac{6}{6,32} = 0,949$$

$$[T_1] = \begin{bmatrix} 0,707 & 0,707 & 0 & 0 \\ 0 & 0 & 0,707 & 0,707 \end{bmatrix}$$

$$[T_2] = \begin{bmatrix} 0,316 & 0,949 & 0 & 0 \\ 0 & 0 & 0,316 & 0,949 \end{bmatrix}$$

$$[C_{e1}] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[C_{e2}] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

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$$k_1 = \frac{EA}{L_1} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$k_1 = \frac{(200 \cdot 10^9) 0,01}{8 \text{ Ag}} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$k_1 = 2357 \cdot 10^5 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$k_2 = \frac{EA}{L_2} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$= \frac{(200 \cdot 10^9) 0,01}{6,32} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$= 3162 \cdot 10^5 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[K_1] = [T_1]^T [k_1] [T_1] = 2357 \cdot 10^5 \begin{vmatrix} 0,50 & 0,50 & -0,50 & -0,50 \\ 0,50 & 0,50 & -0,50 & -0,50 \\ -0,50 & -0,50 & 0,50 & 0,50 \\ -0,50 & -0,50 & 0,50 & 0,50 \end{vmatrix}$$

$$[K_2] = [T_2]^T [k_2] [T_2] = 3162 \cdot 10^5 \begin{vmatrix} 0,10 & 0,30 & -0,10 & -0,30 \\ 0,30 & 0,90 & -0,30 & -0,90 \\ -0,10 & -0,30 & 0,10 & 0,30 \\ -0,30 & -0,90 & 0,30 & 0,90 \end{vmatrix}$$

$$[K_3]^1 = [C_1]^T [K_1] [C_1] = 2357 \cdot 10^5 \begin{vmatrix} 0,50 & 0,50 & 0 & 0 & -0,50 & -0,50 \\ 0,50 & 0,50 & 0 & 0 & -0,50 & -0,50 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -0,50 & -0,50 & 0 & 0 & 0,50 & 0,50 \\ -0,50 & -0,50 & 0 & 0 & 0,50 & 0,50 \end{vmatrix}$$

$$[K_3]^2 = [C_2]^T [K_2] [C_2] = 3162 \cdot 10^5$$

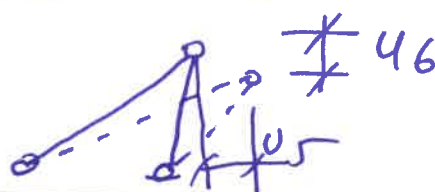
$$^{15/17} \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 9,10 & 0,30 & -0,1 & -0,3 \\ 0 & 0 & 0,3 & 0,9 & -0,3 & -0,9 \\ 0 & 0 & -0,1 & -0,3 & 0,1 & 0,3 \\ 0 & 0 & -0,3 & -0,9 & 0,3 & 0,9 \end{bmatrix}$$

$$[K_3] = [K_1] + [K_2] = 10^5 \begin{bmatrix} 1178,5 & 1178,5 & 0 & 0 & -1178,5 & -1178,5 \\ 1178,5 & 1178,5 & 0 & 0 & -1178,5 & -1178,5 \\ 0 & 0 & 3162 & 948,6 & -316,2 & -948,6 \\ 0 & 0 & 948,6 & 2845,8 & -948,6 & -2845,8 \\ -1178,5 & -1178,5 & -316,2 & -948,6 & 1494,7 & 2127,1 \\ -1178,5 & -1178,5 & -948,6 & -2845,8 & 2127,1 & 4024,3 \end{bmatrix}$$

$$[K_3] \{u\} = \{F\}$$

$$10^5 \begin{bmatrix} \cdot & \cdot & \cdot & \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot & \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot & \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot & \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot & \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot & \cdot & \cdot & \cdot \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ u_5 \\ u_6 \end{Bmatrix} = \begin{Bmatrix} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \\ F_{3x} \\ F_{3y} \end{Bmatrix} \quad \left| \begin{array}{l} u_1 = 0 \\ u_2 = 0 \\ u_3 = 0 \\ u_4 = 0 \end{array} \right.$$

$$10^5 \begin{bmatrix} 1494,7 & 2127,1 \\ 2127,1 & 4024,3 \end{bmatrix} \begin{Bmatrix} u_5 \\ u_6 \end{Bmatrix} = \begin{Bmatrix} 200 \cdot 10^3 \\ 100 \cdot 10^3 \end{Bmatrix} \Rightarrow \begin{array}{l} u_5 = 0,003573 \text{ m} \\ u_6 = -0,00187 \text{ m} \end{array}$$



## Mesnet Tepikteenin Hesabi

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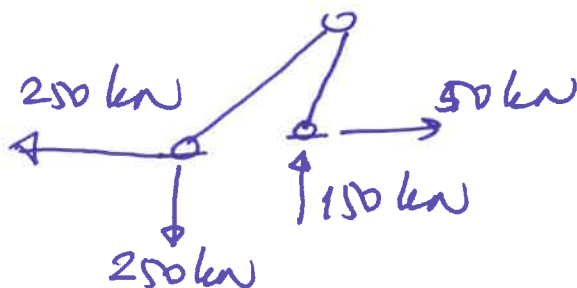
$$10^5 \begin{bmatrix} \vdots \\ \vdots \\ \vdots \\ 0,003973 \\ -0,001857 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0,003973 \\ -0,001857 \end{Bmatrix} = \begin{Bmatrix} R_{1x} \\ R_{1y} \\ R_{2x} \\ R_{2y} \\ 200 \cdot 10^3 \\ 100 \cdot 10^3 \end{Bmatrix}$$

$$10^{-3} [10^5 (-1178,5 \cdot 0,003973) - 10^5 (-1178,5 \cdot 0,001857)] = R_{1x}$$
$$R_{1x} = -250 \text{ kN}$$

$$10^{-3} [10^5 ( \text{ " } \cdot \text{ " } ) - \text{ " } ( \text{ " } \cdot \text{ " } )] = R_{1y}$$
$$R_{1y} = -250 \text{ kN}$$

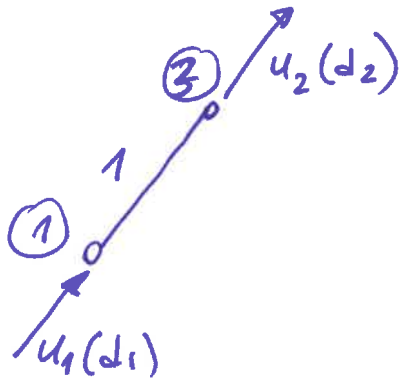
$$10^{-3} [10^5 (-316,2 \cdot 0,003973) - 10^5 (-948,6 \cdot 0,001857)] = R_{2x}$$
$$R_{2x} = 50 \text{ kN}$$

$$10^{-3} [10^5 (-948,6 \cdot \text{ " } ) - 10^5 (-2845,8 \cdot \text{ " } )] = R_{2y}$$
$$R_{2y} = 150 \text{ kN}$$



## 1 nolu elemanın lokal yerdeğiştirme hareketleri

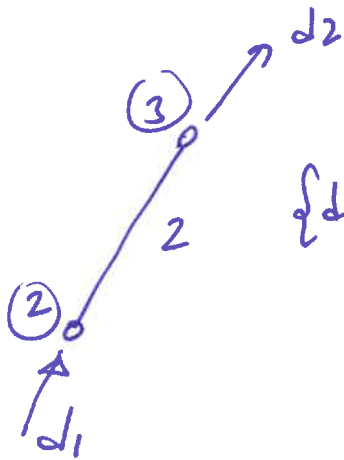
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$$\{d\}^1 = \begin{Bmatrix} d_1^1 \\ d_2^1 \end{Bmatrix} = [T_1] \{u^1\} = \begin{bmatrix} 0.707 & 0.707 & 0 & 0 \\ 0 & 0 & 0.707 & 0.707 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0.0015 \\ -0.0015 \end{Bmatrix}$$

$$= \begin{Bmatrix} 0 \\ 0.0015 \text{ m} \end{Bmatrix}$$

## 2 nolu elemanın lokal yerdeğiştirme hareketleri



$$\{d\}^2 = \begin{Bmatrix} d_1^2 \\ d_2^2 \end{Bmatrix} = [T_2] \{u^2\} = \begin{bmatrix} 0.316 & 0.949 & 0 & 0 \\ 0 & 0 & 0.316 & 0.949 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0.0005 \\ -0.00167 \end{Bmatrix}$$

$$= \begin{Bmatrix} 0 \\ -0.0005 \text{ m} \end{Bmatrix}$$

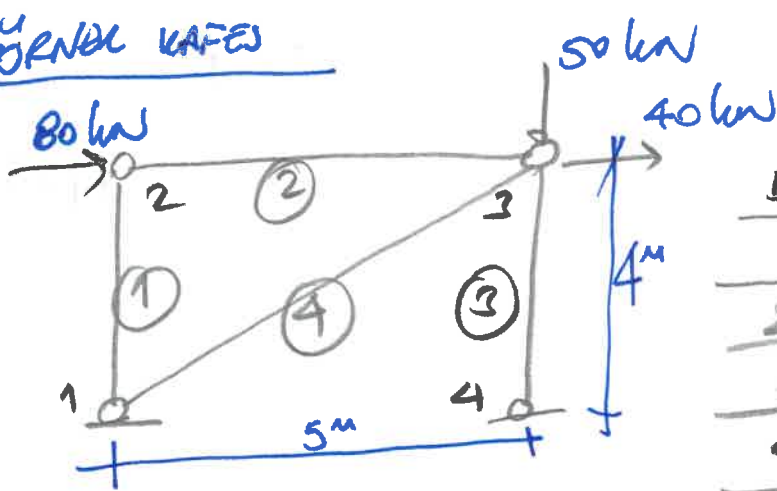
## Elemanlardaki Elemanel Kuvvetler

$$F^{(1)} = k_1 \cdot u^1 = 10^5 \begin{bmatrix} 2357 & -2357 \\ -2357 & 2357 \end{bmatrix} \begin{Bmatrix} 0 \\ 0.0015 \end{Bmatrix} = \begin{Bmatrix} -353553 \\ 353553 \end{Bmatrix} \text{ N}$$

$$F^{(2)} = k_2 \cdot u^2 = 10^5 \begin{bmatrix} 3162 & -3162 \\ -3162 & 3162 \end{bmatrix} \begin{Bmatrix} 0 \\ -0.0005 \end{Bmatrix} = \begin{Bmatrix} 158114 \\ -158114 \end{Bmatrix} \text{ N}$$



# ÖRNEK KAFES



| D.N | X | Y |
|-----|---|---|
| 1   | 0 | 0 |
| 2   | 0 | 4 |
| 3   | 5 | 4 |
| 4   | 5 | 0 |

| E.N | (L) | (R) | L     | C    | S     |
|-----|-----|-----|-------|------|-------|
| 1   | 1   | 2   | 4     | 0    | 1     |
| 2   | 2   | 3   | 5     | 1    | 0     |
| 3   | 3   | 4   | 4     | 0    | -1    |
| 4   | 1   | 3   | 6,403 | 0,78 | 0,624 |

$$[k_1] = \begin{bmatrix} 2 \cdot 10^6 & -2 \cdot 10^6 \\ -2 \cdot 10^6 & 2 \cdot 10^6 \end{bmatrix} \quad [k_2] = 10^5 \begin{bmatrix} 8 & -8 \\ -8 & 8 \end{bmatrix} \quad [k_3] = 10^6 \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix}$$

$$[k_4] = \begin{bmatrix} 624 & -624 \\ -611 & +4 \end{bmatrix}$$

$$[t_1] = \begin{bmatrix} 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} \quad [t_2] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 \end{bmatrix} \quad [t_3] = \begin{bmatrix} 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & -1 \end{bmatrix}$$

$$[t_4] = \begin{bmatrix} 0,78 & 0,624 & 0 & 0 \\ 0 & 0 & 0,78 & 0,624 \end{bmatrix}$$

$$[C_1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix} \quad [C_2] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

$$[C_3] = \begin{bmatrix} 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \quad [C_4] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

K1/A

$$[K_2] = 10^6$$

K2/A

|         |         |      |    |         |         |         |   |   |   |    |     |
|---------|---------|------|----|---------|---------|---------|---|---|---|----|-----|
| 0,3809  | 0,2047  | 0    | 0  | 0       | -0,3809 | -0,3047 | 0 | 0 | 0 | D1 | 0   |
| 0,3047  | 2,2438  | 0    | -2 | -0,3047 | -0,2438 | 0       | 0 | 0 | 0 | D2 | 0   |
| 0       | 0       | 0,8  | 0  | -0,8    | 0       | 0       | 0 | 0 | 0 | D3 | 80  |
| 0       | -2      | 0    | 2  | 0       | 0       | 0       | 0 | 0 | 0 | D4 | 0   |
| -0,3809 | -0,3047 | -0,8 | 0  | 1,1809  | 0,3047  | 0       | 0 | 0 | 0 | D5 | 40  |
| -0,3047 | -0,2438 | 0    | 0  | 0,3047  | 2,2438  | -2      | 0 | 0 | 0 | D6 | -50 |
| 0       | 0       | 0    | 0  | 0       | 0       | 0       | 0 | 0 | 0 | D7 | 0   |
| 0       | 0       | 0    | 0  | 0       | -2      | 0       | 2 | 0 | 0 | D8 | 0   |

$$D3 = 0,4734 \times 10^{-3}$$

$$D4 = 0$$

$$D5 = 0,3734 \times 10^{-3}$$

$$D6 = -0,0730 \times 10^{-3}$$

# Lokal yordamr formuler

$$\{d\}^1 = \begin{bmatrix} 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0,4734 \cdot 10^{-3} \\ 0 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$$

$$\{d\}^2 = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 \end{bmatrix} \begin{Bmatrix} 0,4734 \cdot 10^{-3} \\ 0 \\ 0,3734 \cdot 10^{-3} \\ -0,0730 \cdot 10^{-3} \end{Bmatrix} = \begin{Bmatrix} 0,4734 \cdot 10^{-3} \\ 0,3734 \cdot 10^{-3} \end{Bmatrix}$$

$$\{d\}^3 = \begin{bmatrix} 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & -1 \end{bmatrix} \begin{Bmatrix} 0,3734 \cdot 10^{-3} \\ 0 \\ -0,0730 \cdot 10^{-3} \\ 0 \end{Bmatrix} = \begin{Bmatrix} +0,0730 \cdot 10^{-3} \\ 0 \end{Bmatrix}$$

$$\{d\}^4 = \begin{bmatrix} 0,7809 & 0,6247 & 0 & 0 \\ 0 & 0 & 0,7809 & 0,6247 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0,3734 \cdot 10^{-3} \\ -0,0730 \cdot 10^{-3} \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0,246 \cdot 10^{-3} \end{Bmatrix}$$

## Gesamte KVV resultate

$$F_1 = 10^6 \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \end{Bmatrix} = \begin{Bmatrix} 0 \end{Bmatrix}$$

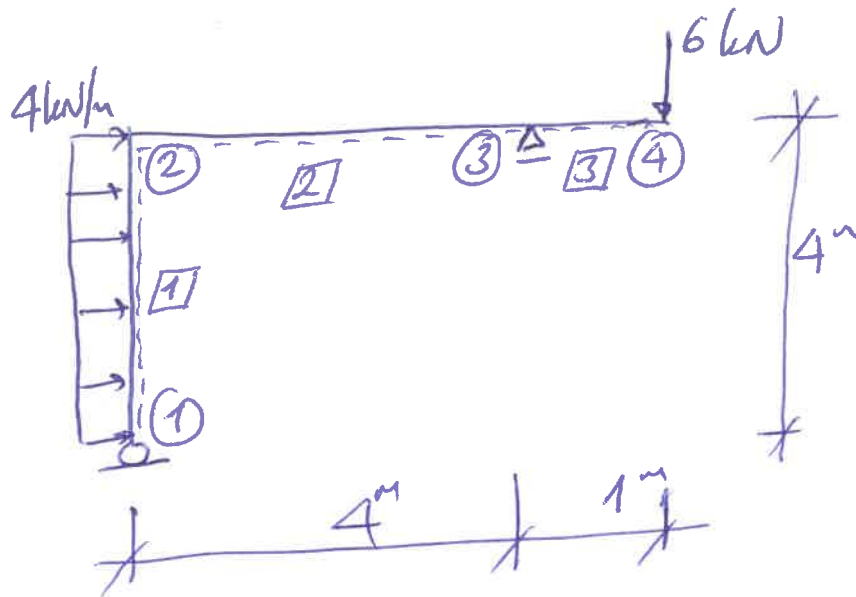
$$F_2 = 10^5 \begin{bmatrix} 8 & -8 \\ -8 & 8 \end{bmatrix} \begin{Bmatrix} 0,4734 \cdot 10^{-3} \\ 0,3734 \cdot 10^{-3} \end{Bmatrix} = \begin{Bmatrix} 80 \\ -80 \end{Bmatrix}$$

$$F_3 = 10^6 \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix} \begin{Bmatrix} 0,0720 \cdot 10^{-3} \\ 0 \end{Bmatrix} = \begin{Bmatrix} 146 \\ -146 \end{Bmatrix}$$

$$F_4 = \begin{bmatrix} 62400 & -62400 \\ -62400 & 62400 \end{bmatrix} \begin{Bmatrix} 0 \\ 0,246 \cdot 10^{-3} \end{Bmatrix} = \begin{Bmatrix} -153,678 \\ +153,678 \end{Bmatrix}$$

EK:1B

## ÇERÇEVELERİN SONLU ELEMANLAR YÖNTEMİ İLE ÇÖZÜMÜ



2-4 elem



$$I = 16 \cdot 10^4 \text{ cm}^4$$

$$A = 120 \text{ cm}^2$$

$$E = 21 \cdot 10^6 \text{ N/cm}^2$$

$$EA_2 = EA_3 = 252 \cdot 10^{10} \text{ N}$$

$$EI_2 = EI_3 = 3,36 \cdot 10^{12} \text{ Ncm}$$

1-2 elem



$$I = 72 \cdot 10^4 \text{ cm}^4$$

$$A = 2400 \text{ cm}^2$$

$$E = 21 \cdot 10^6 \text{ N/cm}^2$$

$$EA_1 = 5,04 \cdot 10^{10} \text{ N}$$

$$EI_1 = 1,512 \cdot 10^{13} \text{ Ncm}$$

| D.N | X (cm) | Y (cm) |
|-----|--------|--------|
| 1   | 0      | 0      |
| 2   | 0      | 400    |
| 3   | 400    | 400    |
| 4   | 500    | 400    |

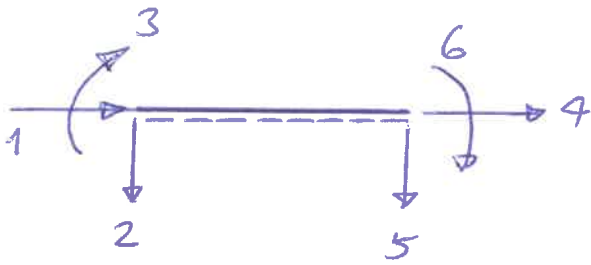
| E.N | l | r | li (cm) | c | s |
|-----|---|---|---------|---|---|
| 1   | 1 | 2 | 400     | 0 | 1 |
| 2   | 2 | 3 | 400     | 1 | 0 |
| 3   | 3 | 4 | 100     | 1 | 0 |

$$C_1 = \cos \theta_1 = \frac{x_2 - x_1}{L_1} = \frac{0}{400} = 0; \quad S_1 = \sin \theta_1 = \frac{y_2 - y_1}{L_1} = \frac{400}{400} = 1$$

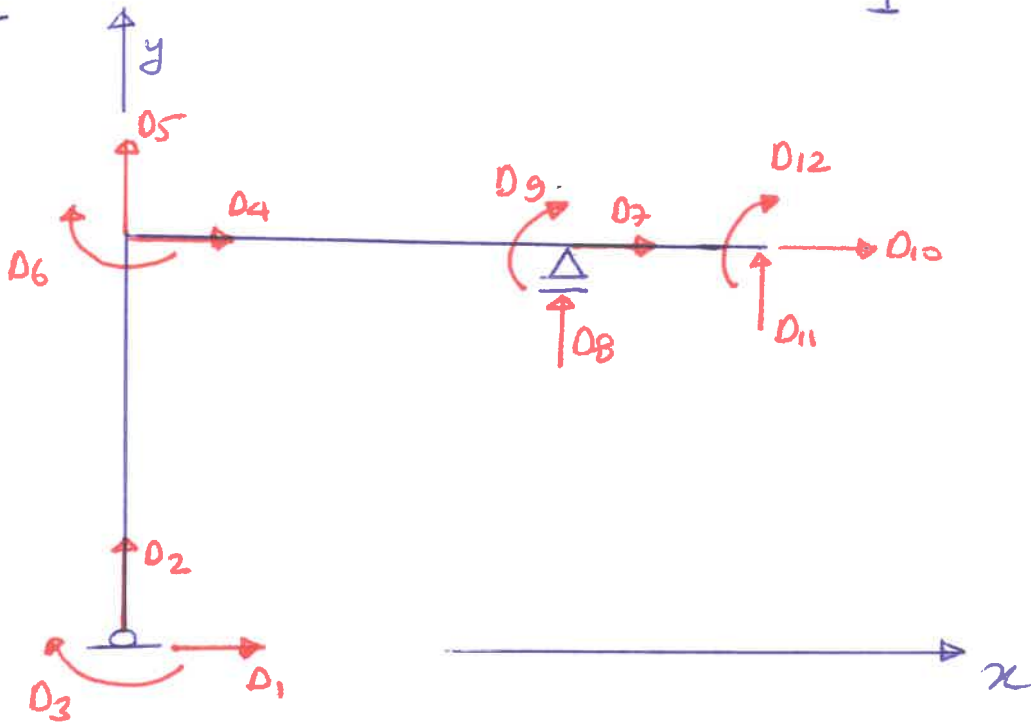
$$C_2 = \cos \theta_2 = \frac{x_3 - x_2}{L_2} = \frac{400 - 0}{400} = 1; \quad S_2 = \sin \theta_2 = \frac{y_3 - y_2}{L_2} = \frac{400 - 400}{400} = 0$$

$$C_3 = \cos \theta_3 = \frac{x_4 - x_3}{L_3} = \frac{500 - 400}{100} = 1; \quad S_3 = \sin \theta_3 = \frac{y_4 - y_3}{L_3} = \frac{400 - 400}{100} = 0$$





$$[k_e] = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{12EI}{L^3} & \frac{6EI}{L^2} & 0 & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{4EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{12EI}{L^3} & -\frac{6EI}{L^2} & 0 & \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{2EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix}$$



Sistem serbestlikleri

$$[k_1] = 10^6 \begin{bmatrix} 126 & 0 & 0 & -126 & 0 & 0 \\ 0 & 2835 & 567 & 0 & -2835 & 567 \\ 0 & 567 & 151200 & 0 & -567 & 75600 \\ -126 & 0 & 0 & 126 & 0 & 0 \\ 0 & -2835 & -567 & 0 & 2835 & -567 \\ 0 & 567 & 75600 & 0 & -567 & 151200 \end{bmatrix}$$

$$[k_2] = 10^6 \begin{bmatrix} 63 & 0 & 0 & -63 & 0 & 0 \\ 0 & 0,63 & 126 & 0 & -0,63 & 126 \\ 0 & 126 & 33600 & 0 & -126 & 16800 \\ -63 & 0 & 0 & 63 & 0 & 0 \\ 0 & -0,63 & -126 & 0 & 0,63 & -126 \\ 0 & 126 & 16800 & 0 & -126 & 33600 \end{bmatrix}$$

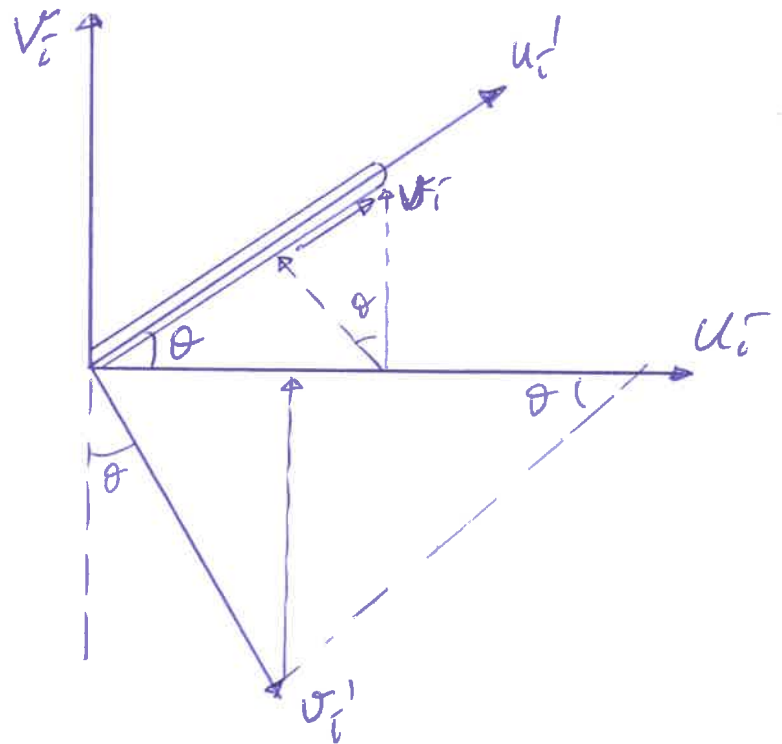
$$[k_3] = 10^6 \begin{bmatrix} 252 & 0 & 0 & -252 & 0 & 0 \\ 0 & 4032 & 2016 & 0 & -4032 & 2016 \\ 0 & 2016 & 134400 & 0 & -2016 & 67200 \\ -252 & 0 & 0 & 252 & 0 & 0 \\ 0 & -4032 & -2016 & 0 & 4032 & -2016 \\ 0 & 2016 & 67200 & 0 & -2016 & 134400 \end{bmatrix}$$

$$[T] = \begin{bmatrix} c & s & 0 & 0 & 0 & 0 \\ s & -c & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[T_1] = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[T_2] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[T_3] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$



$$[k_{e1}] = [T_1]^T [k_1] [T_1]$$

$$[k_{e2}] = [T_2]^T [k_2] [T_2]$$

$$[k_{e3}] = [T_3]^T [k_3] [T_3]$$

$$U_i' = U_i \cos \theta + V_i \sin \theta$$

$$V_i' = U_i \sin \theta - V_i \cos \theta$$

$$[C_{e1}] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[C_{e2}] = \begin{bmatrix} 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[C_{e3}] = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[k_e]^1 = [C_{e1}]^T [k_{e1}] [C_{e1}]$$

$$[k_e]^2 = [C_{e2}]^T [k_{e2}] [C_{e2}]$$

$$[k_e]^3 = [C_{e3}]^T [k_{e3}] [C_{e3}]$$

$$\{Q\}^T = \left\{ \begin{matrix} D_1 & D_2 & D_3 & D_4 & D_5 & D_6 & D_7 & D_8 & D_9 & D_{10} \\ \frac{qL}{2} + R_{AH}, & 0 + R_{AV}, & + \frac{qL^2}{12}, & \frac{qL}{2}, & 0, & -\frac{qL^2}{12}, & 0, & R_{BV}, & 0, & 0, \\ D_{11} & D_{12} \\ -6, & 0 \end{matrix} \right\}$$

$$= \{ 8 + R_{AH}, R_{AV}, +53333, 8, 0, -53333, 0, R_{BV}, 0, 0, -6, 0 \}$$

(5)

$$[ke_1] = 10^6$$

$$[ke_1] = [T_1]^T [k_1] [T_1]$$

|        |      |        |        |      |        |
|--------|------|--------|--------|------|--------|
| 2.835  | 0    | 567    | -2.835 | 0    | 567    |
| 0      | 126  | 0      | 0      | -126 | 0      |
| 567    | 0    | 151200 | -567   | 0    | 75600  |
| -2.835 | 0    | -567   | 2.835  | 0    | -567   |
| 0      | -126 | 0      | 0      | 126  | 0      |
| 567    | 0    | 75600  | -567   | 0    | 151200 |

$$[ke_2] = 10^6$$

$$[ke_2] = [T_2]^T [k_2] [T_2]$$

|     |       |       |     |       |       |
|-----|-------|-------|-----|-------|-------|
| 63  | 0     | 0     | -63 | 0     | 0     |
| 0   | 0.63  | -126  | 0   | -0.63 | -126  |
| 0   | -126  | 33600 | 0   | 126   | 16800 |
| -63 | 0     | 0     | 63  | 0     | 0     |
| 0   | -0.63 | 126   | 0   | 0.63  | 126   |
| 0   | -126  | 16800 | 0   | 126   | 33600 |

$$[ke_3] = 10^6$$

$$[ke_3] = [T_3]^T [k_3] [T_3]$$

|      |        |        |      |        |        |
|------|--------|--------|------|--------|--------|
| 252  | 0      | 0      | -252 | 0      | 0      |
| 0    | 40.32  | -2016  | 0    | -40.32 | -2016  |
| 0    | -2016  | 134400 | 0    | 2016   | 67200  |
| -252 | 0      | 0      | 252  | 0      | 0      |
| 0    | -40.32 | 2016   | 0    | 40.32  | 2016   |
| 0    | -2016  | 67200  | 0    | 2016   | 134400 |

$$[ke]^1 = [ce_1]^T [ke_1] [ce_1]$$

$$[ke]^2 = [ce_2]^T [ke_2] [ce_2]$$

$$[ke]^3 = [ce_3]^T [ke_3] [ce_3]$$

Birimler Ncm, ve N' dur.

$$[Ksis] = [ke]^1 + [ke]^2 + [ke]^3$$

$[is] = 10^6$

|        |      |        |        |        |        |      |        |        |      |       |        |   |
|--------|------|--------|--------|--------|--------|------|--------|--------|------|-------|--------|---|
| 2.8350 | 0    | 567    | -2.835 | 0      | 567    | 0    | 0      | 0      | 0    | 0     | 0      | 0 |
| 0      | -126 | 0      | 0      | -126   | 0      | 0    | 0      | 0      | 0    | 0     | 0      | 0 |
| 567    | 0    | 151200 | -567   | 0      | 75600  | 0    | 0      | 0      | 0    | 0     | 0      | 0 |
| -2.835 | 0    | -567   | 65.835 | 0      | -567   | -63  | 0      | 0      | 0    | 0     | 0      | 0 |
| 0      | -126 | 0      | 0      | 126.63 | -126   | 0    | -0.63  | -126   | 0    | 0     | 0      | 0 |
| 567    | 0    | 75600  | -567   | -126   | 184800 | 0    | 126    | 16800  | 0    | 0     | 0      | 0 |
| 0      | 0    | 0      | -63    | 0      | 0      | 315  | 0      | 0      | -252 | 0     | 0      | 0 |
| 0      | 0    | 0      | 0      | -0.63  | 126    | 0    | 40.95  | 1890   | 0    | 40.32 | 2020   | 0 |
| 0      | 0    | 0      | 0      | -126   | 16800  | 0    | -1890  | 168000 | 0    | 2016  | 67200  | 0 |
| 0      | 0    | 0      | 0      | 0      | 0      | -252 | 0      | 0      | 252  | 0     | 0      | 0 |
| 0      | 0    | 0      | 0      | 0      | 0      | 0    | -40.32 | 2016   | 0    | 40.32 | 2016   | 0 |
| 0      | 0    | 0      | 0      | 0      | 0      | 0    | -2016  | 67200  | 0    | 2016  | 134400 | 0 |

$$\{Q\} = \begin{Bmatrix} \frac{qL}{2} + R_{AH} \\ 0 + R_{AV} \\ -\frac{qL^2}{12} \\ \frac{qL}{2} \\ 0 \\ \frac{qL^2}{12} \\ 0 \\ R_{BV} \\ 0 \\ 0 \\ -6 \\ 0 \end{Bmatrix} = \begin{Bmatrix} 800 + R_{AH} \\ R_{AV} \\ 533333.33 \\ 80000 \\ 0 \\ -533333.33 \\ 0 \\ R_{BV} \\ 0 \\ 0 \\ -6000 \\ 0 \end{Bmatrix}$$

$[K]\{Q\} = \{F\}$  'den; [indirgenmemiş] [matrisler]

$$(10^6) (567 \times 0,000171705) + (-2835 \times 0,060216491) + (567 \times 0,000115268) = 8000 + R_{AH}$$

$$R_{AH} = -16000 \text{ N}$$

$$(10^6) (-126 \times 7,53968E-05) = R_{AV} \rightarrow R_{AV} = -9500 \text{ N}$$

$$(10^6) (-0,3 \times 7,53968E-05) + (126 \times 0,000115268) + (1890 \times 3,9494E-05) - (40,32 \times 0,003354167) + 2020 \times 3,0565E-05 = 15500 \text{ N} = R_{BV}$$

Tüm sistemin indirgenmemiş rijitlik matrisi hesabından  $R_{AV} = -9500 \text{ N}$ ,  $R_{AH} = -16000 \text{ N}$ ,  $R_{BV} = 15500 \text{ N}$  bulunmuştur.



$$\{D\} \text{ veya } \{U\} = [K_{sis}]^{-1} * \{Q\}$$

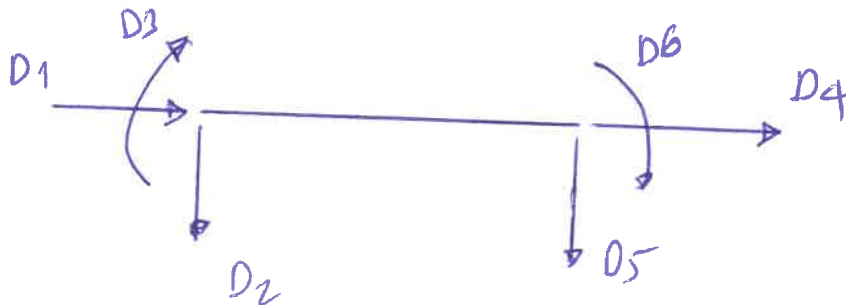
$$\{D\} \text{ veya } \{U\} = \begin{Bmatrix} U1 \\ U2 \\ U3 \\ U4 \\ U5 \\ U6 \\ U7 \\ U8 \\ U9 \\ U10 \\ U11 \\ U12 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0.000171705 \\ 0.060216491 \\ 7.53968E-05 \\ 0.000115268 \\ 0.060216491 \\ 0 \\ -3.9494E-05 \\ 0.060216491 \\ 0.003354167 \\ -3.0565E-05 \end{Bmatrix} \begin{Bmatrix} - \\ - \\ \text{radyan} \\ \text{cm} \\ \text{cm} \\ \text{radyan} \\ \text{cm} \\ - \\ \text{radyan} \\ \text{cm} \\ \text{cm} \\ \text{radyan} \end{Bmatrix}$$

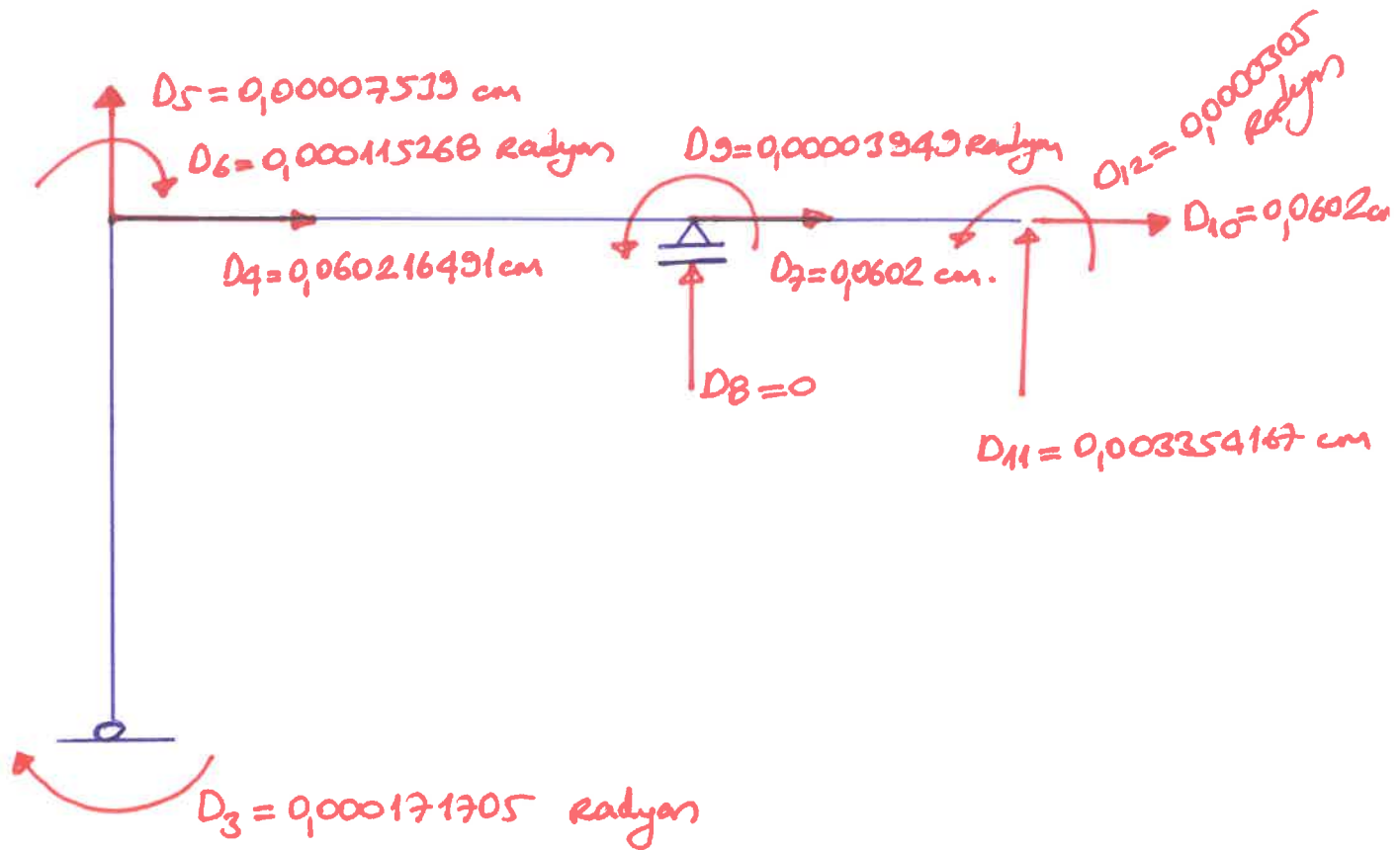
### Yerel Koordinatlarda Yerdeğiřtirmeler

$$U'_1 = [T_1] * \{U\} = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0.000171705 \\ 0.060216491 \\ 7.53968E-05 \\ -0.000115268 \end{Bmatrix} = \begin{Bmatrix} D1 \\ D2 \\ D3 \\ D4 \\ D5 \\ D6 \end{Bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0.000171705 \\ 7.53968E-05 \\ 0.060216491 \\ 0.000115268 \end{Bmatrix}$$

$$U'_2 = [T_2] * \{U\} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0.060216491 \\ 7.53968E-05 \\ -0.000115268 \\ 0.060216491 \\ 0 \\ 3.9494E-05 \end{Bmatrix} = \begin{Bmatrix} D1 \\ D2 \\ D3 \\ D4 \\ D5 \\ D6 \end{Bmatrix} \begin{Bmatrix} 0.060216491 \\ -7.53968E-05 \\ 0.000115268 \\ 0.060216491 \\ 0 \\ -0.39494E-05 \end{Bmatrix}$$

$$U'_3 = [T_3] * \{U\} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0.060216491 \\ 0 \\ 3.9494E-05 \\ 0.06021491 \\ 0.003354167 \\ -3.0565E-05 \end{Bmatrix} = \begin{Bmatrix} D1 \\ D2 \\ D3 \\ D4 \\ D5 \\ D6 \end{Bmatrix} \begin{Bmatrix} 0.060216491 \\ 0 \\ -3.9494E-05 \\ 0.060216491 \\ -0.003354167 \\ -3.05655E-05 \end{Bmatrix}$$





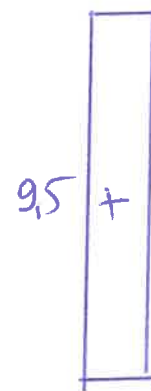
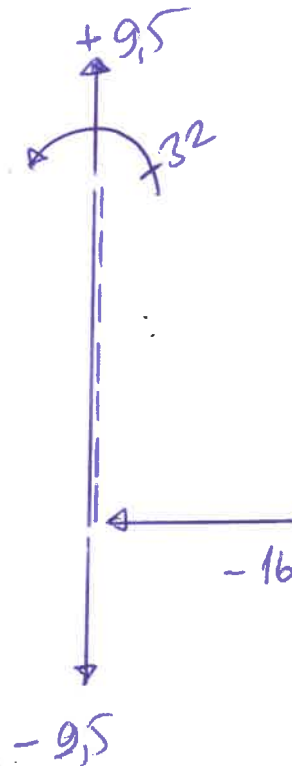
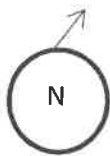
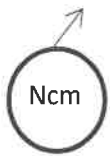
## Kesit Tesirleri

$$\{f_1\} + \{Q_1\} = \begin{Bmatrix} N_1^1 \\ V_1^1 \\ M_1^1 \\ N_2^1 \\ V_2^1 \\ M_2^1 \end{Bmatrix} + \begin{Bmatrix} 0 \\ 8000 \\ 533333.33 \\ 0 \\ 8000 \\ -533333.33 \end{Bmatrix} = [k_1] * \{U_1'\}$$

$$\{F\}_e = [k]_e \{D\}_e - \{Q\}_e$$

$$= 10^6 \begin{bmatrix} 126 & 0 & 0 & -126 & 0 & 0 \\ 0 & 2.835 & 567 & 0 & -2.835 & 567 \\ 0 & 567 & 151200 & 0 & -567 & 75600 \\ -126 & 0 & 0 & 126 & 0 & 0 \\ 0 & -2.835 & -567 & 0 & 2.835 & -567 \\ 0 & 567 & 75600 & 0 & -567 & 151200 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0.000171705 \\ 7.53968E-05 \\ 0.060216491 \\ 0.000115268 \end{Bmatrix} * 10^{-3} + \begin{Bmatrix} 0 \\ -8000 \\ -533333.33 \\ 0 \\ -8000 \\ 533333.33 \end{Bmatrix} =$$

$$\begin{Bmatrix} -9.5 \\ -8 \\ 533.333 \\ 9.5 \\ 8 \\ -3733333.33 \end{Bmatrix} + \begin{Bmatrix} 0 \\ -8 \\ -533.333 \\ 0 \\ -8 \\ 533333.33 \end{Bmatrix} = \begin{Bmatrix} -9.5 \\ -16 \\ 0 \\ 9.5 \\ 0 \\ -32 \end{Bmatrix} \begin{matrix} kN \\ kN \\ kNm \\ kN \\ kN \\ kNm \end{matrix} \begin{Bmatrix} N1 \\ V1 \\ M1 \\ N2 \\ V2 \\ M2 \end{Bmatrix}$$



(N)



(V)

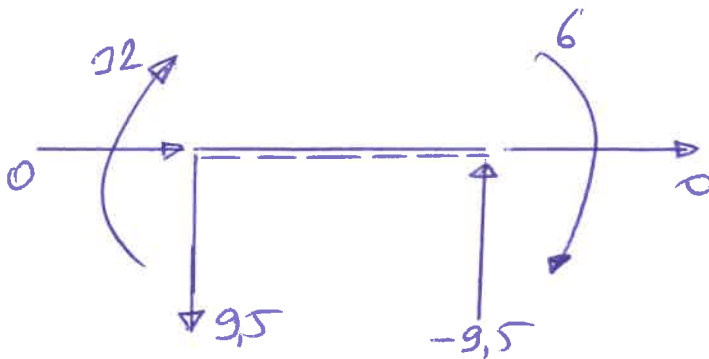


(M)

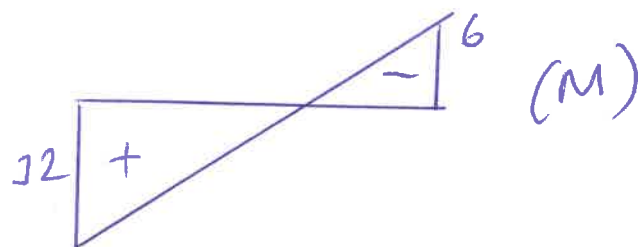
$$\{f_2\} + \{Q_2\} = \begin{Bmatrix} N_1^2 \\ V_1^2 \\ M_1^2 \\ N_2^2 \\ V_2^2 \\ M_2^2 \end{Bmatrix} + \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} = [k_2] * \{U_2'\}$$

$$= 10^6 \begin{bmatrix} 63 & 0 & 0 & -63 & 0 & 0 \\ 0 & 0.63 & 126 & 0 & -0.63 & 126 \\ 0 & 126 & 33600 & 0 & -126 & 16800 \\ -63 & 0 & 0 & 63 & 0 & 0 \\ 0 & -0.63 & -126 & 0 & 0.63 & -126 \\ 0 & 126 & 16800 & 0 & -126 & 33600 \end{bmatrix} \begin{Bmatrix} 0.060216491 \\ -7.53968E-05 \\ 0.000115268 \\ 0.060216491 \\ 0 \\ -3.9494E-05 \end{Bmatrix} * 10^{-3} - \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} =$$

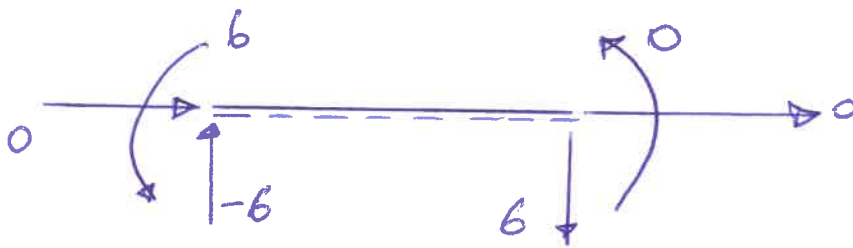
$$= \begin{Bmatrix} 0 \\ 9.5 \\ 32 \\ 0 \\ -9.5 \\ 6 \end{Bmatrix} \begin{matrix} kN \\ kN \\ kNm \\ kN \\ kN \\ kNm \end{matrix} \begin{Bmatrix} N1 \\ V1 \\ M1 \\ N2 \\ V2 \\ M2 \end{Bmatrix}$$



—○— (N)



$$\begin{aligned}
 \{f_3\} + \{Q_3\} &= \begin{Bmatrix} N_1^3 \\ V_1^3 \\ M_1^3 \\ N_2^3 \\ V_2^3 \\ M_2^3 \end{Bmatrix} + \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} = [k_3] * \{U_3'\} \\
 &= 10^6 \begin{bmatrix} 252 & 0 & 0 & -252 & 0 & 0 \\ 0 & 40.32 & 2016 & 0 & -40.32 & 2016 \\ 0 & 2016 & 134400 & 0 & -2016 & 67200 \\ -252 & 0 & 0 & 252 & 0 & 0 \\ 0 & -40.32 & -2016 & 0 & 40.32 & -2016 \\ 0 & 2016 & 67200 & 0 & -2016 & 134400 \end{bmatrix} \begin{Bmatrix} 0.060216491 \\ 0 \\ -3.9494E-05 \\ 0.060216491 \\ -0.003354167 \\ -3.05655E-05 \end{Bmatrix} * 10^{-3} - \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \\
 &= \begin{Bmatrix} 0 \\ -6 \\ -6 \\ 0 \\ 6 \\ 0 \end{Bmatrix} \begin{matrix} kN \\ kN \\ kNm \\ kN \\ kN \\ kNm \end{matrix} \begin{Bmatrix} N1 \\ V1 \\ M1 \\ N2 \\ V2 \\ M2 \end{Bmatrix}
 \end{aligned}$$

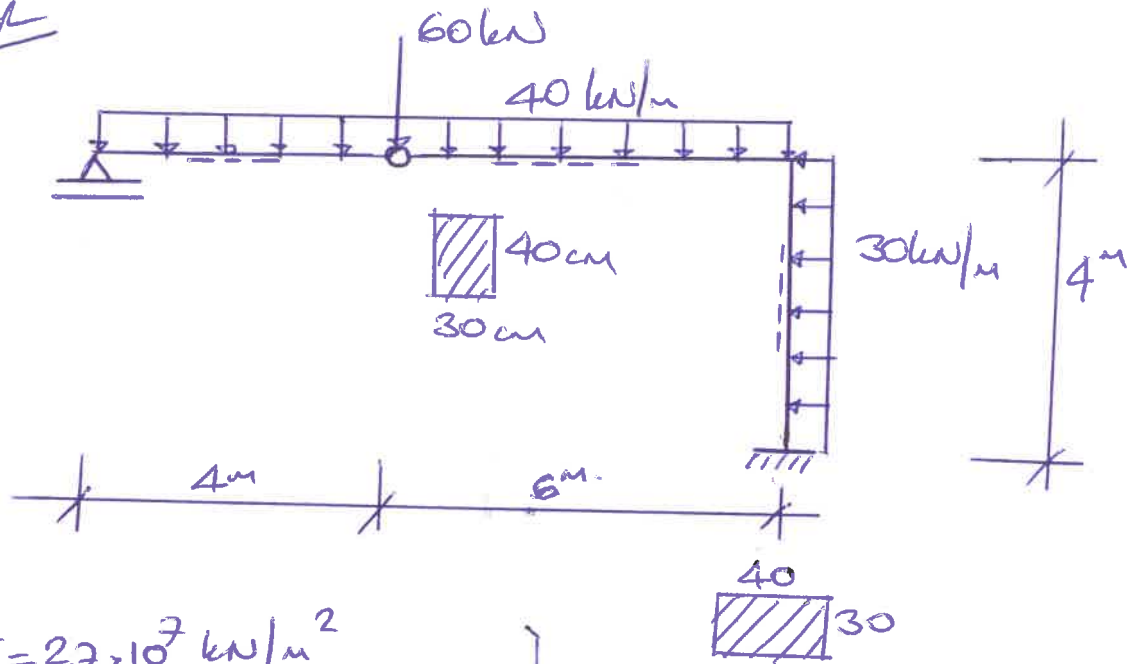


— 0 — (N)

6  
+  
(V)

6  
-  
(M)

ÖRNEK



$$E = 2,7 \cdot 10^7 \text{ kN/m}^2$$

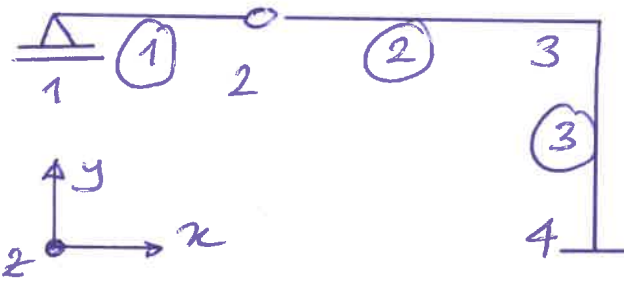
$$I = \frac{0,30 \cdot 0,40^3}{12} = 1,6 \cdot 10^{-3} \text{ m}^4$$

$$A = 0,12 \text{ m}^2$$

$$EA = 324 \cdot 10^4 \text{ kN}$$

$$EI = 4,32 \cdot 10^4 \text{ kNm}^2$$

Verilen sistemin yedeği-  
melerini ve kesit tesirlerini  
hesaplayalım.



| D.N | X(m) | Y(m) |
|-----|------|------|
| 1   | 0    | 4    |
| 2   | 4    | 4    |
| 3   | 10   | 4    |
| 4   | 10   | 0    |

| E.N | l | r | l(m) | c | s  |
|-----|---|---|------|---|----|
| 1   | 1 | 2 | 4    | 1 | 0  |
| 2   | 2 | 3 | 6    | 1 | 0  |
| 3   | 3 | 4 | 4    | 0 | -1 |

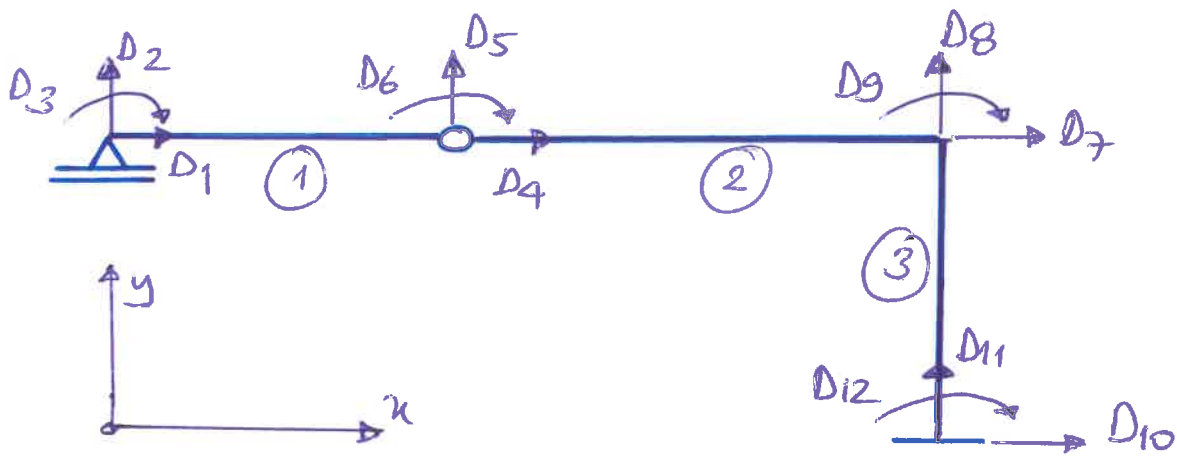
$$C = \frac{X_r - X_l}{L}$$

$$S = \frac{Y_r - Y_l}{L}$$

$$C_1 = \frac{X_2 - X_1}{L_1} = \frac{4 - 0}{4} = 1 \quad C_2 = \frac{X_3 - X_2}{L_2} = \frac{10 - 4}{6} = 1 \quad C_3 = \frac{X_4 - X_3}{L_3} = \frac{10 - 10}{4} = 0$$

$$S_1 = \frac{Y_2 - Y_1}{L_1} = \frac{4 - 4}{4} = 0 \quad S_2 = \frac{Y_3 - Y_2}{L_2} = \frac{4 - 4}{6} = 0 \quad S_3 = \frac{Y_4 - Y_3}{L_3} = \frac{0 - 4}{4} = -1$$

(12)



masal 1 nolu elemanlar dır.

$$[k_1] = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{3EI}{L^3} & \frac{3EI}{L^2} & 0 & -\frac{3EI}{L^3} & 0 \\ 0 & \frac{3EI}{L^2} & \frac{3EI}{L} & 0 & -\frac{3EI}{L^2} & 0 \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{3EI}{L^3} & -\frac{3EI}{L^2} & 0 & \frac{3EI}{L^3} & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{matrix} u_1 \\ \theta_1 \\ \varphi_1 \\ u_2 \\ \theta_2 \\ \varphi_2 \end{matrix}$$

$$[k_1] = 10^4 \begin{bmatrix} 81 & 0 & 0 & -81 & 0 & 0 \\ 0 & 0,2025 & 0,81 & 0 & -0,2025 & 0 \\ 0 & 0,81 & 3,24 & 0 & -0,81 & 0 \\ -81 & 0 & 0 & 81 & 0 & 0 \\ 0 & -0,2025 & -0,81 & 0 & 0,2025 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$



$$[k_2] = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{12EI}{L^3} & \frac{6EI}{L^2} & 0 & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{4EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{12EI}{L^3} & -\frac{6EI}{L^2} & 0 & \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{2EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} \begin{matrix} u_1 \\ \theta_1 \\ \varphi_1 \\ u_2 \\ \theta_2 \\ \varphi_2 \end{matrix}$$

$$[k_2] = 10^4 \begin{bmatrix} 54 & 0 & 0 & -54 & 0 & 0 \\ 0 & 0,24 & 0,72 & 0 & -0,24 & 0,72 \\ 0 & 0,72 & 2,88 & 0 & -0,72 & 1,44 \\ -54 & 0 & 0 & 54 & 0 & 0 \\ 0 & -0,24 & -0,72 & 0 & 0,24 & -0,72 \\ 0 & 0,72 & 1,44 & 0 & -0,72 & 2,88 \end{bmatrix}$$

$$[k_3] = 10^4 \begin{bmatrix} 81 & 0 & 0 & -81 & 0 & 0 \\ 0 & 0,81 & 1,62 & 0 & -0,81 & 1,62 \\ 0 & 1,62 & 4,32 & 0 & -1,62 & 2,16 \\ -81 & 0 & 0 & 81 & 0 & 0 \\ 0 & -0,81 & -1,62 & 0 & 0,81 & -1,62 \\ 0 & 1,62 & 2,16 & 0 & -1,62 & 4,32 \end{bmatrix}$$

$$[\tau_1] = [\tau_2] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[\tau_3] = \begin{bmatrix} 0 & -1 & 0 & 0 & 0 & 0 \\ -1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[c_1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[c_2] = \begin{bmatrix} 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \end{bmatrix}$$

$$[C_3] = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$\{Q\}_s \left\{ \begin{array}{l} 0 \\ -\frac{40 \times 4}{2} + R_{AV} \\ 0 \\ 0 \\ -\frac{40 \times 4}{2} - 60 - \frac{40 \times 6}{2} \\ \frac{40 \times 6^2}{12} \\ -\frac{30 \times 4}{2} \\ -\frac{40 \times 6}{2} \\ -\frac{40 \times 6^2}{12} + \frac{30 \times 4^2}{12} \\ -\frac{30 \times 4}{2} + R_{BH} \\ 0 + R_{BV} \\ -\frac{30 \times 4^2}{12} + M_B \end{array} \right\} \begin{array}{l} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \\ D_8 \\ D_9 \\ D_{10} \\ D_{11} \\ D_{12} \end{array} = \left\{ \begin{array}{l} 0 \\ -80 + R_{AV} \\ 0 \\ 0 \\ -260 \\ 120 \\ -60 \\ -120 \\ -80 \\ -60 + R_{BH} \\ R_{BV} \\ -40 + M_B \end{array} \right\}$$

$$[K_{e1}] = [T_1]^T [k_1] [T_1]$$

$$[K_{e2}] = [T_2]^T [k_2] [T_2]$$

$$[K_{e3}] = [T_3]^T [k_3] [T_3]$$

$$[K_e]^1 = [C_1]^T [K_{e1}] [C_1]$$

$$[K_e]^2 = [C_2]^T [K_{e2}] [C_2]$$

$$[K_e]^3 = [C_3]^T [K_{e3}] [C_3]$$

$$[K] = [K_e]^1 + [K_e]^2 + [K_e]^3$$

$$\begin{bmatrix}
 81 & 0 & 0 & -81 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0,2025 & -0,81 & 0 & 0,2025 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & -0,81 & 324 & 0 & 0,81 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 -81 & 0 & 0 & 135 & 0 & 0 & 54 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & -0,2025 & 0,81 & 0 & 0,425 & -0,72 & 0 & -0,24 & -0,72 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & -0,72 & 288 & 0 & 0,72 & 144 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & -54 & 0 & 0 & 54,81 & 0 & -162 & -0,81 & 0 & -162 & 0 \\
 0 & 0 & 0 & 0 & -0,24 & 0,72 & 0 & 81,24 & 0,72 & 0 & -81 & 0 & 0 \\
 0 & 0 & 0 & 0 & -0,72 & 144 & -162 & 0,72 & 720 & 162 & 0 & 216 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & -0,81 & 0 & 1,62 & 0,81 & 0 & 1,62 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & -81 & 0 & 0 & 81 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & -162 & 0 & 216 & 162 & 0 & 4,32 & 0
 \end{bmatrix}
 \begin{bmatrix}
 D_1 \\
 D_2 \\
 D_3 \\
 D_4 \\
 D_5 \\
 D_6 \\
 D_7 \\
 D_8 \\
 D_9 \\
 D_{10} \\
 D_{11} \\
 D_{12}
 \end{bmatrix}
 =
 \begin{bmatrix}
 0 \\
 -80 + R_{2V} \\
 0 \\
 0 \\
 -260 \\
 120 \\
 -60 \\
 -120 \\
 -80 \\
 -60 + R_{2H} \\
 R_{2V} \\
 -40 + M_3
 \end{bmatrix}$$

$$D_2 = D_{10} = D_{11} = D_{12} = 0$$

$$D_1 = -0,3111 \text{ m.} ; D_3 = 0,3237 \text{ rad.} ; D_4 = -0,3111 \text{ m.} ; D_5 = -1,2949 \text{ m.} ; D_6 = -0,2935 \text{ rad.}$$

$$D_7 = -0,3111 \text{ m.} ; D_8 = -0,000469 ; D_9 = -0,1519 \text{ rad.}$$

(17)



## Tepele Koordinatlar da Yerdeğiştirilmeler

$$\{u_1\}' = [T_1]\{u\} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} -0,3111 \\ 0 \\ 0,3237 \\ -0,3111 \\ -1,2949 \\ -0,2435 \end{Bmatrix} = \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{Bmatrix} = \begin{Bmatrix} -0,3111 \\ 0 \\ 0,3237 \\ -0,3111 \\ +1,2949 \\ -0,2435 \end{Bmatrix}$$

$$\{u_2\}' = [T_2]\{u\} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} -0,3111 \\ -1,2949 \\ -0,2435 \\ -0,3111 \\ -0,000469 \\ -0,1519 \end{Bmatrix} = \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{Bmatrix} = \begin{Bmatrix} -0,3111 \\ +1,2949 \\ -0,2435 \\ -0,3111 \\ +0,000469 \\ -0,1519 \end{Bmatrix}$$

$$\{u_3\}' = [T_3]\{u\} = \begin{bmatrix} 0 & -1 & 0 & 0 & 0 & 0 \\ -1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} -0,3111 \\ -0,000469 \\ -0,1519 \\ 0 \\ 0 \\ 0 \end{Bmatrix} = \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{Bmatrix} = \begin{Bmatrix} 0,000469 \\ 0,3111 \\ -0,1519 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

\*  $u = -0,3111$  m. yerdeğiştirmeye genel sistende  $D_7$ 'ye denk düşmekte ve genel sistende yatay yerdeğiştirmedir. (3) nokta yukarı doğru hareket etmektedir.

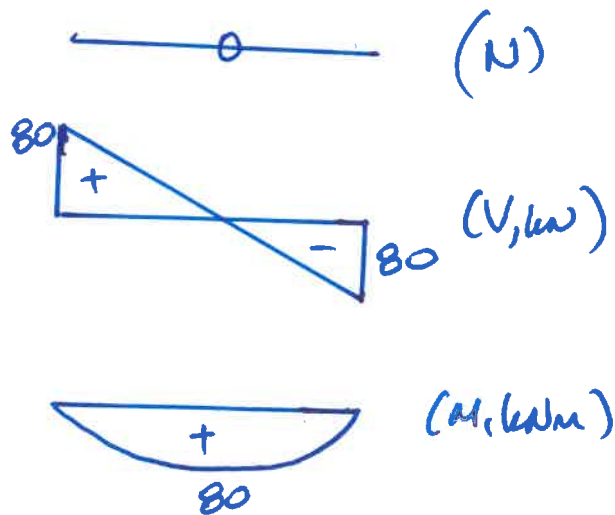
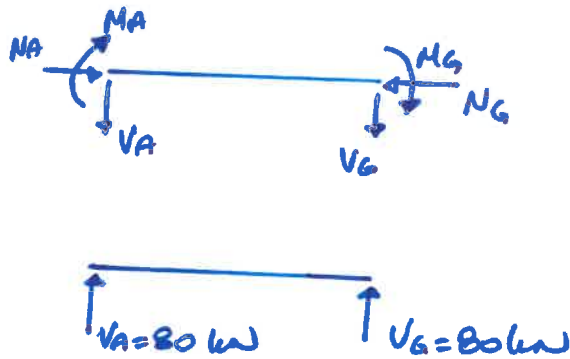
# Güçlük kuvvetlerinin hesabı

## 1 nolu gübük

$$\{F\}^e = [K]^e \cdot \{D\}^e - \{Q\}^e$$

$$\{F\} = \begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = 10^4 \cdot \begin{bmatrix} 81 & 0 & 0 & -81 & 0 & 0 \\ 0 & 0,2025 & 0,81 & 0 & -0,2025 & 0 \\ 0 & 0,81 & 324 & 0 & -0,81 & 0 \\ -81 & 0 & 0 & 81 & 0 & 0 \\ 0 & -0,2025 & -0,81 & 0 & 0,2025 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} -0,3111 \\ 0 \\ 0,3237 \\ -0,3111 \\ +1,2949 \\ -0,2435 \end{Bmatrix} - \begin{Bmatrix} 0 \\ +80 \\ 0 \\ 0 \\ +80 \\ 0 \end{Bmatrix}$$

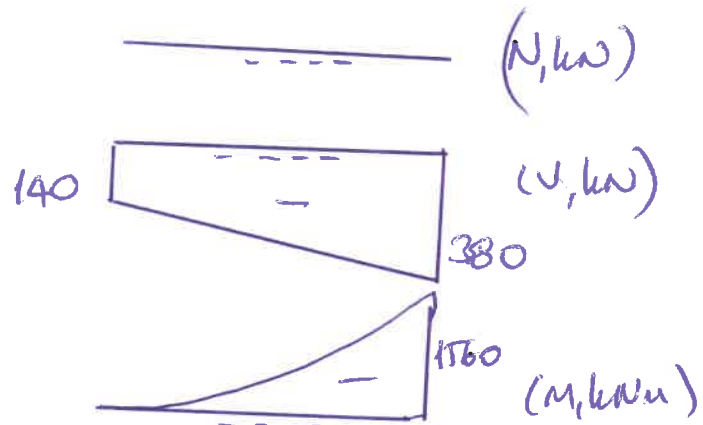
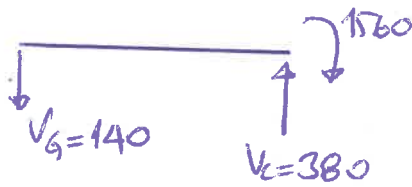
$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = \begin{Bmatrix} N_A \\ V_A \\ M_A \\ N_G \\ V_G \\ M_G \end{Bmatrix} \approx \begin{Bmatrix} 0 \\ -80 \\ 0 \\ 0 \\ -80 \\ 0 \end{Bmatrix}$$





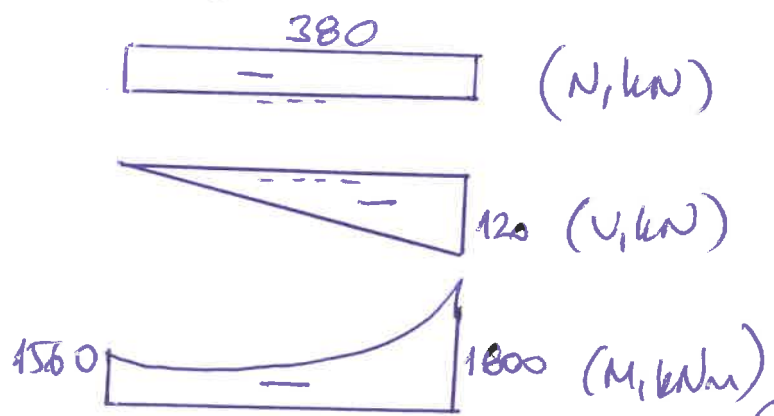
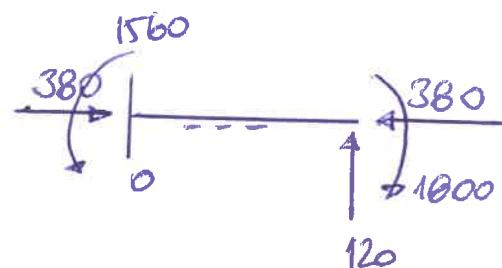
## 2 node gubuk

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = 10^4 \times \begin{bmatrix} 54 & 0 & 0 & -54 & 0 & 0 \\ 0 & 0,24 & 0,72 & 0 & -0,24 & 0,72 \\ 0 & 0,72 & 2,88 & 0 & -0,72 & 1,44 \\ -54 & 0 & 0 & 54 & 0 & 0 \\ 0 & -0,24 & -0,72 & 0 & 0,24 & -0,72 \\ 0 & 0,72 & 1,44 & 0 & -0,72 & 2,88 \end{bmatrix} \begin{Bmatrix} -0,3111 \\ +1,2949 \\ -0,2435 \\ -0,3111 \\ 0,000469 \\ -0,1519 \end{Bmatrix} - \begin{Bmatrix} 0 \\ 120 \\ 120 \\ 0 \\ 120 \\ -120 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 140 \\ 0 \\ 0 \\ -380 \\ 1560 \end{Bmatrix}$$



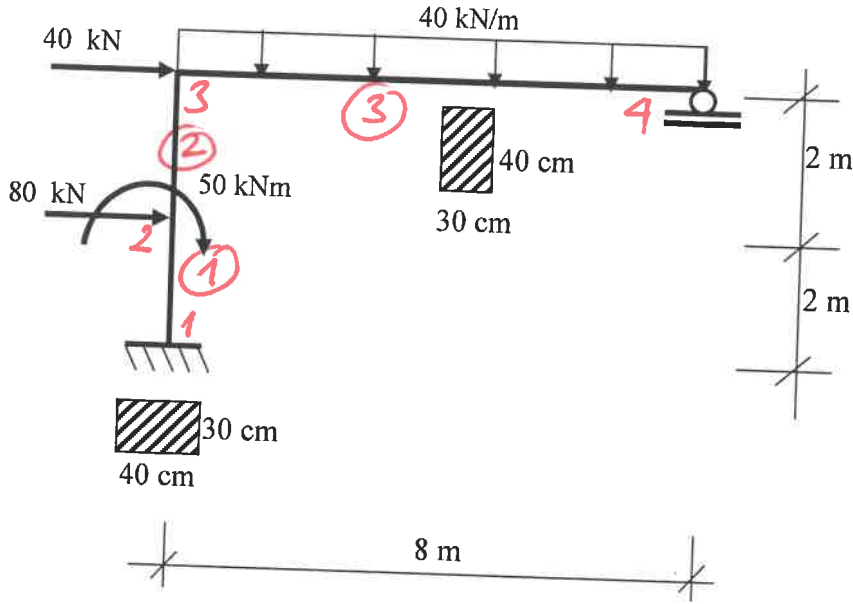
## 3 node gubuk

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = 10^4 \times \begin{bmatrix} 81 & 0 & 0 & -81 & 0 & 0 \\ 0 & 0,81 & 1,62 & 0 & -0,81 & 1,62 \\ 0 & 1,62 & 4,32 & 0 & -1,62 & 2,16 \\ -81 & 0 & 0 & 81 & 0 & 0 \\ 0 & -0,81 & -1,62 & 0 & 0,81 & -1,62 \\ 0 & 1,62 & 2,16 & 0 & -1,62 & 4,32 \end{bmatrix} \begin{Bmatrix} 0,000469 \\ 0,3111 \\ -0,1519 \\ 0 \\ 0 \\ 0 \end{Bmatrix} - \begin{Bmatrix} 0 \\ 60 \\ 40 \\ 0 \\ 60 \\ -40 \end{Bmatrix} = \begin{Bmatrix} 380 \\ 0 \\ -1560 \\ -380 \\ -120 \\ 1800 \end{Bmatrix}$$

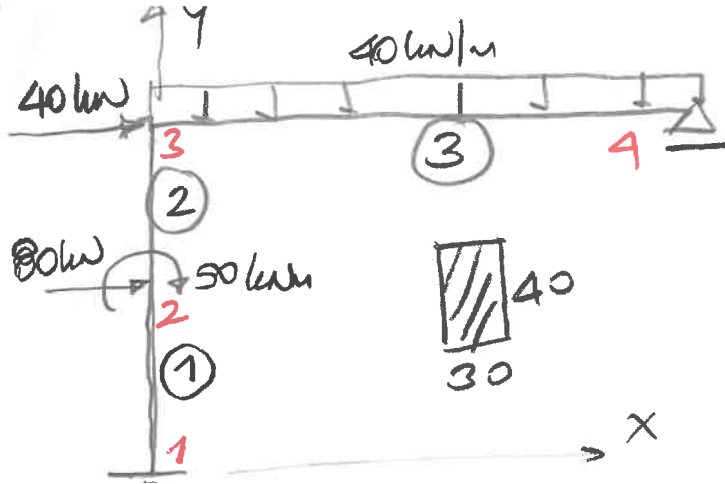


ÖRNEK: Şekilde verilen sistemin düğüm noktalarının yerdeğiştirmelerini ve çubuk kuvvetlerini sonlu elemanlar yöntemi ile bulunuz.

( $E: 21 \times 10^6 \text{ kN/m}^2$ )



Not: Tekil yük bulunan çubuklarda,  
tekil yükün etki ettiği' noktaları düğüm  
noktası olarak ele almak işlemi  
kolaylığı sağlayacaktır!



| DN | X | Y |
|----|---|---|
| 1  | 0 | 0 |
| 2  | 0 | 2 |
| 3  | 0 | 4 |
| 4  | 8 | 4 |



$$E = 21 \times 10^6 \text{ kN/m}^2$$

$$I = \frac{0,3 \times 0,4^3}{12} = 1,6 \times 10^{-3} \text{ m}^4$$

$$EA = 252 \times 10^4 \text{ kN}$$

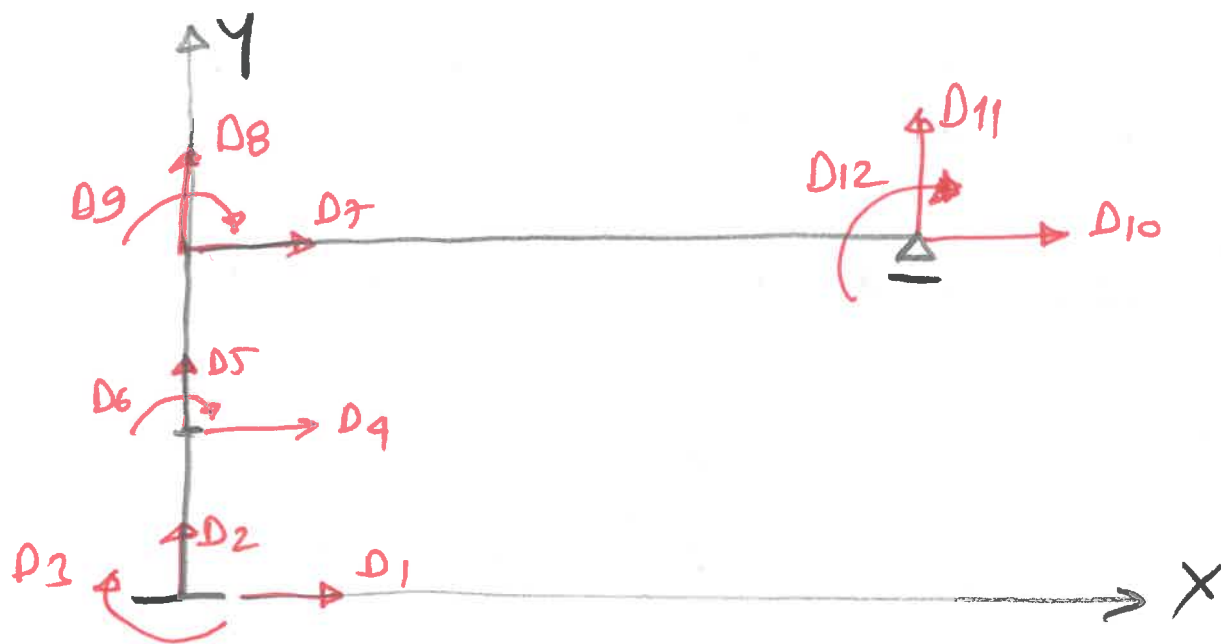
$$EI = 3,36 \times 10^4 \text{ kNm}^2$$

| E.N | $\bar{i}$ | $\bar{j}$ | L | C | S |
|-----|-----------|-----------|---|---|---|
| 1   | 1         | 2         | 2 | 0 | 1 |
| 2   | 2         | 3         | 2 | 0 | 1 |
| 3   | 3         | 4         | 8 | 1 | 0 |



Eleman yedegötkme yönleri

$$[k_e] = \begin{bmatrix} EA/L & 0 & 0 & -EA/L & 0 & 0 \\ 0 & 12EI/L^3 & 6EI/L^2 & 0 & -12EI/L^3 & 6EI/L^2 \\ 0 & 6EI/L^2 & 4EI/L & 0 & -6EI/L^2 & 2EI/L \\ -EA/L & 0 & 0 & EA/L & 0 & 0 \\ 0 & -12EI/L^3 & -6EI/L^2 & 0 & 12EI/L^3 & -6EI/L^2 \\ 0 & 6EI/L^2 & 2EI/L & 0 & -6EI/L^2 & 4EI/L \end{bmatrix}$$



$$[k_1][k_2] = 10^4 \begin{bmatrix} 126 & 0 & 0 & -126 & 0 & 0 \\ 0 & 5,04 & 5,04 & 0 & -5,04 & 5,04 \\ 0 & 5,04 & 6,72 & 0 & -5,04 & 3,36 \\ -126 & 0 & 0 & 126 & 0 & 0 \\ 0 & -5,04 & -5,04 & 0 & 5,04 & -5,04 \\ 0 & 5,04 & 3,36 & 0 & -5,04 & 6,72 \end{bmatrix}$$

$$[k_3] = 10^4 \begin{bmatrix} 31,5 & 0 & 0 & -31,5 & 0 & 0 \\ 0 & 0,07875 & 0,315 & 0 & -0,07875 & 0,315 \\ 0 & 0,315 & 1,68 & 0 & -0,315 & 0,84 \\ -31,5 & 0 & 0 & 31,5 & 0 & 0 \\ 0 & -0,07875 & -0,315 & 0 & 0,07875 & -0,315 \\ 0 & 0,315 & 0,84 & 0 & -0,315 & 1,68 \end{bmatrix}$$

$$\begin{aligned} [T_1] &= \\ [T_2] &= \end{aligned}$$

$$[\tau_3] =$$

$$[c_1] =$$

$$[C_2] =$$

$$[C_3] =$$

$$[K_{e1}] = [T_1]^T [K_m] [T_1]$$

$$[K_1] = [C_1]^T [K_e] [C_1]$$

$$[k_2] = [T_2]^T [k_1] [T_2]$$

$$[K]_2 = [C]_2^T [k]_2 [C]_2$$

$$[u_3] = [C_3]^T [u_2] [C_3]$$

1.2. 1.2.2.

$$[K] = [K]_1 + [K]_2 + [K]_3$$

0.3/9









# Yerel Koordinatlarda Yerdeğiştirme

$$\{u_1\}' = [T_1]\{u\} = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0,0197 \\ -0,000131 \\ 0,0173 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -0,000131 \\ 0,0197 \\ 0,0173 \end{Bmatrix}^{1e}$$

$$\{u_2\}^2 = [T_2]\{u\} = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0,0197 \\ -0,000131 \\ 0,0173 \\ 0,0599 \\ -0,000262 \\ 0,0221 \end{Bmatrix} = \begin{Bmatrix} -0,000131 \\ 0,0197 \\ 0,0173 \\ -0,000262 \\ 0,0599 \\ 0,0221 \end{Bmatrix}^{2e}$$

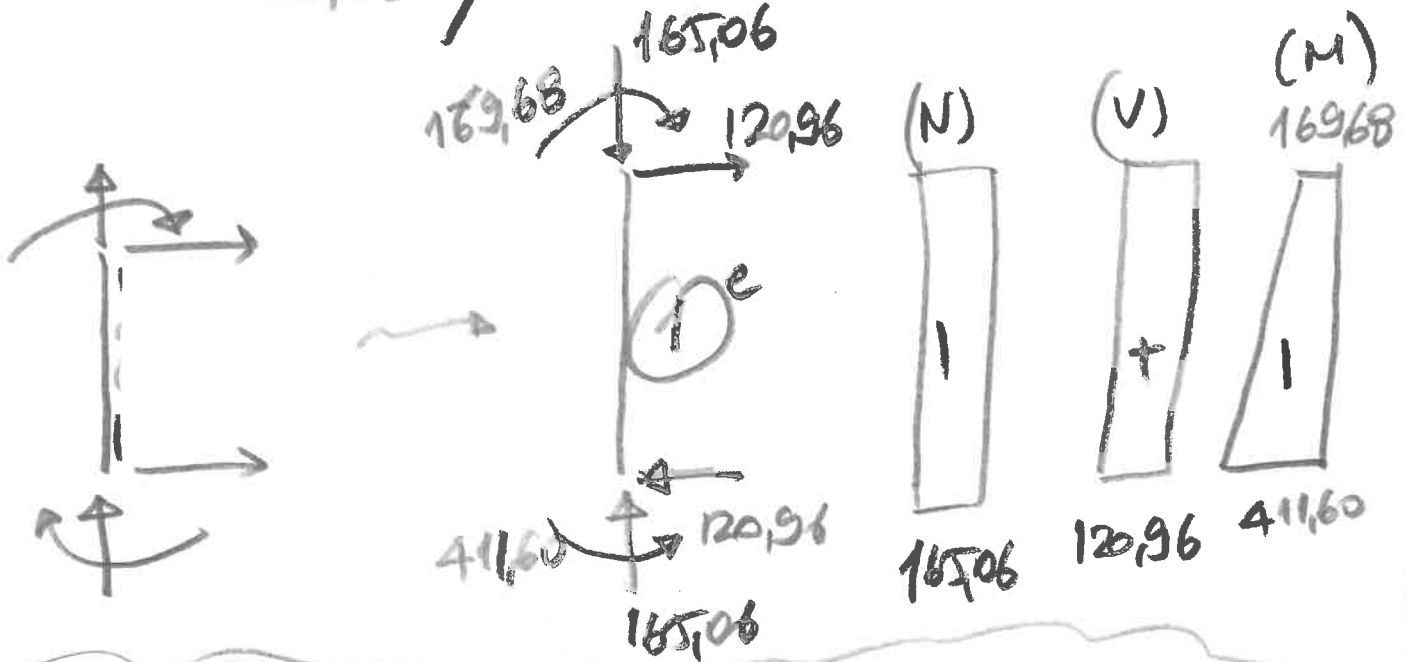
$$\{u_3\}^3 = [T_3]\{u\} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 0,0599 \\ -0,000262 \\ 0,0221 \\ 0,0599 \\ 0 \\ -0,0238 \end{Bmatrix} = \begin{Bmatrix} 0,0599 \\ 0,000262 \\ 0,0221 \\ 0,0599 \\ 0 \\ -0,0238 \end{Bmatrix}$$

# Çubuk Kuvvetlerinin Hesabı

1. eleman

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = 10^4 \begin{bmatrix} 126 & 0 & 0 & -126 & 0 & 0 \\ 0 & 5,04 & 5,04 & 0 & -5,04 & 5,04 \\ 0 & 5,04 & 6,72 & 0 & -5,04 & 3,36 \\ -126 & 0 & 0 & 126 & 0 & 0 \\ 0 & -5,04 & -5,04 & 0 & 5,04 & -5,04 \\ 0 & 5,04 & 3,36 & 0 & -5,04 & 6,72 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -0,000131 \\ 0,0197 \\ 0,0133 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ - \\ * \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = \begin{Bmatrix} 165,06 \\ -120,96 \\ -411,60 \\ -165,06 \\ 120,96 \\ 169,68 \end{Bmatrix}$$

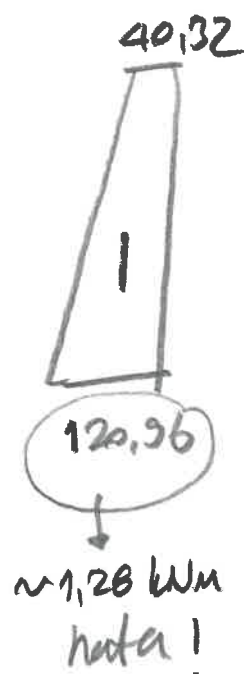
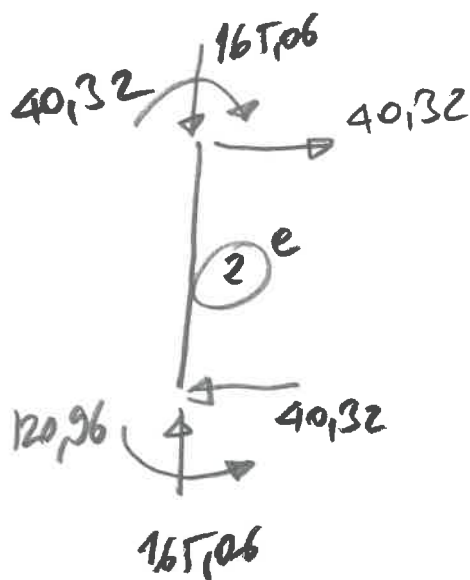


\* Not:  $D_5 = -0,000131$  değeri hesaplarda  $-0,0001$  olarak görülebilmek. Bu durumda  $N$  değeri  $126 \text{ kN}$  gibi ( $N_{gerçek} = 165 \text{ kN}$ ) farklı bir değer hesaplanabilmektedir.

2. element

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix}^{2e} = 10^4 \begin{bmatrix} 126 & 0 & 0 & -126 & 0 & 0 \\ 0 & 5,04 & 5,04 & 0 & -5,04 & 5,04 \\ 0 & 5,04 & 6,72 & 0 & -5,04 & 3,36 \\ -126 & 0 & 0 & 126 & 0 & 0 \\ 0 & -5,04 & -5,04 & 0 & 5,04 & -5,04 \\ 0 & 5,04 & 3,36 & 0 & -5,04 & 6,72 \end{bmatrix} \begin{Bmatrix} -0,000131 \\ 0,0197 \\ 0,0173 \\ -0,000262 \\ 0,0599 \\ 0,0221 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix}^{2e} = \begin{Bmatrix} 165,06 \\ -40,32 \\ -120,96 \\ -165,06 \\ 40,32 \\ 40,32 \end{Bmatrix}$$

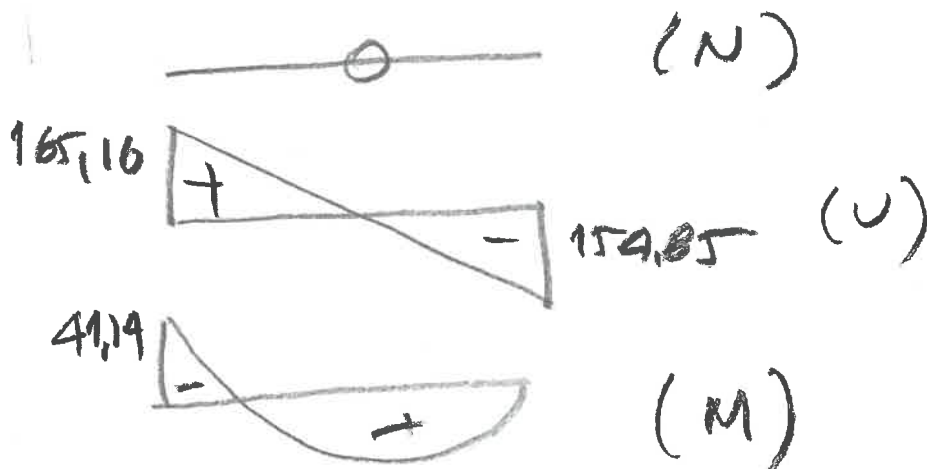
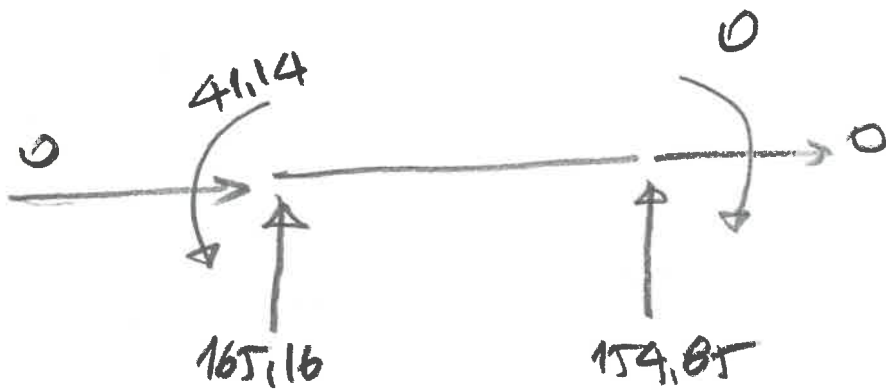


5. eleman)

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = 10^4 \begin{bmatrix} 31,5 & 0 & 0 & -31,5 & 0 & 0 \\ 0 & 0,07875 & 0,315 & 0 & -0,07875 & 0,315 \\ 0 & 0,315 & 1,68 & 0 & -0,315 & 0,84 \\ -31,5 & 0 & 0 & 31,5 & 0 & 0 \\ 0 & -0,07875 & -0,315 & 0 & 0,07875 & -0,315 \\ 0 & 0,315 & 0,84 & 0 & -0,315 & 1,68 \end{bmatrix} \begin{Bmatrix} 0,0593 \\ 0,000262 \\ 0,0221 \\ 0,0593 \\ 0 \\ -0,0238 \end{Bmatrix} - \begin{Bmatrix} 0 \\ +160 \\ 213,333 \\ 0 \\ +160 \\ -213,333 \end{Bmatrix}$$

$$\begin{Bmatrix} N_1 \\ V_1 \\ M_1 \\ N_2 \\ V_2 \\ M_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ -165,15 \\ -41,14 \\ 0 \\ -154,85 \\ 0 \end{Bmatrix} \rightarrow 0,826 \text{ Nm hata!}$$

elemanlar yonler  
azagıya dogru  
pozitif alınmıstır.

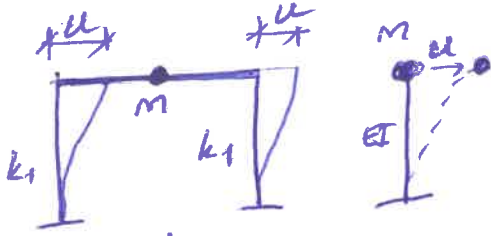


0.9/9

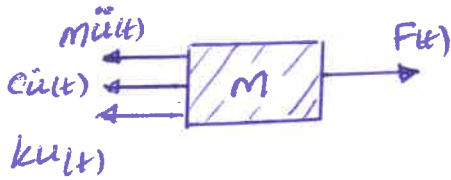
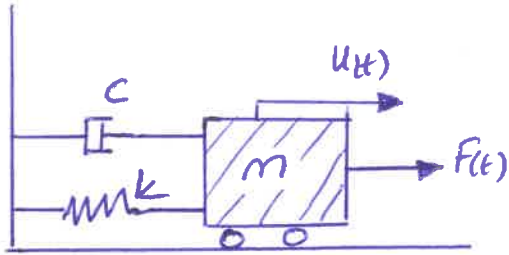
EK:2

## ÇERÇEVELERİN SONLU ELEMANLAR YÖNTEMİ İLE SERBEST TİTREŞİMİ

# SERBEST TITREYİM



Şekil 1



$$m\ddot{u}(t) + c\dot{u}(t) + ku(t) = F(t)$$

(Hareket Denklemi)

$m$  = kütle matrisi

$\ddot{u}(t)$  = ivme vektörü

$c$  = sönüm matrisi

$\dot{u}(t)$  = hız vektörü

$k$  = rijitlik matrisi

$u$  = yerdeğiştirme vektörü

$F(t)$  = zamana bağlı dış kuvvet

$$[M]\{\ddot{u}\} + [C]\{\dot{u}\} + [K]\{u\} = \{F\}$$

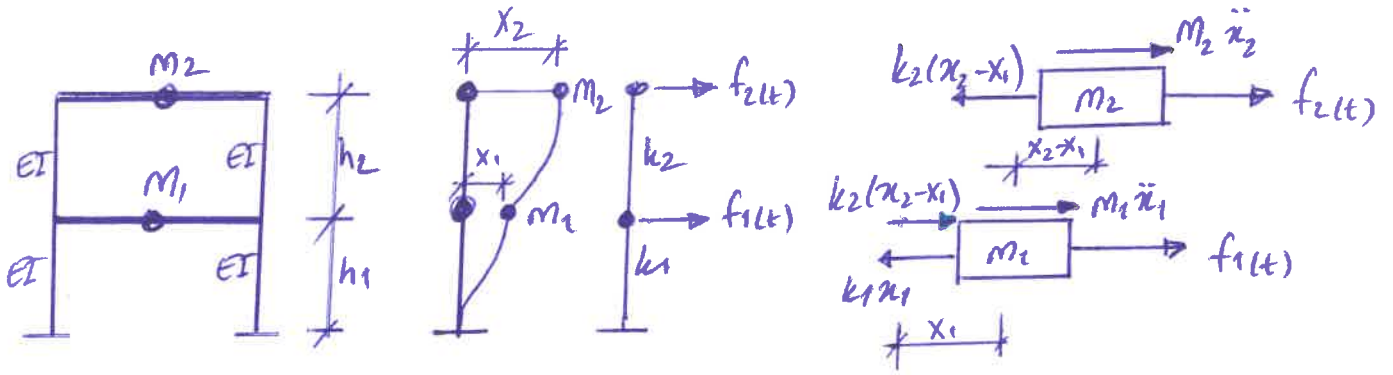
$$([K] - \omega_i^2 [M])\{u_i\} = 0 \quad \text{özdeğer problemi}$$

$\omega_i$  = sistemin doğal açısal frekansı

$$T = \frac{2\pi}{\omega_i} \quad \text{sistemin doğal titreşim periyodu}$$

Sayet, Şekil 1'de verilen serbest titreşim kaynağı serbest olarak dikkate alınıyorsa kolon rijitliği  $k_1$ ,

$$k_1 = \frac{12EI}{h^3} \quad \text{olarak hesaplanacaktır.}$$



$$\sum F = m \cdot a$$

$$f_1(t) - k_1 x_1 + k_2 (x_2 - x_1) = m_1 \ddot{x}_1 \leadsto -m_1 \ddot{x}_1 - k_1 x_1 - k_2 x_1 + k_2 x_2 = -f_1(t)$$

$$f_2(t) - k_2 (x_2 - x_1) = m_2 \ddot{x}_2 \leadsto -m_2 \ddot{x}_2 - k_2 x_2 + k_2 x_1 = -f_2(t)$$

$$m_1 \ddot{x}_1 + (k_1 + k_2) x_1 - k_2 x_2 = f_1(t) \quad \text{--- (1)}$$

$$m_2 \ddot{x}_2 - k_2 x_1 + k_2 x_2 = f_2(t) \quad \text{--- (2)}$$

(1) ve (2) bağıntıları matris formunda yazılabilir;

$$\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{Bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{Bmatrix} + \begin{bmatrix} (k_1 + k_2) & -k_2 \\ -k_2 & +k_2 \end{bmatrix} \begin{Bmatrix} x_1 \\ x_2 \end{Bmatrix} = \begin{Bmatrix} f_1(t) \\ f_2(t) \end{Bmatrix}$$

Bu serbest titreşim durumundadır;

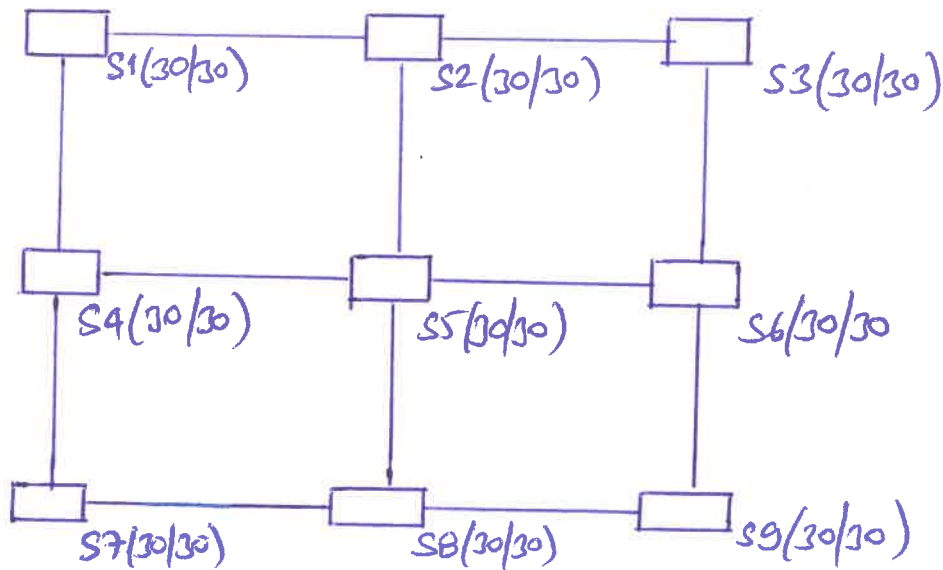
$$\left| \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & +k_2 \end{bmatrix} - \omega^2 \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \right| = 0$$

olarak ele alınacaktır. Buradan detaylı olarak kökleri alın  $\omega_i^2$  sistemin doğal doğal frekansları hesaplanacaktır. Hesaplanan  $\omega_i$ 'ler her bir periyoda kaç tane geçeceğinden

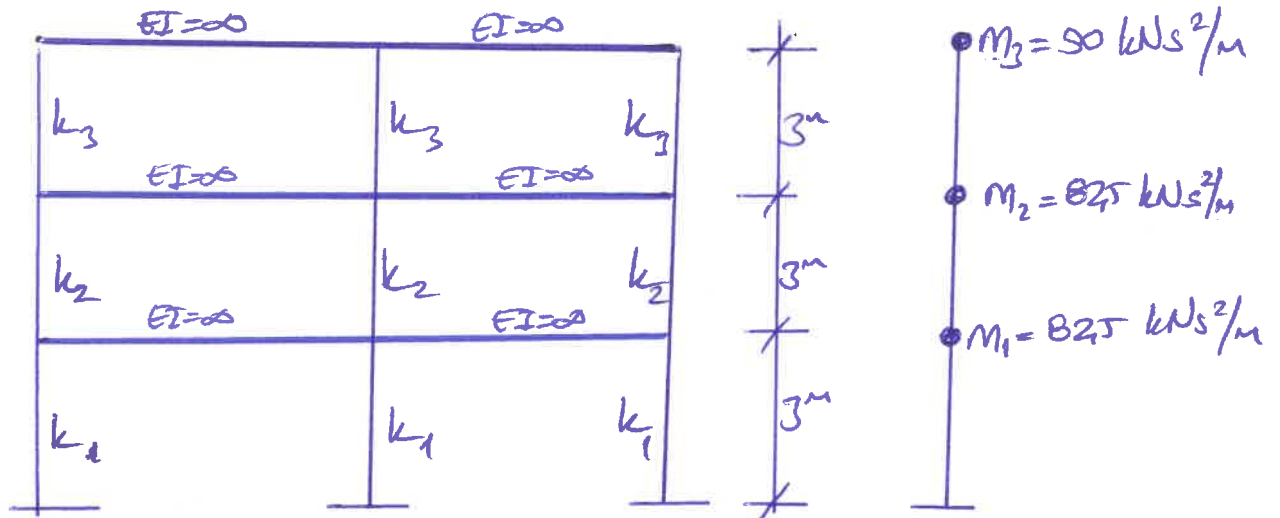
$T_i = \frac{2\pi}{\omega_i}$  ile serbestlik derecesi kadar periyot bulunabilecektir.



# ÖRNEK



$$E_c = 2,8 \cdot 10^7 \text{ kN/m}^2$$



$$k = \frac{12EI}{h^3} = 12 \cdot 2,8 \cdot 10^7 \cdot \frac{0,30 \cdot 0,3^3}{12} \cdot \frac{1}{3^3} = 8400 \text{ kN/m}$$

Kat. Rijitlik  $\Sigma k_1 = \Sigma k_2 = \Sigma k_3 = 9 \cdot k = 75600 \text{ kN/m}$

$$[K] = \begin{bmatrix} k_1 + k_2 & -k_2 & 0 \\ -k_2 & k_2 + k_3 & -k_3 \\ 0 & -k_3 & k_3 \end{bmatrix} = \begin{bmatrix} 151200 & -75600 & 0 \\ -75600 & 151200 & -75600 \\ 0 & -75600 & 75600 \end{bmatrix}$$

$$[M] = \begin{bmatrix} m_1 & 0 & 0 \\ 0 & m_2 & 0 \\ 0 & 0 & m_3 \end{bmatrix} = \begin{bmatrix} 825 & 0 & 0 \\ 0 & 825 & 0 \\ 0 & 0 & 50 \end{bmatrix}$$

$$|[K] - \omega^2 [M]| = 0 \quad \text{serbest titreşim}$$

$$\left| \begin{bmatrix} 15120 & -75600 & 0 \\ -75600 & 15120 & -75600 \\ 0 & -75600 & 75600 \end{bmatrix} - \omega^2 \begin{bmatrix} 825 & 0 & 0 \\ 0 & 825 & 0 \\ 0 & 0 & 50 \end{bmatrix} \right| = 0$$

$$\left| \begin{array}{ccc} 15120 - 825\omega^2 & -75600 & 0 \\ -75600 & 15120 - 825\omega^2 & -75600 \\ 0 & -75600 & 75600 - 50\omega^2 \end{array} \right| = 0$$

$\omega^2 = \lambda$  ile ve determinantı  
açarak yapılır;

$$(15120 - 825\omega^2) \left[ (15120 - 825\omega^2)(75600 - 50\omega^2) - (-75600)(-75600) \right] - (-75600) \left[ (-75600)(75600 - 50\omega^2) - (0)(-75600) \right] = 0$$

elde edilmiş. Birinci terimden dışarı çıkarsa;

$$3403125 \lambda^3 - 1761952500 \lambda^2 + 22718556 \cdot 10^5 \lambda - 432081216 \cdot 10^6 = 0$$

Deriveleri gözden geçir;

$$\lambda_1 = \omega_1^2 = 0,2291 \cdot 10^3 \text{ (rad/sec)}^2 \rightarrow \omega_1 = 15,1357 \text{ rad/s}$$

$$\lambda_2 = \omega_2^2 = 1,7130 \cdot 10^3 \text{ (rad/sec)}^2 \rightarrow \omega_2 = 41,3883 \text{ rad/s}$$

$$\lambda_3 = \omega_3^2 = 3,2354 \cdot 10^3 \text{ (rad/sec)}^2 \rightarrow \omega_3 = 56,8803 \text{ rad/s}$$

değerleri ekle edilmiş Buradan;

$$T_1 = \frac{2\pi}{\omega_1} = 0,42 \text{ s.}$$

$$T_2 = \frac{2\pi}{\omega_2} = 0,15 \text{ s.}$$

$$T_3 = \frac{2\pi}{\omega_3} = 0,11 \text{ s.}$$

olarak periyotlar belirlenir.

Yukarıda verilen denkleme yaygın Wile olarak ele alınan Wile matrisi' yaygın olarak dikilde alınacak  
 De:

$$[M^e] = \int_V \rho \phi^T \phi dV = \int_0^1 \bar{\rho}^T \phi (L d\xi) \int_A \rho dA$$

bağıntıyı kullanılabılır. Bağlıdır;

$$\phi^T = \begin{Bmatrix} \phi_1^T \\ \phi_2^T \\ \phi_3^T \\ \phi_4^T \end{Bmatrix} = \begin{Bmatrix} 1 - 3\xi^2 + 2\xi^3 \\ L\xi - 2L\xi^2 + L\xi^3 \\ 3\xi^2 - 2\xi^3 \\ -L\xi^2 + L\xi^3 \end{Bmatrix}$$

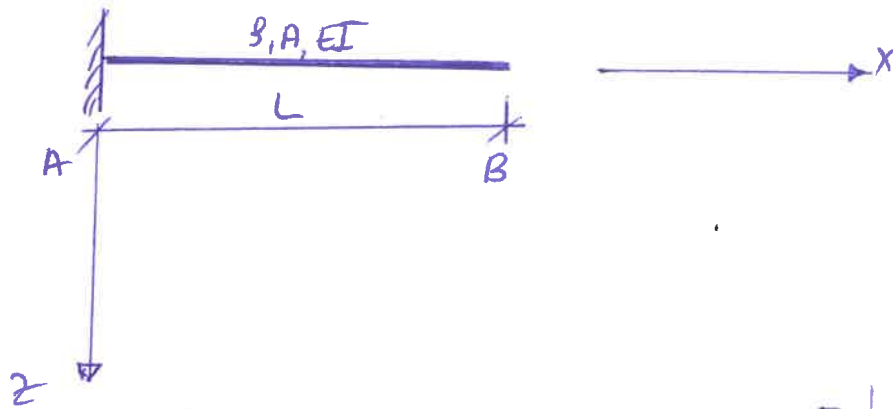
$\rho$  = birim hacim Wile'dir.

Bu durumda Wile matrisi;

$$[M^e] = L \int_0^1 \begin{bmatrix} 1 - 3\xi^2 + 2\xi^3 \\ L(\xi - 2\xi^2 + \xi^3) \\ 3\xi^2 - 2\xi^3 \\ L(-\xi^2 + \xi^3) \end{bmatrix} \begin{bmatrix} 1 - 3\xi^2 + 2\xi^3 & L(\xi - 2\xi^2 + \xi^3) & 3\xi^2 - 2\xi^3 & L(-\xi^2 + \xi^3) \end{bmatrix} d\xi \int_A \rho dA$$

$$= \frac{\rho A L}{420} \begin{bmatrix} 156 & 22L & 54 & -13L \\ 22L & 4L^2 & 13L & -3L^2 \\ 54 & 13L & 156 & -22L \\ -13L & -3L^2 & -22L & 4L^2 \end{bmatrix}$$

OR



$$\left( \frac{EI}{L^2} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} - \omega^2 \frac{\rho A L}{420} \begin{bmatrix} 156 & 22L & 54 & -13L \\ 22L & 4L^2 & 13L & -3L^2 \\ 54 & 13L & 156 & -22L \\ -13L & -3L^2 & -22L & 4L^2 \end{bmatrix} \right) \begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{Bmatrix} = 0$$

$$\left( \frac{EI}{L^3} \begin{bmatrix} 12 & -6L \\ -6L & 4L^2 \end{bmatrix} - \omega^2 \frac{\rho A L}{420} \begin{bmatrix} 156 & -22L \\ -22L & 4L^2 \end{bmatrix} \right) \begin{Bmatrix} D_1 \\ D_4 \end{Bmatrix} = 0$$

$$\lambda = \omega^2 \text{ i.e.,}$$

$$\left| \begin{bmatrix} \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} - \begin{bmatrix} \frac{156\rho A L}{420} \lambda & -\frac{22}{420} \rho A L^2 \lambda \\ -\frac{22}{420} \rho A L^2 \lambda & +\frac{4}{420} \rho A L^3 \lambda \end{bmatrix} \right| = 0$$

$$\left| \begin{array}{cc} \frac{12EI}{L^3} - \frac{156\rho A L}{420} \lambda & -\frac{6EI}{L^2} + \frac{22}{420} \rho A L^2 \lambda \\ -\frac{6EI}{L^2} + \frac{22}{420} \rho A L^2 \lambda & \frac{4EI}{L} - \frac{4}{420} \rho A L^3 \lambda \end{array} \right| = 0$$

$$\frac{48(EI)^2}{L^4} - \frac{48}{420} \rho \frac{AL^3}{L^3} EI \omega^2 - \frac{624}{420} EI \rho A \omega^2 + \frac{624}{420^2} \rho^2 A^2 L^4 \omega^2$$

$$- \left[ \frac{36(EI)^2}{L^4} - \frac{132}{420} \frac{EI}{L^2} \rho A L^2 \omega^2 - \frac{132}{420} \frac{EI}{L^2} \rho A L^2 \omega^2 + \frac{484}{420^2} \rho^2 A^2 L^4 \omega^2 \right] = 0$$

$$\alpha = \omega^2 \frac{\rho A L^4}{EI} \text{ re;}$$

$$\frac{12(EI)^2}{L^4} - \alpha \left( \frac{48}{420} \frac{(EI)^2}{L^4} \right) - \alpha \left( \frac{624}{420} \frac{(EI)^2}{L^4} \right) + \alpha^2 \left( \frac{624}{420^2} \frac{(EI)^2}{L^4} \right)$$

$$+ \alpha \left( \frac{132}{420} \frac{(EI)^2}{L^4} \right) + \alpha \left( \frac{132}{420} \frac{(EI)^2}{L^4} \right) - \alpha^2 \left( \frac{484}{420^2} \frac{(EI)^2}{L^4} \right) = 0$$

$$\frac{12(EI)^2}{L^4} - \frac{408}{420} \frac{(EI)^2}{L^4} \alpha + \frac{1}{1260} \frac{(EI)^2}{L^4} \alpha^2 = 0$$

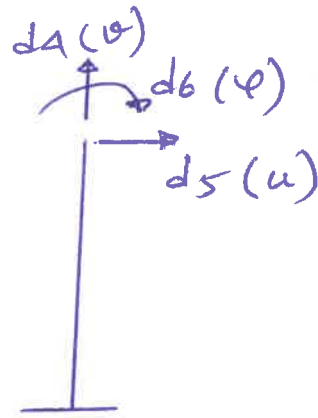
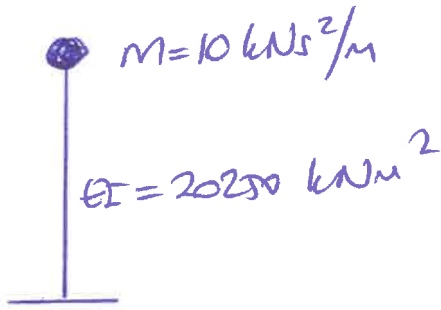
$$12 - 0,971 \alpha + \frac{1}{1260} \alpha^2 = 0$$

$$\alpha_1 = 12,48 \quad \alpha_2 = 1211,52$$

$$12,48 = \omega_1^2 \frac{\rho A L^4}{EI} \rightarrow \omega_1 = \sqrt{\frac{12,48 EI}{\rho A L^4}}$$

$$1211,52 = \omega_2^2 \frac{\rho A L^4}{EI} \rightarrow \omega_2 = \sqrt{\frac{1211,52 EI}{\rho A L^4}}$$

ÖRNEK



$[K] =$

| $D_1$          | $D_2$               | $D_3$              | $D_4$          | $D_5$               | $D_6$              |
|----------------|---------------------|--------------------|----------------|---------------------|--------------------|
| $\frac{EA}{L}$ | 0                   | 0                  | $\frac{EA}{L}$ | 0                   | 0                  |
| 0              | $\frac{12EI}{L^3}$  | $\frac{6EI}{L^2}$  | 0              | $-\frac{12EI}{L^3}$ | $\frac{6EI}{L^2}$  |
| 0              | $\frac{6EI}{L^2}$   | $\frac{4EI}{L}$    | 0              | $-\frac{6EI}{L^2}$  | $\frac{2EI}{L}$    |
| $\frac{EA}{L}$ | 0                   | 0                  | $\frac{EA}{L}$ | 0                   | 0                  |
| 0              | $-\frac{12EI}{L^3}$ | $-\frac{6EI}{L^2}$ | 0              | $\frac{12EI}{L^3}$  | $-\frac{6EI}{L^2}$ |
| 0              | $\frac{6EI}{L^2}$   | $\frac{2EI}{L}$    | 0              | $-\frac{6EI}{L^2}$  | $\frac{4EI}{L}$    |

Diagram showing degrees of freedom  $d_1, d_2, d_3, d_4, d_5, d_6$  at the top of the column.

Boyuna yerdeğiştirme olmuyacağından  $D_4 = 0$

$$| [K] - \omega^2 [M] | = 0$$

$$\left| \begin{bmatrix} \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} - \omega^2 \begin{bmatrix} 10 & 0 \\ 0 & 10 \end{bmatrix} \right| = 0$$



$$\left| \begin{bmatrix} 9000 & -13500 \\ -13500 & 27000 \end{bmatrix} - \omega^2 \begin{bmatrix} 10 & 0 \\ 0 & 10 \end{bmatrix} \right| = 0$$

$$\left| \begin{array}{cc} 9000 - 10\omega^2 & -13500 \\ -13500 & 27000 - 10\omega^2 \end{array} \right| = 0$$

$$\omega^2 = \lambda \text{ re}$$

$$(9000 - 10\lambda)(27000 - 10\omega^2) - (-13500)(-13500) = 0$$

$$243000000 - 90000\lambda - 270000\lambda + 100\lambda^2 - 13500^2 = 0$$

$$60750000 - 360000\lambda + 100\lambda^2 = 0$$

$$607500 - 3600\lambda + \lambda^2 = 0$$

$$\lambda_1 = \omega_1^2 = 17750 \rightarrow \omega_1 = 13,32 \text{ rad/s}$$

$$\lambda_2 = \omega_2^2 = 342250 \rightarrow \omega_2 = 58,50 \text{ rad/s}$$

$$T_1 = \frac{2\pi}{\omega_1} = 0,47 \text{ s.}$$

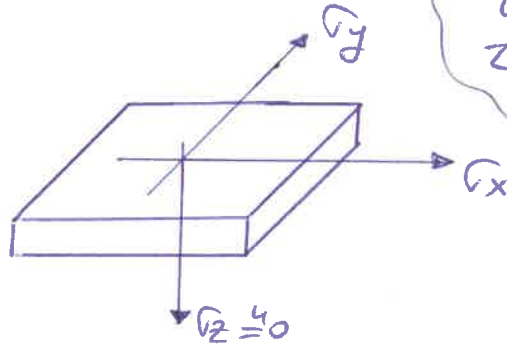
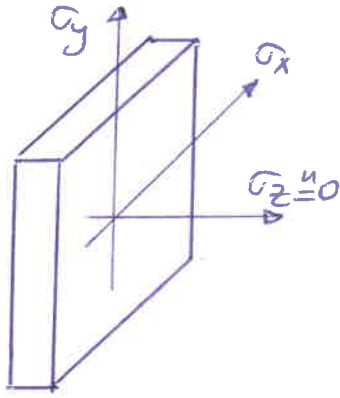
$$T_2 = \frac{2\pi}{\omega_2} = 0,10 \text{ s.}$$

## BÖLÜM VI

### 10) DÜZLEM GERİLME VE DÜZLEM ŞEKİL DEĞİŞTİRME HALİ

#### 10.1) Düzlem Gerilme Halı

Bir elemanın aynı kesitindeki gerilme vektörlerinin doğrultuları aynı düzlem içinde de buna  $\tau_{xy}$  eksenli gerilme halı veya düzlem gerilme halı denir. Levha veya plak gibi elemanlar bu gerilme halı ile ifade edilebilir. Düzlem gerilme halinde eleman kalınlığı boyunca gerilmeler sıfırdır. Buna bağlı olarak da kayma gerilmeleri de sıfırdır (Şekil 10.1)



$$\sigma_z = 0$$

$$\tau_{zx} = \tau_{xz} = 0$$

$$\tau_{zy} = \tau_{yz} = 0$$

Şekil 10.1. Düzlem gerilme halı

Bu durumda gerilme tensörü 3x3 bağımsız olmak üzere 4 skaler büyüklükler olacaktır.

$$[\sigma] = \begin{bmatrix} \sigma_x & \tau_{xy} \\ \tau_{yx} & \sigma_y \end{bmatrix} \quad |\tau_{xy}| = |\tau_{yx}| \quad \text{----- (1)}$$

Birim şekil değişimeler ise;

$$\{\epsilon\} = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u}{\partial x} \\ \frac{\partial v}{\partial y} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \end{Bmatrix} \quad \text{şeklinde dir. ----- (2)}$$

Gerilmeler' selül deęiztikmeler amsından yazarsak;

$$\{\sigma\} = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = [D] \{\epsilon\} = [D] \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} \text{ ---- (3)}$$

İzotrop bir malzemede gerilmeler amsından elastikite baęıntıları;

$$\sigma_x = \frac{E}{1-\nu^2} (\epsilon_x + \nu \epsilon_y)$$

$$\sigma_y = \frac{E}{1-\nu^2} (\nu \epsilon_x + \epsilon_y) \text{ ---- (4)}$$

$$\sigma_z = 0$$

$$\tau_{xy} = \frac{E}{2(1+\nu)} \gamma_{xy}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} \text{ ---- (5)}$$

$$\{\sigma\} = [D] \{\epsilon\} \text{ ---- (6)}$$

Bizim selül deęiztikmeler amsından elastikite baęıntıları;

$$\left. \begin{aligned} \epsilon_x &= \frac{1}{E} (\sigma_x - \nu \sigma_y) \\ \epsilon_y &= \frac{1}{E} (\sigma_y - \nu \sigma_x) \end{aligned} \right\} \epsilon_z = -\frac{\nu}{(1-\nu)} (\epsilon_x + \epsilon_y) \text{ ---- (7)}$$

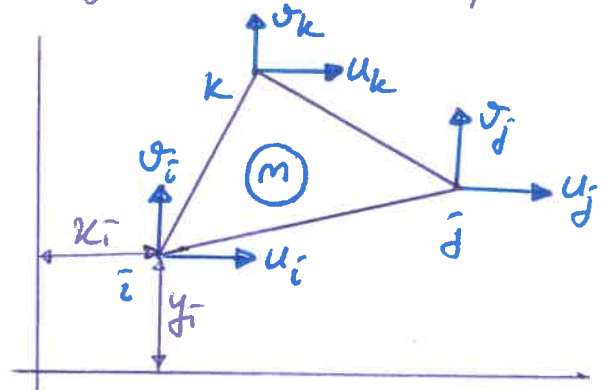
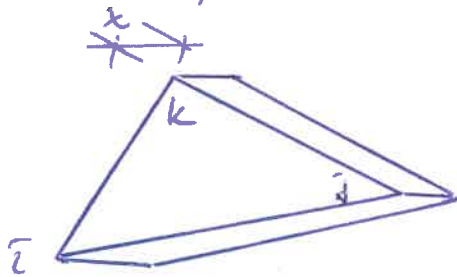
$$\gamma_{xy} = \frac{\tau_{xy}}{G}$$

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -\nu & 0 \\ -\nu & 1 & 0 \\ 0 & 0 & 2(1+\nu) \end{bmatrix} \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} \quad (8)$$

$$\{\epsilon\} = [D]^{-1} \{\sigma\} \quad (9)$$

### 10.2) İnce Levha

$t$  kalınlığında üçgen bir levha alırsak ve bu elemanın her düğüm noktasında  $u$  ve  $v$  yer değiştikmelere sahip olduğunu düşünersek;



Buradaki yer değiştikme;

$$\{d_i\} = \begin{Bmatrix} u_i \\ v_i \end{Bmatrix} \quad \{d_j\} = \begin{Bmatrix} u_j \\ v_j \end{Bmatrix} \quad \{d_k\} = \begin{Bmatrix} u_k \\ v_k \end{Bmatrix} \quad (10)$$

$$\{d\}_e = \begin{Bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{Bmatrix} \quad \text{şeklinde olacaktır.} \quad (11)$$

Şekiller de göreceği üzere iken levharın her düğüm noktasında  $x$  ve  $y$  yönündeki yer değiştirmelerden dolayı 2 serbestlik derecesi bulunmaktadır. Toplam 6 serbestlik derecesi bulunan elemanda düğüm noktalarındaki yer değiştirmelerin koordinatları lineer polinom olarak;

$$\begin{aligned} u &= \alpha_1 + \alpha_2 x + \alpha_3 y \\ v &= \alpha_4 + \alpha_5 x + \alpha_6 y \end{aligned} \quad \text{----- (12)}$$

şeklinde tanımlanabilir. Matris formunda;

$$u^m = \begin{bmatrix} 1 & x & y \end{bmatrix} \begin{bmatrix} \alpha_1 \\ \alpha_2 \\ \alpha_3 \end{bmatrix} \quad \text{----- (13)}$$

$$v^m = \begin{bmatrix} 1 & x & y \end{bmatrix} \begin{bmatrix} \alpha_4 \\ \alpha_5 \\ \alpha_6 \end{bmatrix}$$

şeklinde gösterebiliriz. Bu durumda düğüm noktası koordinatlarına göre yer değiştirme fonksiyonları;

$$\begin{aligned} u_i &= \alpha_1 + \alpha_2 x_i + \alpha_3 y_i \\ v_i &= \alpha_4 + \alpha_5 x_i + \alpha_6 y_i \\ u_j &= \alpha_1 + \alpha_2 x_j + \alpha_3 y_j \\ v_j &= \alpha_4 + \alpha_5 x_j + \alpha_6 y_j \end{aligned} \quad \text{----- (14)}$$

$$u_k = \alpha_1 + \alpha_2 x_k + \alpha_3 y_k$$

$$v_k = \alpha_4 + \alpha_5 x_k + \alpha_6 y_k$$

şeklinde yazılabilir. Matris formunda yazılırsa;

$$\{u_{(n,y)}^m\} = \begin{bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{bmatrix} = \begin{bmatrix} u_i^m \\ u_j^m \\ u_k^m \end{bmatrix} \quad \text{----- (15)}$$

$$\{u_{(n,y)}^m\} = \begin{bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{bmatrix} = \begin{bmatrix} 1 & x_i & y_i & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x_i & y_i \\ 1 & x_j & y_j & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x_j & y_j \\ 1 & x_k & y_k & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x_k & y_k \end{bmatrix} \begin{bmatrix} \alpha_1 \\ \alpha_2 \\ \alpha_3 \\ \alpha_4 \\ \alpha_5 \\ \alpha_6 \end{bmatrix} \quad \text{----- (16)}$$

şeklinde elde edilir. Buradan kapalı ifade;

$$\{u_{(n,y)}^m\} = [h(n,y)] \{\alpha\}^{(17)} \quad \text{şeklinde dir. Buradan da } h(n,y)$$

yaklaşım matrisi, ele alınan  $(m)$  elemanın  $i, j$  veya  $k$  gibi herhangi bir noktasının yer değiştirme vektörünü, düğüm noktalarının yer değiştirme vektörü ile tanımlar.

(14) bağıntısıyla ifade edilen yer değiştirme fonksiyonu  $\alpha$  bilinmeyen katsayılarından ve  $u(n,y)$  düğüm noktası koordinatlarından oluşmaktadır. Matris formunda ifade edilen (17) bağıntısı düzenlenirse;

$$\{\alpha\} = [h(n,y)]^{-1} \{u_{(n,y)}^m\} \quad \text{----- (18)}$$

olur.

$$[h_{xy}]^{-1} = \begin{bmatrix} h_{11} & h_{12} & h_{13} & h_{14} & h_{15} & h_{16} \\ h_{21} & h_{22} & h_{23} & h_{24} & h_{25} & h_{26} \\ h_{31} & h_{32} & h_{33} & h_{34} & h_{35} & h_{36} \\ h_{41} & h_{42} & h_{43} & h_{44} & h_{45} & h_{46} \\ h_{51} & h_{52} & h_{53} & h_{54} & h_{55} & h_{56} \\ h_{61} & h_{62} & h_{63} & h_{64} & h_{65} & h_{66} \end{bmatrix} \quad \text{----- (19)}$$

$$h_{11} = \left[ \frac{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)}{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)} \right]$$

$$h_{21} = \left[ \frac{(y_j - y_k) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)}{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)} \right]$$

$$h_{31} = \left[ \frac{-(x_j - x_k) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)}{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)} \right]$$

$$h_{41} = 0$$

$$h_{51} = 0$$

$$h_{61} = 0$$

$$h_{12} = 0$$

$$h_{22} = 0$$

$$h_{32} = 0$$

$$h_{42} = \left[ \frac{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)}{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)} \right]$$

$$h_{52} = \left[ \frac{(y_j - y_k) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)}{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)} \right]$$

$$h_{62} = \left[ \frac{-(x_j - x_k) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)}{(x_j^* y_k - x_k^* y_j) / (x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j)} \right]$$



$$h_{13} = \left[ - \left( x_i^* y_k - x_k^* y_i \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{23} = \left[ - \left( y_i - y_k \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{33} = \left[ \left( x_i - x_k \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{43} = 0$$

$$h_{53} = 0$$

$$h_{63} = 0$$

---


$$h_{14} = 0$$

$$h_{24} = 0$$

$$h_{34} = 0$$

$$h_{44} = \left[ - \left( x_i^* y_k - x_k^* y_i \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{54} = \left[ - \left( y_i - y_k \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{64} = \left[ \left( x_i - x_k \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

---


$$h_{15} = \left[ \left( x_i^* y_j - x_j^* y_i \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{25} = \left[ \left( y_i - y_j \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{35} = \left[ - \left( x_i - x_j \right) / \left( x_i^* y_j - x_j^* y_i - x_i^* y_k + x_k^* y_i + x_j^* y_k - x_k^* y_j \right) \right]$$

$$h_{45} = 0$$

$$h_{55} = 0$$

$$h_{65} = 0$$

$$h_{16}=0$$

$$h_{26}=0$$

$$h_{36}=0$$

$$h_{46} = \left[ \frac{(\kappa_i^* y_j - \kappa_j^* y_i)}{(\kappa_i^* y_j - \kappa_j^* y_i - \kappa_i^* y_k + \kappa_k^* y_i + \kappa_j^* y_k - \kappa_k^* y_j)} \right]$$

$$h_{56} = \left[ \frac{(y_i - y_j)}{(\kappa_i^* y_j - \kappa_j^* y_i - \kappa_i^* y_k + \kappa_k^* y_i + \kappa_j^* y_k - \kappa_k^* y_j)} \right]$$

$$h_{66} = \left[ \frac{-(\kappa_i - \kappa_j)}{(\kappa_i^* y_j - \kappa_j^* y_i - \kappa_i^* y_k + \kappa_k^* y_i + \kappa_j^* y_k - \kappa_k^* y_j)} \right]$$

Burada;

$$\{\alpha\} = \begin{Bmatrix} \alpha_1 \\ \alpha_2 \\ \alpha_3 \\ \alpha_4 \\ \alpha_5 \\ \alpha_6 \end{Bmatrix} = \begin{bmatrix} h_{11} & h_{12} & h_{13} & h_{14} & h_{15} & h_{16} \\ h_{21} & h_{22} & h_{23} & h_{24} & h_{25} & h_{26} \\ h_{31} & h_{32} & h_{33} & h_{34} & h_{35} & h_{36} \\ h_{41} & h_{42} & h_{43} & h_{44} & h_{45} & h_{46} \\ h_{51} & h_{52} & h_{53} & h_{54} & h_{55} & h_{56} \\ h_{61} & h_{62} & h_{63} & h_{64} & h_{65} & h_{66} \end{bmatrix} \begin{Bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{Bmatrix} \quad \text{de;}$$

Bunun sonucunda;

$$\alpha_1 = \frac{\kappa_j^* y_k - \kappa_k^* y_i}{\kappa_i(y_j - y_k) + \kappa_j(y_k - y_i) + \kappa_k(y_i - y_j)} u_i + \frac{-\kappa_i^* y_k + \kappa_k^* y_i}{\kappa_i(y_j - y_k) + \kappa_j(y_k - y_i) + \kappa_k(y_i - y_j)} u_j + \frac{\kappa_i^* y_j - \kappa_j^* y_i}{\kappa_i(y_j - y_k) + \kappa_j(y_k - y_i) + \kappa_k(y_i - y_j)} u_k$$

$$\alpha_2 = \frac{y_j' - y_k}{x_i^-(y_j' - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i^- - y_j^-)} u_i'$$

$$+ \frac{y_k - y_i'}{x_i^-(y_j' - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i^- - y_j^-)} u_j'$$

$$+ \frac{y_i' - y_j^-}{x_i^-(y_j' - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i^- - y_j^-)} u_k$$

$$\alpha_3 = \frac{x_k - x_j'}{x_i^-(y_j' - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i^- - y_j^-)} u_i'$$

$$+ \frac{x_i' - x_k}{x_i^-(y_j' - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i^- - y_j^-)} u_j'$$

$$+ \frac{x_j' - x_i^-}{x_i^-(y_j' - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i^- - y_j^-)} u_k$$

$$\alpha_4 = \frac{x_j y_k - x_k y_j'}{x_i^-(y_j - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i' - y_j^-)} v_i^-$$

$$+ \frac{x_k y_i' - x_i^- y_k}{x_i^-(y_j - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i' - y_j^-)} v_j^-$$

$$+ \frac{x_i' y_j - x_j' y_i^-}{x_i^-(y_j - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i' - y_j^-)} v_k$$

$$\alpha_5 = \frac{y_j - y_k}{x_i^-(y_j - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i' - y_j^-)} v_i^-$$

$$+ \frac{y_k - y_i'}{x_i^-(y_j - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i' - y_j^-)} v_j^-$$

$$+ \frac{y_i' - y_j^-}{x_i^-(y_j - y_k) + x_j^-(y_k - y_i^-) + x_k^-(y_i' - y_j^-)} v_k$$

$$\alpha_6 = \frac{\kappa_k - \kappa_j^-}{\kappa_i^-(y_j^- - y_k) + \kappa_j^-(y_k - y_i^-) + \kappa_k(y_i^- - y_j^-)} \vartheta_i^-$$

$$+ \frac{\kappa_i^- - \kappa_k}{\kappa_i^-(y_j^- - y_k) + \kappa_j^-(y_k - y_i^-) + \kappa_k(y_i^- - y_j^-)} \vartheta_j^-$$

$$+ \frac{\kappa_j^- - \kappa_i^-}{\kappa_i^-(y_j^- - y_k) + \kappa_j^-(y_k - y_i^-) + \kappa_k(y_i^- - y_j^-)} \vartheta_k$$

$\alpha_1, \alpha_2, \alpha_3, \alpha_4, \alpha_5$  ve  $\alpha_6$  bağıntılarının paydaları determinanlar ve;

$$2\Delta = \det \begin{vmatrix} 1 & \kappa_i^- & y_i^- \\ 1 & \kappa_j^- & y_j^- \\ 1 & \kappa_k & y_k \end{vmatrix} = \kappa_i^-(y_j^- - y_k) + \kappa_j^-(y_k - y_i^-) + \kappa_k(y_i^- - y_j^-)$$

----- (20)

şeklinde olur. Bu durumda  $\alpha'$ 'lar;

$$\alpha_1 = \frac{1}{2\Delta} \left[ (\kappa_j^- y_k - \kappa_k y_j^-) u_i + (\kappa_k y_i^- - \kappa_i^- y_k) u_j^- + (\kappa_i^- y_j^- - \kappa_j^- y_i^-) u_k \right]$$

$$\alpha_2 = \frac{1}{2\Delta} \left[ (y_j^- - y_k) u_i^- + (y_k - y_i^-) u_j^- + (y_i^- - y_j^-) u_k \right]$$

$$\alpha_3 = \frac{1}{2\Delta} \left[ (\kappa_k - \kappa_j^-) u_i + (\kappa_i^- - \kappa_k) u_j^- + (\kappa_j^- - \kappa_i^-) u_k \right] \text{ ----- (21)}$$

$$\alpha_4 = \frac{1}{2\Delta} \left[ (\kappa_j^- y_k - \kappa_k y_j^-) \vartheta_i^- + (\kappa_k y_i^- - \kappa_i^- y_k) \vartheta_j^- + (\kappa_i^- y_j^- - \kappa_j^- y_i^-) \vartheta_k \right]$$

$$\alpha_5 = \frac{1}{2\Delta} \left[ (y_i^- - y_k) \vartheta_i^- + (y_k - y_i^-) \vartheta_j^- + (y_i^- - y_j^-) \vartheta_k \right]$$

$$\alpha_6 = \frac{1}{2\Delta} \left[ (\kappa_k - \kappa_j^-) \vartheta_i^- + (\kappa_i^- - \kappa_k) \vartheta_j^- + (\kappa_j^- - \kappa_i^-) \vartheta_k \right]$$

şeklini alacaktır

(14) bağıntılarını (12) bağıntısına adapte edersek;

$$u = (\alpha_1 + \alpha_2 x_i + \alpha_3 y_i) u_i + (\alpha_1 + \alpha_2 x_j + \alpha_3 y_j) u_j + (\alpha_1 + \alpha_2 x_k + \alpha_3 y_k) u_k$$

$$v = (\alpha_4 + \alpha_5 x_i + \alpha_6 y_i) v_i + (\alpha_4 + \alpha_5 x_j + \alpha_6 y_j) v_j + (\alpha_4 + \alpha_5 x_k + \alpha_6 y_k) v_k$$

şeklini alır. Buradan  $\alpha$  değerleri için aşağıdaki kısıtlamaları yaparsak (21) bağıntısı;

$$\begin{array}{ccc|ccc} a_i = x_j y_k - x_k y_j & a_j = x_k y_i - x_i y_k & a_k = x_i y_j - x_j y_i \\ b_i = y_j - y_k & b_j = y_k - y_i & b_k = y_i - y_j \\ c_i = x_k - x_j & c_j = x_i - x_k & c_k = x_j - x_i \end{array}$$

$$\alpha_1 = \frac{1}{2\Delta} [a_i u_i + a_j u_j + a_k u_k]$$

$$\alpha_2 = \frac{1}{2\Delta} [b_i u_i + b_j u_j + b_k u_k]$$

$$\alpha_3 = \frac{1}{2\Delta} [c_i u_i + c_j u_j + c_k u_k] \quad \text{----- (22)}$$

$$\alpha_4 = \frac{1}{2\Delta} [a_i v_i + a_j v_j + a_k v_k]$$

$$\alpha_5 = \frac{1}{2\Delta} [b_i v_i + b_j v_j + b_k v_k]$$

$$\alpha_6 = \frac{1}{2\Delta} [c_i v_i + c_j v_j + c_k v_k]$$

şeklini alır.  $u$  ve  $v$  bağıntıları ise;

$$u = \frac{1}{2\Delta} \left[ (a_i + b_i x + c_i y) u_i + (a_j + b_j x + c_j y) u_j + (a_k + b_k x + c_k y) u_k \right]$$

$$v = \frac{1}{2\Delta} \left[ (a_i + b_i x + c_i y) v_i + (a_j + b_j x + c_j y) v_j + (a_k + b_k x + c_k y) v_k \right]$$

(23) bağıntısındaki parantez içi terimler düğüm noktalarına ait şekil fonksiyonlarını ifade etmektedir. ----- (23)

$$N_i = \frac{1}{2\Delta} (a_i + b_i x + c_i y)$$

$$N_j = \frac{1}{2\Delta} (a_j + b_j x + c_j y) \text{ ----- (24)}$$

$$N_k = \frac{1}{2\Delta} (a_k + b_k x + c_k y)$$

Matriks formunda yazılırsa;

$$\{u\}_e = [N] \{d\}$$

$$\begin{Bmatrix} u \\ v \end{Bmatrix} = \begin{bmatrix} N_i & 0 & N_j & 0 & N_k & 0 \\ 0 & N_i & 0 & N_j & 0 & N_k \end{bmatrix} \begin{Bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{Bmatrix} \text{ ----- (25)}$$

diğer birim şekil değişkenler;

$$\{\varepsilon\} = \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix} = \underbrace{[\Delta][N]}_{[G]} \{d\} = [G] \{d\} \text{ 'dir.}$$

$$\{\varepsilon\} = \frac{1}{2\Delta} \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix} \begin{bmatrix} N_i & 0 & N_j & 0 & N_k & 0 \\ 0 & N_i & 0 & N_j & 0 & N_k \end{bmatrix} \{d\}$$

$$\{\epsilon\} = \frac{1}{2\Delta} \begin{bmatrix} \frac{\partial N_i}{\partial x} & 0 & \frac{\partial N_j}{\partial x} & 0 & \frac{\partial N_k}{\partial x} & 0 \\ 0 & \frac{\partial N_i}{\partial y} & 0 & \frac{\partial N_j}{\partial y} & 0 & \frac{\partial N_k}{\partial y} \\ \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & \frac{\partial N_j}{\partial y} & \frac{\partial N_j}{\partial x} & \frac{\partial N_k}{\partial y} & \frac{\partial N_k}{\partial x} \end{bmatrix} \{d\}$$

$$\{\epsilon\} = \frac{1}{2\Delta} \begin{bmatrix} b_i & 0 & b_j & 0 & b_k & 0 \\ 0 & c_i & 0 & c_j & 0 & c_k \\ c_i & b_i & c_j & b_j & c_k & b_k \end{bmatrix} \begin{Bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{Bmatrix}$$

İzotrop malzeme için gerilme bağıntıları;

$$\{\sigma\} = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = [D] \{\epsilon\} = [D][B] \{d\} \text{ ---- (26)}$$

$$\{\sigma\} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \frac{1}{2\Delta} \begin{bmatrix} b_i & 0 & b_j & 0 & b_k & 0 \\ 0 & c_i & 0 & c_j & 0 & c_k \\ c_i & b_i & c_j & b_j & c_k & b_k \end{bmatrix} \begin{Bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \end{Bmatrix}$$



$$\{\sigma\} = \frac{E}{2\Delta(1-\nu^2)} \begin{bmatrix} b_i & \nu c_i & b_j & \nu c_j & b_k & \nu c_k \\ \nu b_i & c_i & \nu b_j & c_j & \nu b_k & c_k \\ \frac{1-\nu}{2} c_i & \frac{1-\nu}{2} b_i & \frac{1-\nu}{2} c_j & \frac{1-\nu}{2} b_j & \frac{1-\nu}{2} c_k & \frac{1-\nu}{2} b_k \end{bmatrix} \begin{Bmatrix} u_i \\ \sigma_i \\ u_j \\ \sigma_j \\ u_k \\ \sigma_k \end{Bmatrix}$$

$$\{\sigma\} = [D][G]\{\alpha\} \text{ elde edilir. } \text{--- (27)}$$

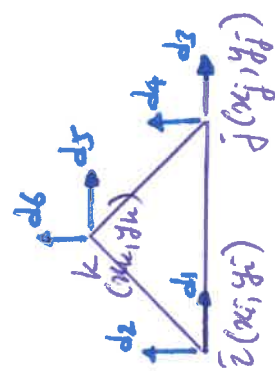
Eleman rijitlik matrisi de;

$$[k_e] = \int [G]^T [D] [G] dV \text{--- (28)}$$

$$[k_e] = \frac{Et}{4\Delta^2(1-\nu^2)} \int \begin{bmatrix} b_i & 0 & c_i \\ 0 & c_i & b_i \\ b_j & 0 & c_j \\ 0 & c_j & b_j \\ b_k & 0 & c_k \\ 0 & c_k & b_k \end{bmatrix} \begin{bmatrix} b_i & \nu c_i & b_j & \nu c_j & b_k & \nu c_k \\ \nu b_i & c_i & \nu b_j & c_j & \nu b_k & c_k \\ \frac{1-\nu}{2} c_i & \frac{1-\nu}{2} b_i & \frac{1-\nu}{2} c_j & \frac{1-\nu}{2} b_j & \frac{1-\nu}{2} c_k & \frac{1-\nu}{2} b_k \end{bmatrix} dx dy$$

$$[k_e] = \frac{Et}{4\Delta^2(1-\nu^2)} \iint \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} & k_{15} & k_{16} \\ k_{21} & k_{22} & k_{23} & k_{24} & k_{25} & k_{26} \\ k_{31} & k_{32} & k_{33} & k_{34} & k_{35} & k_{36} \\ k_{41} & k_{42} & k_{43} & k_{44} & k_{45} & k_{46} \\ k_{51} & k_{52} & k_{53} & k_{54} & k_{55} & k_{56} \\ k_{61} & k_{62} & k_{63} & k_{64} & k_{65} & k_{66} \end{bmatrix} dx dy \text{--- (29)}$$

$$[k_e] = \frac{Et}{4\Delta^2(1-v^2)} \iint \begin{bmatrix} b_i^2 + \left(\frac{1-v}{2}\right)c_i^2 & vb_i c_i + \left(\frac{1-v}{2}\right)b_i c_i & b_i b_j + \left(\frac{1-v}{2}\right)c_i c_j & vb_i c_j + \left(\frac{1-v}{2}\right)b_j c_j & b_i b_k + \left(\frac{1-v}{2}\right)c_i c_k & vb_i c_k + \left(\frac{1-v}{2}\right)b_k c_i \\ vb_i c_i + \left(\frac{1-v}{2}\right)b_i c_i & \left(\frac{1-v}{2}\right)b_i^2 + c_i^2 & vb_j c_i + \left(\frac{1-v}{2}\right)b_i c_j & c_i c_j + \left(\frac{1-v}{2}\right)b_i b_j & vb_k c_i + \left(\frac{1-v}{2}\right)b_i c_k & c_i c_k + \left(\frac{1-v}{2}\right)b_i b_k \\ b_i b_j + \left(\frac{1-v}{2}\right)c_i c_j & vb_j c_i + \left(\frac{1-v}{2}\right)b_i c_j & b_j^2 + \left(\frac{1-v}{2}\right)c_j^2 & vb_j c_j + \left(\frac{1-v}{2}\right)b_j c_j & b_j b_k + \left(\frac{1-v}{2}\right)c_j c_k & vb_j c_k + \left(\frac{1-v}{2}\right)b_k c_j \\ vb_i c_j + \left(\frac{1-v}{2}\right)b_j c_j & vb_j c_i + \left(\frac{1-v}{2}\right)b_i c_j & vb_j c_j + \left(\frac{1-v}{2}\right)b_j c_j & \left(\frac{1-v}{2}\right)b_j^2 + c_j^2 & vb_k c_j + \left(\frac{1-v}{2}\right)b_j c_k & c_j c_k + \left(\frac{1-v}{2}\right)b_j b_k \\ b_i b_k + \left(\frac{1-v}{2}\right)c_i c_k & vb_k c_i + \left(\frac{1-v}{2}\right)b_i c_k & vb_k c_j + \left(\frac{1-v}{2}\right)b_j c_k & vb_k c_j + \left(\frac{1-v}{2}\right)b_j c_k & b_k^2 + \left(\frac{1-v}{2}\right)c_k^2 & vb_k c_k + \left(\frac{1-v}{2}\right)b_k c_k \\ vb_i c_k + \left(\frac{1-v}{2}\right)b_k c_i & vb_i c_k + \left(\frac{1-v}{2}\right)b_k c_i & vb_i c_k + \left(\frac{1-v}{2}\right)b_k c_i & vb_i c_k + \left(\frac{1-v}{2}\right)b_k c_i & vb_i c_k + \left(\frac{1-v}{2}\right)b_k c_i & \left(\frac{1-v}{2}\right)b_k^2 + c_k^2 \end{bmatrix} \quad \text{--- (30)}$$



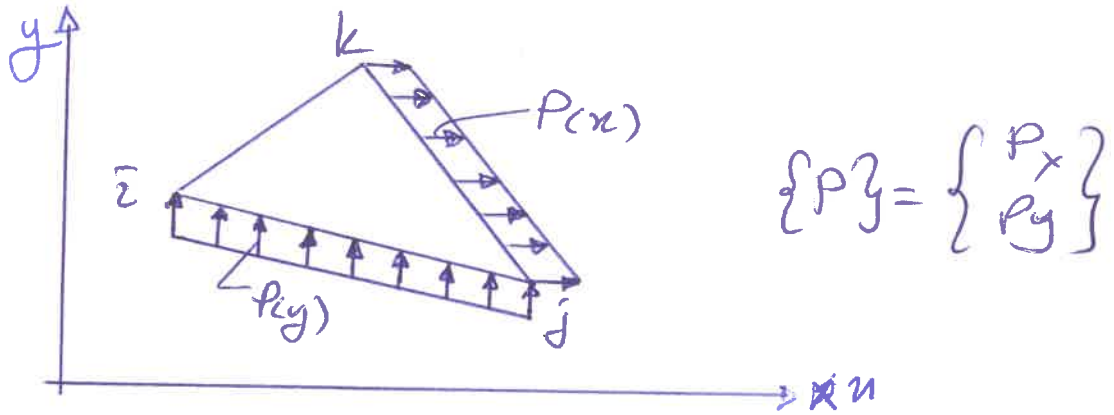
$$\begin{aligned} a_i &= x_j y_k - x_k y_j \\ b_i &= y_j - y_k \\ c_i &= x_j - x_k \end{aligned}$$

$$\begin{aligned} a_j &= x_k y_i - x_i y_k \\ b_j &= y_k - y_i \\ c_j &= x_k - x_i \end{aligned}$$

$$\begin{aligned} a_k &= x_i y_j - x_j y_i \\ b_k &= y_i - y_j \\ c_k &= x_i - x_j \end{aligned}$$

### 10.2.1) Yük Matrisi

#### a) Düzgün Yayılı Yük Durumu (Edge load)



Sisteme etkiyen yayılı yük olması durumunda;

$$\{q\}_s = - \int [N]^T \{P\} d\Omega$$

$$\{q_i\} = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_i d\Omega$$

$$\{q_j\} = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_j d\Omega \quad \text{--- (31)}$$

$$\{q_k\} = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_k d\Omega$$

$N_i$ ,  $N_j$  ve  $N_k$  şeklin fonksiyonlarını (31) bağıntısındaki yere yerine yazarsak;

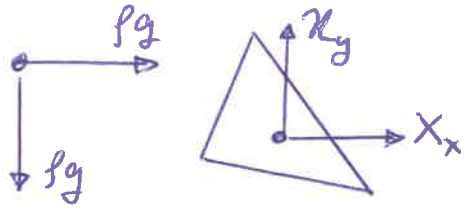
$$\{q_i\} = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int \frac{1}{2\Delta} (a_i + b_i x + c_i y) d\Omega$$

$$= - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int \frac{1}{2\Delta} a_i d\Omega = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \frac{a_i}{2} \quad \text{--- (32)}$$

$$\{q_j\} = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int \frac{1}{2\Delta} (a_j + b_j x + c_j y) d\Omega$$

$$\{q_k\} = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int \frac{1}{2\Delta} (a_k + b_k x + c_k y) d\Omega$$

## b) Küresel yük (Body Forces)



$$\{x\} = \begin{Bmatrix} x_x(x,y) \\ x_y(x,y) \end{Bmatrix} = \begin{Bmatrix} f_g \\ -f_g \end{Bmatrix}$$

$f = \text{yöğünlük}$

$$W = \text{ağırlık} = A t (f_g)$$

$$\{x\} = \begin{Bmatrix} f_g \\ -f_g \end{Bmatrix}$$

$$\{f\}_x = - \int_V [N]^T \{x\} dV \quad \text{--- (33)}$$

## c) Yayıllı kütle (Consistent mass)

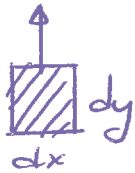
$$[M] = \int_V \rho [N]^T [N] dV \quad \text{--- (34)}$$

$$[M] = \begin{bmatrix} 1/6 & 1/12 & 1/12 & 0 & 0 & 0 \\ 1/12 & 1/6 & 1/12 & 0 & 0 & 0 \\ 1/12 & 1/12 & 1/6 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1/6 & 1/12 & 1/12 \\ 0 & 0 & 0 & 1/12 & 1/6 & 1/12 \\ 0 & 0 & 0 & 1/12 & 1/12 & 1/6 \end{bmatrix} \quad \text{A.f.t --- (35)}$$

d) 1st Degizim

$$\{q\}_t = -\alpha_t (\Delta t) \left( \int [G]^T dV \right) \{D_T\} \quad (36)$$

1st degiziminde elastikite matrisi  $\{D_T\}$ ;



$$\begin{cases} \epsilon_x = \alpha_{tx} (\Delta t) \\ \epsilon_y = \alpha_{ty} (\Delta t) \\ \gamma_{xy} = 0 \end{cases}$$

$$\{\sigma\} = [D] \{\epsilon\}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{21} & D_{22} & 0 \\ 0 & 0 & D_{33} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

$$\{\epsilon\} = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{Bmatrix} \alpha_{tx} (\Delta T) \\ \alpha_{ty} (\Delta T) \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{21} & D_{22} & 0 \\ 0 & 0 & D_{33} \end{bmatrix} \begin{Bmatrix} \alpha_{tx} \cdot \Delta T \\ \alpha_{ty} \cdot \Delta T \\ 0 \end{Bmatrix} = \begin{Bmatrix} \alpha_{tx} D_{11} + \alpha_{ty} D_{12} \\ \alpha_{tx} D_{21} + \alpha_{ty} D_{22} \\ 0 \end{Bmatrix} \Delta T$$

$$\{\sigma\} = [D] \{\epsilon\}$$

$$= \begin{Bmatrix} D_{11} + D_{12} \\ D_{21} + D_{22} \\ 0 \end{Bmatrix} \alpha \cdot \Delta T$$

$$= \alpha \cdot \Delta T \cdot \frac{E}{1-\nu} \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix}$$

matzene ortotropix  $\alpha_{tx}, \alpha_{ty}$  'lar

" izotropix  $\alpha_{tx} = \alpha_{ty} = \alpha_t$

izotrop matzene;

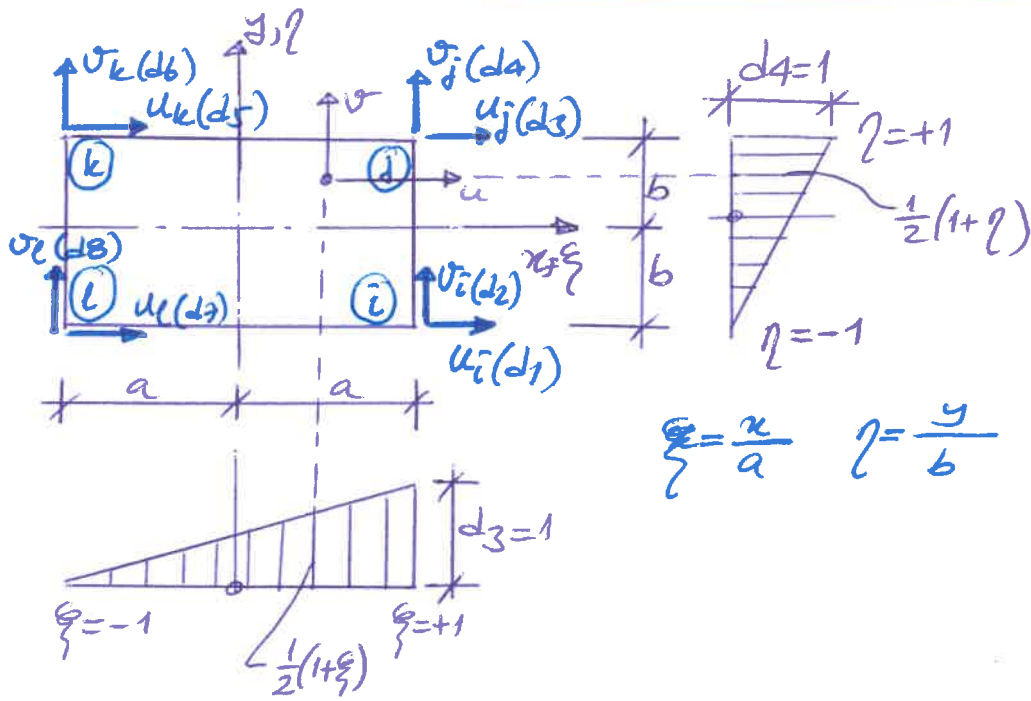
$$D_{11} + D_{12} = \frac{E}{1-\nu^2} + \frac{\nu E}{1-\nu^2} = \frac{E}{1-\nu}$$

elke edine

$$D_{21} + D_{22} = \frac{E}{1-\nu}$$

$$\{D\}_T = \frac{E}{1-\nu} \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix}$$

### 10.3) İnce levhaların Dörtgen Elemanlara Hesabı



$$\xi = \frac{x}{a} \quad \eta = \frac{y}{b}$$

Şekil fonksiyonu  $N_i = \frac{1}{4} (1 + \xi \xi_i) (1 + \eta \eta_i) \quad \text{--- (37)}$

$$\{u\}_e = [N] \{d\}$$

$$\begin{Bmatrix} u \\ v \end{Bmatrix} = \begin{bmatrix} N_i & 0 & N_j & 0 & N_k & 0 & N_l & 0 \\ 0 & N_i & 0 & N_j & 0 & N_k & 0 & N_l \end{bmatrix} \begin{Bmatrix} u_i \\ v_i \\ u_j \\ v_j \\ u_k \\ v_k \\ u_l \\ v_l \end{Bmatrix} \quad \text{--- (38)}$$

$$\{d\}^T = [u_i, v_i; u_j, v_j; u_k, v_k; u_l, v_l] \quad \text{--- (39)}$$

Sevilen dörtgen elemanın her dörtgen noktasında 2 yerdışı birleşim bulunduktan Elemanın şekil fonksiyonu (37) başından ile verilmektedir. Buna göre dörtgen noktasının şekil fonksiyonları;

$$\begin{array}{l|ll}
 N_i = \frac{1}{4} (1+\xi)(1+\eta) & \xi_i = +1 & \eta_i = +1 \\
 N_j = \frac{1}{4} (1+\xi)(1-\eta) & \xi_i = +1 & \eta_i = -1 \\
 N_k = \frac{1}{4} (1-\xi)(1+\eta) & \xi_i = -1 & \eta_i = +1 \\
 N_l = \frac{1}{4} (1-\xi)(1-\eta) & \xi_i = -1 & \eta_i = -1
 \end{array} \quad \text{--- (40)}$$

$$\{\xi\} = \begin{Bmatrix} \xi_x \\ \xi_y \\ \xi_{xy} \end{Bmatrix} = \begin{Bmatrix} \partial u / \partial x \\ \partial u / \partial y \\ \partial u / \partial y + \partial u / \partial x \end{Bmatrix}$$

$$\{\xi\} = \begin{bmatrix} \frac{\partial N_i}{\partial x} & 0 & \frac{\partial N_j}{\partial x} & 0 & \frac{\partial N_k}{\partial x} & 0 & \frac{\partial N_l}{\partial x} & 0 \\ 0 & \frac{\partial N_i}{\partial y} & 0 & \frac{\partial N_j}{\partial y} & 0 & \frac{\partial N_k}{\partial y} & 0 & \frac{\partial N_l}{\partial y} \\ \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & \frac{\partial N_j}{\partial y} & \frac{\partial N_j}{\partial x} & \frac{\partial N_k}{\partial y} & \frac{\partial N_k}{\partial x} & \frac{\partial N_l}{\partial y} & \frac{\partial N_l}{\partial x} \end{bmatrix} \{d\}$$

$$\begin{aligned}
 \frac{\partial N_i}{\partial x} &= \frac{\partial}{\partial x} \left[ \frac{1}{4} (1+\xi + \eta + \xi\eta) \right] = \frac{1}{4a} (1+\eta) & \left\| \begin{aligned} \frac{\partial N}{\partial x} &= \frac{\partial N}{\partial \xi} \frac{\partial \xi}{\partial x} = \frac{\partial N}{\partial \xi} \cdot \frac{1}{a} \\ \frac{\partial N}{\partial y} &= \frac{\partial N}{\partial \eta} \frac{\partial \eta}{\partial y} = \frac{\partial N}{\partial \eta} \cdot \frac{1}{b} \end{aligned} \right. \\
 \frac{\partial N_j}{\partial x} &= \frac{\partial}{\partial x} \left[ \frac{1}{4} (1+\xi - \eta - \xi\eta) \right] = \frac{1}{4a} (1-\eta) \\
 \frac{\partial N_k}{\partial x} &= \frac{\partial}{\partial x} \left[ \frac{1}{4} (1-\xi + \eta - \xi\eta) \right] = -\frac{1}{4a} (1+\eta) \\
 \frac{\partial N_l}{\partial x} &= \frac{\partial}{\partial x} \left[ \frac{1}{4} (1-\xi - \eta + \xi\eta) \right] = -\frac{1}{4a} (1-\eta)
 \end{aligned} \quad \text{--- (41)}$$

$$\frac{\partial N_k}{\partial x} = \frac{\partial}{\partial x} \left[ \frac{1}{4} (1-\xi + \eta - \xi\eta) \right] = -\frac{1}{4a} (1+\eta)$$

$$\begin{aligned}
 \frac{\partial N_l}{\partial x} &= \frac{\partial}{\partial x} \left[ \frac{1}{4} (1-\xi - \eta + \xi\eta) \right] = -\frac{1}{4a} (1-\eta) \\
 &= -\frac{1}{4a} (1-\eta)
 \end{aligned}$$



$$\frac{\partial N_i}{\partial y} = \frac{1}{4b} (1+\xi) ; \quad \frac{\partial N_j}{\partial y} = -\frac{1}{4b} (1+\xi)$$

$$\frac{\partial N_k}{\partial y} = \frac{1}{4b} (1-\xi) ; \quad \frac{\partial N_l}{\partial y} = -\frac{1}{4b} (1-\xi)$$

$$\{\epsilon\} = \frac{1}{4ab} \begin{bmatrix} b(1+\eta) & 0 & b(1-\eta) & 0 & -b(1+\eta) & 0 & -b(1-\eta) & 0 \\ 0 & a(1+\xi) & 0 & -a(1+\xi) & 0 & a(1-\xi) & 0 & -a(1-\xi) \\ a(1+\xi) & b(1+\eta) & -a(1+\xi) & b(1-\eta) & a(1-\xi) & -b(1+\eta) & -a(1-\xi) & -b(1-\eta) \end{bmatrix} \{d\}$$

----- (42)

$$\{\sigma\} = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = [D] \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} \quad [D] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \quad \begin{matrix} \text{Isotrop material} \\ \text{T.M.N} \end{matrix}$$

$$\{\epsilon\} = \underbrace{[D][N]}_{[G]} \{d\} \longrightarrow \{\sigma\} = [D][G] \{d\}$$

$$\{\sigma\} = \frac{1}{4ab} \frac{E}{(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} b(1+\eta) & 0 & b(1-\eta) & 0 & -b(1+\eta) & 0 & -b(1-\eta) & 0 \\ 0 & a(1+\xi) & 0 & -a(1+\xi) & 0 & a(1-\xi) & 0 & -a(1-\xi) \\ a(1+\xi) & b(1+\eta) & -a(1+\xi) & b(1-\eta) & a(1-\xi) & -b(1+\eta) & -a(1-\xi) & -b(1-\eta) \end{bmatrix} \{d\}$$

$\downarrow$   
 $[D]$

$\downarrow$   
 $[G]$

$$\{a\} = \frac{E}{4ab(1-v^2)} \begin{bmatrix} b(1+\eta) & va(1+\xi) & b(1-\eta) & -va(1+\xi) & -b(1+\eta) & va(1-\xi) \\ vb(1+\eta) & a(1+\xi) & vb(1-\eta) & -a(1+\xi) & -vb(1-\eta) & a(1-\xi) \\ a\left(\frac{1-v}{2}\right)(1+\xi) & b\left(\frac{1-v}{2}\right)(1+\eta) & -a\left(\frac{1-v}{2}\right)(1+\xi) & b\left(\frac{1-v}{2}\right)(1-\eta) & -a\left(\frac{1-v}{2}\right)(1-\xi) & -b\left(\frac{1-v}{2}\right)(1-\eta) \end{bmatrix} \{a\}$$

$$\uparrow$$

$$[D][\epsilon]$$

Element Stiff Matrix

$$[k_e] = \int_V [G]^T [D] [G] dV$$

$$[k_e] = \frac{t}{16a^2b^2(1-v^2)} \begin{bmatrix} b(1+\eta) & 0 & a(1+\xi) & 0 & 0 & 0 \\ 0 & a(1+\xi) & b(1+\eta) & 0 & 0 & 0 \\ b(1-\eta) & 0 & -a(1+\xi) & 0 & 0 & 0 \\ 0 & -a(1+\xi) & b(1-\eta) & 0 & 0 & 0 \\ -b(1+\eta) & 0 & a(1-\xi) & 0 & 0 & 0 \\ 0 & a(1-\xi) & -b(1+\eta) & 0 & 0 & 0 \\ -b(1-\eta) & 0 & -a(1-\xi) & 0 & 0 & 0 \\ 0 & -a(1-\xi) & -b(1-\eta) & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 1 & v & 0 \\ v & 1 & 0 \\ 0 & 0 & \frac{1-v}{2} \end{bmatrix} \begin{bmatrix} b(1+\eta) & 0 & b(1-\eta) & 0 & b(1+\eta) & 0 \\ 0 & a(1+\xi) & 0 & -a(1-\xi) & 0 & a(1-\xi) \\ a(1+\xi) & b(1+\eta) & -a(1+\xi) & b(1-\eta) & a(1-\xi) & -b(1-\eta) \end{bmatrix} dx dy$$

$$[k_e] = \frac{Et}{16a^2b^2(1-v^2)} \iint_{-a}^a \iint_{-b}^b \begin{bmatrix} \dots \end{bmatrix} dx dy$$

$$\xi = \frac{x}{a} \quad \eta = \frac{y}{b}$$

$$[k_e] = \frac{Et}{16a^2b^3(1-\nu^2)} \frac{1}{A} \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} & k_{15} & k_{16} & k_{17} & k_{18} \\ k_{21} & k_{22} & k_{23} & k_{24} & k_{25} & k_{26} & k_{27} & k_{28} \\ k_{31} & k_{32} & k_{33} & k_{34} & k_{35} & k_{36} & k_{37} & k_{38} \\ k_{41} & k_{42} & k_{43} & k_{44} & k_{45} & k_{46} & k_{47} & k_{48} \\ k_{51} & k_{52} & k_{53} & k_{54} & k_{55} & k_{56} & k_{57} & k_{58} \\ k_{61} & k_{62} & k_{63} & k_{64} & k_{65} & k_{66} & k_{67} & k_{68} \\ k_{71} & k_{72} & k_{73} & k_{74} & k_{75} & k_{76} & k_{77} & k_{78} \\ k_{81} & k_{82} & k_{83} & k_{84} & k_{85} & k_{86} & k_{87} & k_{88} \end{bmatrix} dx dy$$

$$k_{11} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ b^2(1+\eta)^2 + a^2\left(\frac{1-\nu}{2}\right)\left(1+\frac{\xi}{2}\right)^2 \right] dx dy$$

$$k_{12} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1+\eta)\left(1+\frac{\xi}{2}\right) + \left(\frac{1-\nu}{2}\right)ab(1+\eta)\left(1+\frac{\xi}{2}\right) \right] dx dy$$

$$k_{13} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ b^2(1+\eta)(1-\eta) - a^2\left(\frac{1-\nu}{2}\right)\left(1+\frac{\xi}{2}\right)^2 \right] dx dy$$

$$k_{14} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1+\frac{\xi}{2})(1+\eta) + ab\left(\frac{1-\nu}{2}\right)\left(1+\frac{\xi}{2}\right)(1-\eta) \right] dx dy$$

$$k_{15} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -b^2(1+\eta)^2 + a^2\left(\frac{1-\nu}{2}\right)\left(1+\frac{\xi}{2}\right)\left(1-\frac{\xi}{2}\right) \right] dx dy$$

$$k_{16} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1+\eta)\left(1-\frac{\xi}{2}\right) - ab\left(\frac{1-\nu}{2}\right)(1+\eta)\left(1+\frac{\xi}{2}\right) \right] dx dy$$

$$k_{17} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -b^2(1+\eta)(1-\eta) - a^2\left(\frac{1-\nu}{2}\right)\left(1+\frac{\xi}{2}\right)\left(1-\frac{\xi}{2}\right) \right] dx dy$$

$$k_{18} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1+\eta)\left(1-\frac{\xi}{2}\right) - ab\left(\frac{1-\nu}{2}\right)\left(1+\frac{\xi}{2}\right)(1-\eta) \right] dx dy$$

$$k_{21} = k_{12}$$

$$k_{22} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ a^2(1+\xi)^2 + b^2\left(\frac{1-\nu}{2}\right)(1+\eta)^2 \right] dA$$

$$k_{23} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1-\eta)(1+\xi) - ab\left(\frac{1-\nu}{2}\right)(1+\eta)(1+\xi) \right] dx dy$$

$$k_{24} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -a^2(1+\xi)^2 + b^2\left(\frac{1-\nu}{2}\right)(1-\eta)(1+\eta) \right] dx dy$$

$$k_{25} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1+\eta)(1+\xi) + ab\left(\frac{1-\nu}{2}\right)(1+\eta)(1-\xi) \right] dx dy$$

$$k_{26} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ a^2(1+\xi)(1-\xi) - b^2\left(\frac{1-\nu}{2}\right)(1+\eta)^2 \right] dx dy$$

$$k_{27} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1+\xi)(1-\eta) - ab\left(\frac{1-\nu}{2}\right)(1+\eta)(1-\xi) \right] dx dy$$

$$k_{28} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -a^2(1+\xi)(1-\xi) - b^2\left(\frac{1-\nu}{2}\right)(1+\eta)(1-\eta) \right] dx dy$$

$$k_{31} = k_{13}$$

$$k_{32} = k_{23}$$

$$k_{33} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ b^2(1-\eta)^2 + a^2\left(\frac{1-\nu}{2}\right)(1+\xi)^2 \right] dx dy$$

$$k_{34} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1-\eta)(1+\xi) - ab\left(\frac{1-\nu}{2}\right)(1-\eta)(1+\xi) \right] dx dy$$

$$k_{35} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -b^2(1+\eta)(1-\eta) - a^2\left(\frac{1-\nu}{2}\right)(1+\xi)(1-\xi) \right] dx dy$$

$$k_{36} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1-\eta)(1-\xi) + ab\left(\frac{1-\nu}{2}\right)(1+\eta)(1+\xi) \right] dx dy$$

$$k_{37} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -b^2(1-\eta)^2 + a^2\left(\frac{1-\nu}{2}\right)(1+\xi)(1-\xi) \right] dx dy$$

$$k_{38} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1-\eta)(1-\xi) + ab\left(\frac{1-\nu}{2}\right)(1+\xi)(1-\eta) \right] dx dy$$

$$k_{41} = k_{14}$$

$$k_{42} = k_{24}$$

$$k_{43} = k_{34}$$

$$k_{44} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ a^2(1+\xi)^2 + b^2\left(\frac{1-\nu}{2}\right)(1-\eta)^2 \right]$$

$$k_{45} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1+\eta)(1+\xi) + ab\left(\frac{1-\nu}{2}\right)(1-\eta)(1-\xi) \right] dx dy$$

$$k_{46} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -a^2(1+\xi)(1-\xi) - b^2\left(\frac{1-\nu}{2}\right)(1+\eta)(1-\eta) \right] dx dy$$

$$k_{47} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1-\eta)(1+\xi) - ab\left(\frac{1-\nu}{2}\right)(1-\eta)(1-\xi) \right] dx dy$$

$$k_{48} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ a^2(1+\xi)(1-\xi) - b^2\left(\frac{1-\nu}{2}\right)(1-\eta)^2 \right] dx dy$$



$$k_{51} = k_{15}$$

$$k_{52} = k_{25}$$

$$k_{53} = k_{35}$$

$$k_{54} = k_{45}$$

$$k_{55} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ b^2(1+\eta)^2 + a^2\left(\frac{1-\nu}{2}\right)\left(1-\xi\right)^2 \right] dx dy$$

$$k_{56} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab(1+\eta)\left(1-\xi\right) - ab\left(\frac{1-\nu}{2}\right)(1+\eta)\left(1-\xi\right) \right] dx dy$$

$$k_{57} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ b^2(1+\eta)(1-\eta) - a^2\left(\frac{1-\nu}{2}\right)\left(1-\xi\right)^2 \right] dx dy$$

$$k_{58} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1+\eta)\left(1-\xi\right) - ab\left(\frac{1-\nu}{2}\right)(1-\eta)\left(1-\xi\right) \right] dx dy$$

$$k_{61} = k_{16}$$

$$k_{62} = k_{26}$$

$$k_{63} = k_{36}$$

$$k_{64} = k_{46}$$

$$k_{65} = k_{56}$$

$$k_{66} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ a^2 \left(1 - \xi\right)^2 + b^2 \left(\frac{1-\nu}{2}\right) (1+\eta)^2 \right] dx dy$$

$$k_{67} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -\nu ab (1-\eta) \left(1 - \xi\right) + ab \left(\frac{1-\nu}{2}\right) (1+\eta) \left(1 - \xi\right) \right] dx dy$$

$$k_{68} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ -a^2 \left(1 - \xi\right)^2 + b^2 \left(\frac{1-\nu}{2}\right) (1+\eta) (1-\eta) \right] dx dy$$

$$k_{71} = k_{17}$$

$$k_{72} = k_{27}$$

$$k_{73} = k_{37}$$

$$k_{74} = k_{47}$$

$$k_{75} = k_{57}$$

$$k_{76} = k_{67}$$

$$k_{77} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ b^2(1-\eta)^2 + a^2\left(\frac{1-\nu}{2}\right)(1-\xi)^2 \right] dx dy$$

$$k_{78} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ \nu ab(1-\eta)(1-\xi) + ab\left(\frac{1-\nu}{2}\right)(1-\eta)(1-\xi) \right] dx dy$$

$$k_{81} = k_{18}$$

$$k_{84} = k_{48}$$

$$k_{82} = k_{28}$$

$$k_{85} = k_{58}$$

$$k_{83} = k_{38}$$

$$k_{86} = k_{68}$$

$$k_{87} = k_{78}$$

$$k_{88} = \frac{Et}{16a^2b^2(1-\nu^2)} \int_A \left[ a^2(1-\xi)^2 + b^2\left(\frac{1-\nu}{2}\right)(1-\eta)^2 \right] dx dy$$

Rijlik matrisinde bazı terimlerin integrasyon değerlerini yazarak terimleri kullandık.

$$\int_V \eta dV = t \int_{-a}^a \int_{-b}^b \frac{y}{b} dA = \frac{t}{b} \left[ \frac{y^2}{2} \times \int_{-a}^a \int_{-b}^b \right] = \frac{t}{b} \left( \frac{b^2}{2} - \frac{b^2}{2} \right) \times$$

$$= 0$$

$$\int_V \xi dV = 0$$

$$\int_V dV = t \int_A dA = txy \Big|_{-a}^a \Big|_{-b}^b = t(a+a)(b+b) = 4abt$$

$$\int_V \xi^2 dV = \frac{t}{a^2} \int_A x^2 dA = \frac{t}{a^2} \left[ \frac{x^3}{3} \cdot y \Big|_{-a}^a \Big|_{-b}^b \right] = \frac{t}{a^2} \left[ \frac{a^3}{3} + \frac{a^3}{3} \right] [b+b]$$

$$= \frac{t}{a^2} \left( 2 \frac{a^3}{3} \right) 2b = \frac{4}{3} tab$$

$$\int_V \eta^2 dV = \frac{4}{3} tab$$

$$\int_V \xi \eta dV = \frac{t}{ab} \int_A xy dA = \frac{t}{ab} \left[ \frac{x^2}{2} \cdot \frac{y^2}{2} \Big|_{-a}^a \Big|_{-b}^b \right] = 0$$

$$\zeta = \frac{b}{a}; \quad \frac{1}{\zeta} = \frac{a}{b} \quad \text{ve}$$

$$D_{11} = \frac{E}{1-\nu^2}$$

$$D_{12} = D_{21} = \frac{E}{1-\nu^2} \nu$$

$$D_{33} = \frac{E}{(1+\nu)2}$$

$$[D] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \rightarrow [D] = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{21} & D_{22} & 0 \\ 0 & 0 & D_{33} \end{bmatrix}$$

$$k_{11} = \frac{1}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2(4abt) + a^2\left(\frac{1-\nu}{2}\right)(4abt) \right]$$

$$= \frac{Et}{4(1-\nu^2)} \left( \frac{b}{a} \right) + \frac{Et}{4(1-\nu^2)} \left( \frac{1-\nu}{2} \right) \left( \frac{a}{b} \right)$$

$$= \frac{Et}{4(1-\nu^2)} \left( \frac{b}{a} \right) + \frac{Et}{4} \frac{1}{2(1+\nu)} \left( \frac{a}{b} \right)$$

$$= \frac{t}{4} D_{11} \cdot \frac{3}{3} + \frac{t}{4} D_{33} \cdot \frac{1}{3} = \frac{t}{4} \left( D_{11} \cdot \frac{3}{3} + D_{33} \cdot \frac{1}{3} \right)$$


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$$k_{12} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab(4ab) + \left( \frac{1-\nu}{2} \right) ab(4ab) \right]$$

$$= \frac{t}{4} \frac{E}{(1-\nu^2)} \nu + \frac{t}{4} \frac{E}{(1-\nu^2)} \left( \frac{1-\nu}{2} \right)$$

$$= \frac{t}{4} \frac{E}{(1-\nu^2)} \nu + \frac{t}{4} \frac{E}{(1+\nu)2}$$

$$= \frac{t}{4} (D_{12} + D_{33})$$


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$$k_{13} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( 4ab - \frac{4}{3} ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( \frac{8}{3} ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( \frac{16}{3} ab \right) \right]$$

$$= \frac{t}{3} \frac{E}{(1-\nu^2)} \frac{b}{2a} - \frac{tE}{2(1+\nu)} \frac{1}{3} \frac{a}{b}$$

$$= \frac{t}{3} \left( D_{11} \cdot \frac{3}{2} - D_{33} \cdot \frac{1}{3} \right)$$

$$\begin{aligned}
 k_{14} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) + ab \left( \frac{1-\nu}{2} \right) (4ab) \right] \\
 &= -\frac{t}{4} \frac{E}{(1-\nu^2)} \nu + \frac{t}{4} \frac{E}{(1+\nu)^2} \\
 &= \frac{t}{4} (-D_{12} + D_{33})
 \end{aligned}$$


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$$\begin{aligned}
 k_{15} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( 4ab + \frac{4}{3} ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab - \frac{4}{3} ab \right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( \frac{16}{3} ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( \frac{8}{3} ab \right) \right] \\
 &= -\frac{t}{3} \frac{b}{a} \frac{E}{(1-\nu^2)} + \frac{t}{3} \frac{1}{2} \frac{a}{b} \frac{E}{(1+\nu)^2} \\
 &= \frac{t}{3} \left( -D_{11} \cdot 3 + D_{33} \cdot \frac{1}{2} \right)
 \end{aligned}$$


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$$\begin{aligned}
 k_{16} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab(4ab) - ab \left( \frac{1-\nu}{2} \right) (4ab) \right] \\
 &= \frac{t}{4} \frac{E}{(1-\nu^2)} \nu - \frac{t}{4} \frac{E}{(1+\nu)^2} \\
 &= \frac{t}{4} (D_{12} - D_{33})
 \end{aligned}$$


---

$$\begin{aligned}
 k_{17} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( 4ab - \frac{4}{3} ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab - \frac{4}{3} ab \right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( \frac{8}{3} ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( \frac{8}{3} ab \right) \right] \\
 &= -\frac{t}{6} \frac{b}{a} \frac{E}{(1-\nu^2)} - \frac{t}{6} \frac{a}{b} \frac{E}{(1+\nu)^2} \\
 &= -\frac{t}{6} \left( D_{11} \cdot 3 + D_{33} \cdot \frac{1}{3} \right)
 \end{aligned}$$

$$\begin{aligned}
 k_{18} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) - ab\left(\frac{1-\nu}{2}\right)(4ab) \right] \\
 &= -\frac{t}{4} \frac{E}{(1-\nu^2)} \nu - \frac{t}{4} \frac{E}{(1+\nu)2} \\
 &= -\frac{t}{4} (D_{12} + D_{33})
 \end{aligned}$$


---

$$\begin{aligned}
 k_{22} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2\left(4ab + \frac{4}{3}ab\right) + b^2\left(\frac{1-\nu}{2}\right)\left(4ab + \frac{4}{3}ab\right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2\left(\frac{16}{3}ab\right) + b^2\left(\frac{1-\nu}{2}\right)\left(\frac{16}{3}ab\right) \right] \\
 &= \frac{t}{3} \frac{a}{b} \frac{E}{(1-\nu^2)} + \frac{t}{3} \frac{b}{a} \frac{E}{(1+\nu)2} \\
 &= \frac{t}{3} \left( D_{22} \cdot \frac{1}{3} + D_{33} \cdot 3 \right)
 \end{aligned}$$


---

$$\begin{aligned}
 k_{24} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -a^2\left(4ab + \frac{4}{3}ab\right) + b^2\left(\frac{1-\nu}{2}\right)\left(4ab - \frac{4}{3}ab\right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -a^2\left(\frac{16}{3}ab\right) + b^2\left(\frac{1-\nu}{2}\right)\left(\frac{8}{3}ab\right) \right] \\
 &= -\frac{t}{3} \frac{a}{b} D_{22} + \frac{t}{3} \frac{b}{2a} \frac{E}{(1+\nu)2} \\
 &= \frac{t}{3} \left( -D_{22} \cdot \frac{1}{3} + D_{33} \cdot \frac{3}{2} \right)
 \end{aligned}$$


---

$$\begin{aligned}
 k_{23} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab(4ab) - ab\left(\frac{1-\nu}{2}\right)(4ab) \right] \\
 &= \frac{t}{4} (D_{12} - D_{33})
 \end{aligned}$$



$$\begin{aligned}
 k_{25} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) + ab\left(\frac{1-\nu}{2}\right)(4ab) \right] \\
 &= -\frac{t}{4} \frac{E}{(1-\nu^2)} \nu + \frac{t}{4} \frac{E}{(1+\nu)^2} \\
 &= \frac{t}{4} (-D_{21} + D_{33})
 \end{aligned}$$


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$$\begin{aligned}
 k_{26} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2\left(4ab - \frac{4}{3}ab\right) - b^2\left(\frac{1-\nu}{2}\right)\left(4ab + \frac{4}{3}ab\right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2\left(\frac{8}{3}ab\right) - b^2\left(\frac{1-\nu}{2}\right)\left(\frac{16}{3}ab\right) \right] \\
 &= \frac{t}{6} \frac{a}{b} D_{22} - \frac{t}{3} \frac{b}{a} D_{33} \\
 &= \frac{t}{3} \left( D_{22} \cdot \frac{1}{23} - D_{33} \cdot 3 \right)
 \end{aligned}$$


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$$\begin{aligned}
 k_{27} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) - ab\left(\frac{1-\nu}{2}\right)(4ab) \right] \\
 &= -\frac{t}{4} D_{21} - \frac{t}{4} D_{33} \\
 &= -\frac{t}{4} (D_{21} + D_{33})
 \end{aligned}$$


---

$$\begin{aligned}
 k_{28} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -a^2\left(4ab - \frac{4}{3}ab\right) - b^2\left(\frac{1-\nu}{2}\right)\left(4ab - \frac{4}{3}ab\right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -a^2\left(\frac{8}{3}ab\right) - b^2\left(\frac{1-\nu}{2}\right)\left(\frac{8}{3}ab\right) \right] \\
 &= -\left( \frac{t}{6} \frac{a}{b} D_{22} + \frac{t}{6} D_{33} \frac{b}{a} \right) \\
 &= -\frac{t}{6} \left( D_{22} \cdot \frac{1}{3} + D_{33} \cdot 3 \right)
 \end{aligned}$$

$$\begin{aligned}
 k_{33} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( 4ab + \frac{4}{3}ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3}ab \right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( \frac{16}{3}ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( \frac{16}{3}ab \right) \right] \\
 &= \frac{t}{3} \frac{b}{a} \frac{E}{(1-\nu^2)} + \frac{t}{3} \frac{a}{b} \frac{E}{(1+\nu)^2} \\
 &= \frac{t}{3} \left( D_{11} \cdot 3 + D_{33} \cdot \frac{1}{3} \right)
 \end{aligned}$$


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$$\begin{aligned}
 k_{34} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) - ab \left( \frac{1-\nu}{2} \right) (4ab) \right] \\
 &= -\frac{t}{4} \frac{E}{(1-\nu^2)} \nu - \frac{t}{4} \frac{E}{(1+\nu)^2} \\
 &= -\frac{t}{4} (D_{12} + D_{33})
 \end{aligned}$$


---

$$\begin{aligned}
 k_{35} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( 4ab - \frac{4}{3}ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab - \frac{4}{3}ab \right) \right] \\
 &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( \frac{8}{3}ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( \frac{8}{3}ab \right) \right] \\
 &= -\frac{t}{6} \frac{b}{a} \frac{E}{(1-\nu^2)} - \frac{t}{6} \frac{a}{b} \frac{E}{(1+\nu)^2} \\
 &= -\frac{t}{6} \left( D_{11} \cdot 3 + D_{33} \cdot \frac{1}{3} \right)
 \end{aligned}$$


---

$$\begin{aligned}
 k_{36} &= \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab(4ab) + ab \left( \frac{1-\nu}{2} \right) (4ab) \right] \\
 &= \frac{t}{4} \frac{E}{(1-\nu^2)} \nu + \frac{t}{4} \frac{E}{(1+\nu)^2} \\
 &= \frac{t}{4} (D_{12} + D_{33})
 \end{aligned}$$

$$k_{37} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -b^2 \left( 4ab + \frac{4}{3} ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab - \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{3} \left( -D_{11} \cdot 3 + D_{33} \cdot \frac{1}{2 \cdot 3} \right)$$


---

$$k_{38} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab (4ab) + ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= \frac{t}{4} \left( -D_{12} + D_{33} \right)$$


---

$$k_{44} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2 \left( 4ab + \frac{4}{3} ab \right) + b^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{3} \left( D_{22} \frac{1}{3} + D_{33} \cdot 3 \right)$$


---

$$k_{45} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab (4ab) + ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= \frac{t}{4} \left( D_{21} + D_{33} \right)$$


---

$$k_{46} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -a^2 \left( 4ab - \frac{4}{3} ab \right) - b^2 \left( \frac{1-\nu}{2} \right) \left( 4ab - \frac{4}{3} ab \right) \right]$$

$$= -\frac{t}{6} \left( D_{22} \frac{1}{3} + D_{33} \cdot 3 \right)$$


---

$$k_{47} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab (4ab) - ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= \frac{t}{4} \left( D_{21} - D_{33} \right)$$


---

$$k_{48} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2 \left( 4ab - \frac{4}{3} ab \right) - b^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{3} \left( D_{22} \frac{1}{2 \cdot 3} - D_{33} \cdot 3 \right)$$

$$k_{55} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( 4ab + \frac{4}{3}ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3}ab \right) \right]$$

$$= \frac{t}{3} \left( D_{11} \cdot 3 + D_{33} \cdot \frac{1}{3} \right)$$


---

$$k_{56} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) - ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= -\frac{t}{4} (D_{12} + D_{33})$$


---

$$k_{57} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( 4ab - \frac{4}{3}ab \right) - a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3}ab \right) \right]$$

$$= \frac{t}{3} \left( D_{11} \frac{3}{2} - D_{33} \frac{1}{3} \right)$$


---

$$k_{58} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab(4ab) - ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= \frac{t}{4} (D_{12} - D_{33})$$


---

$$k_{66} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2 \left( 4ab + \frac{4}{3}ab \right) + b^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3}ab \right) \right]$$

$$= \frac{t}{3} \left( D_{22} \frac{1}{3} + D_{33} \cdot 3 \right)$$


---

$$k_{67} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -\nu ab(4ab) + ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= \frac{t}{4} (-D_{21} + D_{33})$$

$$k_{68} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ -a^2 \left( 4ab + \frac{4}{3} ab \right) + b^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{3} \left( -D_{22} \frac{1}{3} + D_{33} \frac{3}{2} \right)$$


---

$$k_{77} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ b^2 \left( 4ab + \frac{4}{3} ab \right) + a^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{3} \left( D_{11} \cdot 3 + D_{33} \cdot \frac{1}{3} \right)$$


---

$$k_{78} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ \nu ab(4ab) + ab \left( \frac{1-\nu}{2} \right) (4ab) \right]$$

$$= \frac{t}{4} (D_{12} + D_{33})$$

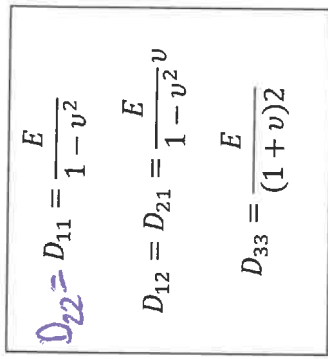

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$$k_{88} = \frac{t}{16a^2b^2} \frac{E}{(1-\nu^2)} \left[ a^2 \left( 4ab + \frac{4}{3} ab \right) + b^2 \left( \frac{1-\nu}{2} \right) \left( 4ab + \frac{4}{3} ab \right) \right]$$

$$= \frac{t}{3} \left( D_{22} \frac{1}{3} + D_{33} \cdot 3 \right)$$

Yukarıda verilen sonuçlar aşağıda tabloda özetlemiştir.  
veya bir başka açıdan rijitlik matrisi yeniden oluşturulursa;

$$\left. \begin{aligned} k_{11} &= \frac{Et}{4(1-\nu^2)} \left( 3 + \frac{(1-\nu)}{23} \right) \\ k_{12} &= \frac{Et}{4(1-\nu^2)} \left( \frac{\nu+1}{2} \right) \end{aligned} \right\} \text{şeklinde elde edilecektir}$$



$$[D] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 & 0 \\ \nu & 1 & 0 & 0 \\ 0 & 0 & 1-\nu & 0 \\ 0 & 0 & 0 & 1-\nu \end{bmatrix} = \begin{bmatrix} D_{11} & D_{12} & 0 & 0 \\ D_{21} & D_{22} & 0 & 0 \\ 0 & 0 & D_{33} & 0 \\ 0 & 0 & 0 & D_{33} \end{bmatrix}$$

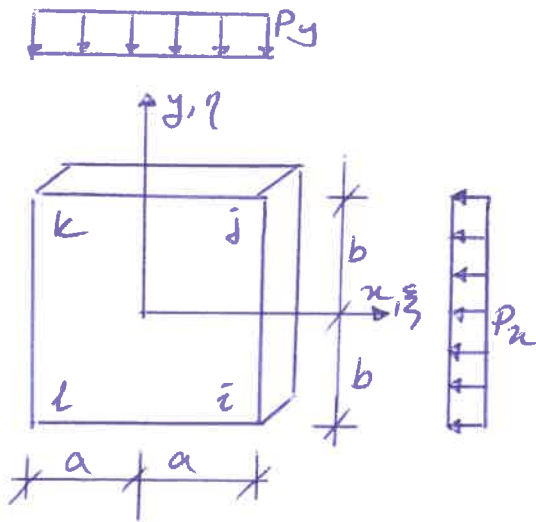
$$z = b/a; \quad 1/z = a/b$$

$$[k_e] = \begin{bmatrix} \frac{t}{4} \left( D_{11} \cdot \beta + \frac{D_{33}}{\beta} \right) & \frac{t}{4} (D_{12} + D_{33}) & \frac{t}{3} \left( \frac{D_{22}}{\beta} + D_{33} \cdot \beta \right) & 0 & 0 & 0 \\ \frac{t}{4} \left( D_{11} \cdot \frac{\beta}{2} - \frac{D_{33}}{\beta} \right) & \frac{t}{4} (D_{12} - D_{33}) & \frac{t}{3} \left( D_{11} \cdot \beta + \frac{D_{33}}{\beta} \right) & \frac{t}{3} \left( \frac{D_{22}}{\beta} + D_{33} \cdot \beta \right) & \frac{t}{4} (D_{11} \cdot \beta + \frac{D_{33}}{\beta}) & \frac{t}{3} \left( \frac{D_{22}}{\beta} + D_{33} \cdot \beta \right) \\ \frac{t}{4} (-D_{12} + D_{33}) & \frac{t}{3} \left( -\frac{D_{22}}{\beta} + D_{33} \cdot \frac{\beta}{2} \right) & \frac{t}{4} (-D_{12} - D_{33}) & \frac{t}{4} (D_{21} + D_{33}) & \frac{t}{3} \left( D_{11} \cdot \beta + \frac{D_{33}}{\beta} \right) & \frac{t}{4} (-D_{12} - D_{33}) \\ \frac{t}{3} \left( -D_{11} \cdot \beta + \frac{D_{33}}{2\beta} \right) & \frac{t}{4} (-D_{21} + D_{33}) & \frac{t}{6} \left( -D_{11} \cdot \beta - \frac{D_{33}}{\beta} \right) & \frac{t}{4} \left( -\frac{D_{22}}{\beta} - D_{33} \cdot \beta \right) & \frac{t}{3} \left( D_{11} \cdot \frac{\beta}{2} - \frac{D_{33}}{\beta} \right) & \frac{t}{4} (-D_{21} - D_{33}) \\ \frac{t}{4} (D_{12} - D_{33}) & \frac{t}{3} \left( \frac{D_{22}}{2\beta} - D_{33} \cdot \beta \right) & \frac{t}{4} (D_{12} + D_{33}) & \frac{t}{6} \left( -\frac{D_{22}}{\beta} - D_{33} \cdot \beta \right) & \frac{t}{4} (D_{21} - D_{33}) & \frac{t}{3} \left( \frac{D_{22}}{\beta} + D_{33} \cdot \beta \right) \\ \frac{t}{6} \left( -D_{11} \cdot \beta - \frac{D_{33}}{\beta} \right) & \frac{t}{4} (-D_{21} - D_{33}) & \frac{t}{3} \left( -D_{11} \cdot \beta + \frac{D_{33}}{2\beta} \right) & \frac{t}{4} (D_{21} - D_{33}) & \frac{t}{3} \left( D_{11} \cdot \beta + \frac{D_{33}}{\beta} \right) & \frac{t}{4} (-D_{11} \cdot \beta - \frac{D_{33}}{\beta}) \end{bmatrix}$$

*simetrik*

### 10.3.1) Yük Matrisi

#### a) Düzgen Yayılı Yük Durumu (Edge Load)



$$\{q\}_s = - \int_s [N]^T \{P\} ds$$

$$\{q\}_i = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_i dx dy$$

$$\{q\}_j = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_j dx dy$$

$$\{q\}_k = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_k dx dy$$

$$\{q\}_l = - \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} \int N_l dx dy$$

$$\{q\}_s = - \int \begin{bmatrix} N_i & 0 \\ 0 & N_i \\ N_j & 0 \\ 0 & N_j \\ N_k & 0 \\ 0 & N_k \\ N_l & 0 \\ 0 & N_l \end{bmatrix} \begin{bmatrix} -P_x \cdot b \\ 0 \\ -P_x \cdot b \\ -P_y \cdot a \\ 0 \\ -P_y \cdot a \\ 0 \\ 0 \end{bmatrix} dx dy$$

$$\{q\}_i = - \int_s N_i \cdot P_x dy = - P_x \int_s \frac{1}{4} (1 + \xi + \eta + \xi\eta) dy$$

$$\{q\}_i = - P_x \cdot b$$

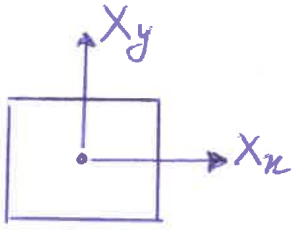
$$\{q\}_k = - P_x \cdot b$$

$$\{q\}_j = \{q\}_l = - P_y \cdot b$$

$$\left\| \begin{aligned} \int_{-b}^b dy &= 2b ; \int_{-b}^b \xi dy = 2b ; \int_{-b}^b \eta dy = 0 \\ \int_{-b}^b \xi \eta dy &= 0 \end{aligned} \right.$$



## b) Kütlesel yük (Body Forces)



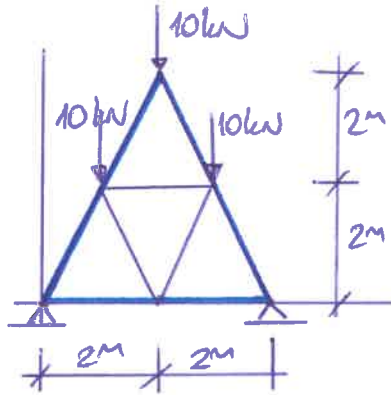
$$\{x\} = \begin{Bmatrix} x_n(x,y) \\ x_y(x,y) \end{Bmatrix}$$

$$\{q\}_x = - \int_V [N]^T \{x\} dV$$

## c) Yayılı yük (Consistent mass)

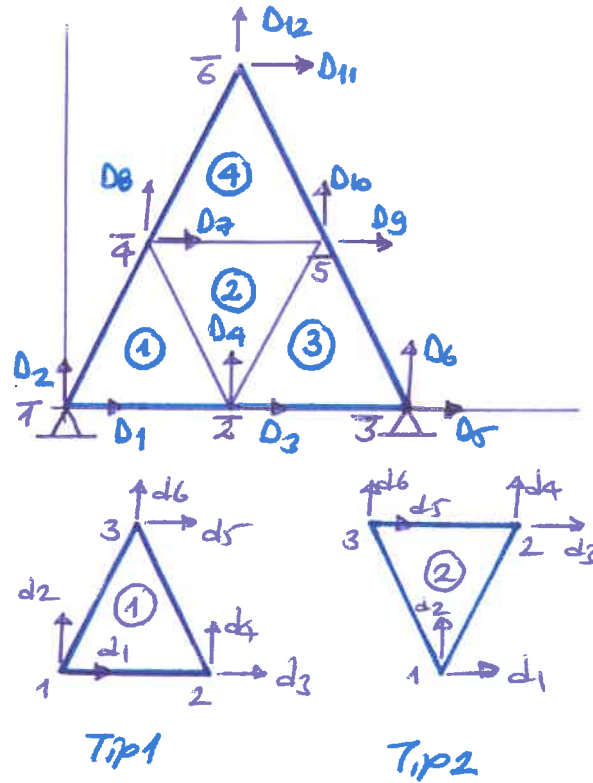
$$[M] = \rho \int [N][N] dV$$

ÖRNEK 1) Şekilde boyutları ve tipi verilen üsgen levhanın yer-  
değiştirilme ve gerilme hesaplarını yapınız. ( $\nu=0.25$ )

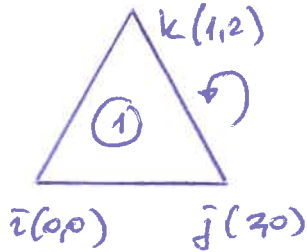


Üsgen noktası koordinatları

| Nokta No | x(m) | y(m) |
|----------|------|------|
| 1        | 0    | 0    |
| 2        | 2    | 0    |
| 3        | 4    | 0    |
| 4        | 1    | 2    |
| 5        | 3    | 2    |
| 6        | 2    | 4    |



1. eleman



$$\begin{cases} a_i' = x_j y_k - x_k y_j = 2 \times 2 - 1 \times 0 = 4 \\ b_i' = y_j - y_k = 0 - 2 = -2 \\ c_i' = x_k - x_j = 1 - 2 = -1 \end{cases}$$

$$\begin{cases} a_j' = x_k y_i - y_k x_i = 1 \times 0 - 0 = 0 \\ b_j' = y_k - y_i = 2 - 0 = 2 \\ c_j' = x_i - x_k = 0 - 1 = -1 \end{cases}$$

$$\begin{cases} a_k = x_i y_j - x_j y_i = 0 - 2 \times 0 = 0 \\ b_k = y_i - y_j = 0 - 0 = 0 \\ c_k = x_j - x_i = 2 - 0 = 2 \end{cases}$$

$$b_i' + b_j' + b_k = 0$$

$$c_i' + c_j' + c_k = 0$$

$$\text{Alan } \Delta = \frac{2 \times 2}{2} = 2 \text{ m}^2$$

$$\{\varepsilon\} = \frac{1}{2 \times 2} \begin{bmatrix} -2 & 0 & 2 & 0 & 0 & 0 \\ 0 & -1 & 0 & -1 & 0 & 2 \\ -1 & -2 & -1 & 2 & 2 & 0 \end{bmatrix} \{d\}$$

$$\{\sigma\} = [D] \{\varepsilon\}$$

$$\{\sigma\} = \frac{E}{4(1-0,2^2)} \begin{bmatrix} 1 & 0,2 & 0 \\ 0,2 & 1 & 0 \\ 0 & 0 & \frac{1-0,2}{2} \end{bmatrix} \begin{bmatrix} -2 & 0 & 2 & 0 & 0 & 0 \\ 0 & -1 & 0 & -1 & 0 & 2 \\ -1 & -2 & -1 & 2 & 2 & 0 \end{bmatrix} \{d\}$$

$$\{\sigma\} = \frac{E}{4(1-0,2^2)} \begin{bmatrix} -2 & -0,2 & 2 & -0,2 & 0 & 0,4 \\ -0,4 & -1 & 0,4 & -1 & 0 & 2 \\ -0,4 & -0,8 & -0,4 & 0,8 & 0,8 & 0 \end{bmatrix} \{d\}$$

1. elementin rijitlik matrici

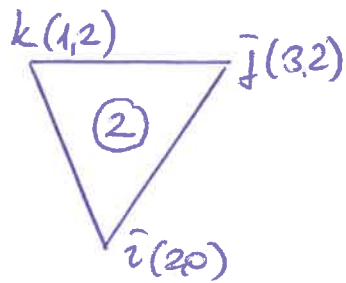
$$[k_e] = \iint [B]^T [D] [B] t dx dy$$

$$[k_{e1}] = \frac{Et}{16(1-0,2^2)} \iint \begin{bmatrix} -2 & 0 & -1 \\ 0 & -1 & -2 \\ 2 & 0 & -1 \\ 0 & -1 & 2 \\ 0 & 0 & 2 \\ 0 & 2 & 0 \end{bmatrix} \begin{bmatrix} -2 & -0,2 & 2 & -0,2 & 0 & 0,4 \\ -0,4 & -1 & 0,4 & -1 & 0 & 2 \\ -0,4 & -0,8 & -0,4 & 0,8 & 0,8 & 0 \end{bmatrix} dx dy$$

$\iint dx dy = 2$

$$[k_{e1}] = \frac{Et}{8(1-0,2^2)} \begin{bmatrix} 4,4 & 1,2 & -3,6 & -0,4 & -0,8 & -0,8 \\ 1,2 & 2,6 & 0,4 & -0,6 & -1,6 & -2,0 \\ -3,6 & 0,4 & 4,4 & -1,2 & -0,8 & 0,8 \\ -0,4 & -0,6 & -1,2 & 2,6 & 1,6 & -2,0 \\ -0,8 & -1,6 & -0,8 & 1,6 & 1,6 & 0 \\ -0,8 & -2,0 & 0,8 & -2,0 & 0 & 4,0 \end{bmatrix}$$

2. element



$$\begin{cases} a_i = x_j y_k - x_k y_j = 3 \times 2 - 1 \times 2 = 4 \\ b_i = y_j - y_k = 2 - 2 = 0 \\ c_i = x_k - x_j = 1 - 3 = -2 \end{cases}$$

$$b_i + b_j + b_k = 0$$

$$c_i + c_j + c_k = 0$$

$$\text{Area } \Delta = \frac{2 \times 2}{2} = 2 \text{ m}^2$$

$$\begin{cases} a_j = x_k y_i - x_i y_k = 1 \times 0 - 2 \times 2 = -4 \\ b_j = y_k - y_i = 0 - 2 = -2 \\ c_j = x_i - x_k = 2 - 1 = 1 \end{cases}$$

$$\begin{cases} a_k = x_i y_j - x_j y_i = 2 \times 2 - 3 \times 0 = 4 \\ b_k = y_i - y_j = 0 - 2 = -2 \\ c_k = x_j - x_i = 3 - 2 = 1 \end{cases}$$

$$\{\epsilon\} = \frac{1}{2 \times 2} \begin{bmatrix} 0 & 0 & 2 & 0 & -2 & 0 \\ 0 & -2 & 0 & 1 & 0 & 1 \\ -2 & 0 & 1 & 2 & 1 & -2 \end{bmatrix} \{\delta\}$$

$$\{\sigma\} = \frac{E}{4(1-\nu^2)} \begin{bmatrix} 1 & \nu & 2 & 0 \\ \nu & 1 & 0 & 0 \\ 0 & 0 & \frac{1-\nu}{2} & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 & 2 & 0 & -2 & 0 \\ 0 & -2 & 0 & 1 & 0 & 1 \\ -2 & 0 & 1 & 2 & 1 & -2 \end{bmatrix} \{\delta\}$$

$$\{\sigma\} = \frac{E}{4(1-\nu^2)} \begin{bmatrix} 0 & -0,4 & 2 & 0,2 & -2 & 0,2 \\ 0 & -2 & 0,4 & 1 & -0,4 & 1 \\ -0,8 & 0 & 0,4 & 0,8 & 0,4 & -0,8 \end{bmatrix} \{\delta\}$$

$$[k]_2 = \frac{Et}{16(1-\nu^2)} \iint \begin{bmatrix} 0 & 0 & -2 \\ 0 & -2 & 0 \\ 2 & 0 & 1 \\ 0 & 1 & 2 \\ -2 & 0 & 1 \\ 0 & 1 & -2 \end{bmatrix} \begin{bmatrix} 0 & -0,4 & 2 & 0,2 & -2 & 0,2 \\ 0 & -2 & 0,4 & 1 & -0,4 & 1 \\ -0,8 & 0 & 0,4 & 0,8 & 0,4 & -0,8 \end{bmatrix} dx dy$$

$$[k_{e2}] = \frac{Et}{8(1-0,2^2)}$$

$$\begin{bmatrix} 1,6 & 0 & -0,8 & -1,6 & -0,8 & 1,6 \\ 0 & 4 & -0,8 & -2 & 0,8 & -2 \\ -0,8 & -0,8 & 4,4 & 1,2 & -3,6 & -0,4 \\ -1,6 & -2 & 1,2 & 2,6 & 0,4 & -0,6 \\ -0,8 & 0,8 & -3,6 & 0,4 & 4,4 & -1,2 \\ 1,6 & -2 & -0,4 & -0,6 & -1,2 & 2,6 \end{bmatrix}$$

Sistem rijitlik matrisi

a) Gevreme matrisleri ile;

$$\begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{bmatrix}_1 = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \\ D_8 \end{bmatrix}$$

$$\begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{bmatrix}_2 = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \\ D_8 \end{bmatrix}$$

$$\begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{bmatrix}_3 = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \\ D_8 \end{bmatrix}$$

$$\begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{bmatrix} = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \\ D_8 \end{bmatrix}$$

Buradan;

$$[K_{e1}] = [C_{e1}]^T [K_e] [C_{e1}]$$

$$[K_{e2}] = [C_{e2}]^T [K_e] [C_{e2}]$$

$$[K_{e3}] = [C_{e3}]^T [K_e] [C_{e3}]$$

$$[K_{e4}] = [C_{e4}]^T [K_e] [C_{e4}]$$

$$[K_e] = [K_{e1}] + [K_{e2}] + [K_{e3}] + [K_{e4}]$$

$$[K] = \frac{Et}{8(1-\nu^2)} \begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{14} & K_{15} & K_{16} & K_{17} & K_{18} & K_{19} & K_{20} & K_{21} \\ K_{21} & K_{22} & K_{23} & K_{24} & K_{25} & K_{26} & K_{27} & K_{28} & K_{29} & K_{210} & K_{211} \\ K_{31} & K_{32} & K_{33} & K_{34} & K_{35} & K_{36} & K_{37} & K_{38} & K_{39} & K_{310} & K_{311} \\ K_{41} & K_{42} & K_{43} & K_{44} & K_{45} & K_{46} & K_{47} & K_{48} & K_{49} & K_{410} & K_{411} \\ K_{51} & K_{52} & K_{53} & K_{54} & K_{55} & K_{56} & K_{57} & K_{58} & K_{59} & K_{510} & K_{511} \\ K_{61} & K_{62} & K_{63} & K_{64} & K_{65} & K_{66} & K_{67} & K_{68} & K_{69} & K_{610} & K_{611} \\ K_{71} & K_{72} & K_{73} & K_{74} & K_{75} & K_{76} & K_{77} & K_{78} & K_{79} & K_{710} & K_{711} \\ K_{81} & K_{82} & K_{83} & K_{84} & K_{85} & K_{86} & K_{87} & K_{88} & K_{89} & K_{810} & K_{811} \\ - & - & - & - & - & - & - & - & - & - & - \\ - & - & - & - & - & - & - & - & - & - & - \end{bmatrix}$$

[illegible]

$$K_{e2} = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1.6 & 0 & 0 & 0 & -0.8 & 1.6 & -0.8 & -1.6 & 0 \\ 0 & 0 & 0 & 4 & 0 & 0 & 0.8 & -2 & -0.8 & -2 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & -0.8 & 0.8 & 0 & 0 & 4.4 & -1.2 & -3.6 & 0.4 & 0 \\ 0 & 0 & 1.6 & -2 & 0 & 0 & -1.2 & 2.6 & -0.4 & -0.6 & 0 \\ 0 & 0 & -0.8 & -0.8 & 0 & 0 & -3.6 & -0.4 & 4.4 & 1.2 & 0 \\ 0 & 0 & -1.6 & -2 & 0 & 0 & 0.4 & -0.6 & 1.2 & 2.6 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$E_t$

$8(1-q_2^2)$

[illegible]

|   |   |   |   |   |   |      |      |      |      |      |      |   |
|---|---|---|---|---|---|------|------|------|------|------|------|---|
| 0 | 0 | 0 | 0 | 0 | 0 | 0    | 0    | 0    | 0    | 0    | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0    | 0    | 0    | 0    | 0    | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0    | 0    | 0    | 0    | 0    | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0    | 0    | 0    | 0    | 0    | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0    | 0    | 0    | 0    | 0    | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0    | 0    | 0    | 0    | 0    | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 4.4  | 1.2  | -3.6 | -0.4 | -0.8 | -0.8 | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | 1.2  | 2.6  | 0.4  | -0.6 | -1.6 | -2   | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | -3.6 | 0.4  | 4.4  | -1.2 | -0.8 | 0.8  | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | -0.4 | -0.6 | -1.2 | 2.6  | 1.6  | -2   | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | -0.8 | -1.6 | -0.8 | 1.6  | 1.6  | 0    | 0 |
| 0 | 0 | 0 | 0 | 0 | 0 | -0.8 | -2   | 0.8  | -2   | 0    | 4    | 0 |

$$K_{e4} = \frac{Et}{8(1-0.2^2)}$$



$$[K] = \frac{Et}{8(1-\nu^2)} \begin{bmatrix} 4.4 & 1.2 & -3.6 & -0.4 & 0.0 & 0.0 & -0.8 & -0.8 & 0.0 & 0.0 & 0.0 & 0.0 \\ 1.2 & 2.6 & 0.4 & -0.6 & 0.0 & 0.0 & -1.6 & -2.0 & 0.0 & 0.0 & 0.0 & 0.0 \\ -3.6 & 0.4 & 10.4 & 0.0 & -3.6 & -0.4 & -1.6 & 2.4 & -1.6 & -2.4 & 0.0 & 0.0 \\ -0.4 & -0.6 & 0.0 & 9.2 & 0.4 & -0.6 & 2.4 & -4.0 & -2.4 & -4.0 & 0.0 & 0.0 \\ 0.0 & 0.0 & -3.6 & 0.4 & 4.4 & -1.2 & 0.0 & 0.0 & -0.8 & 0.8 & 0.0 & 0.0 \\ 0.0 & 0.0 & -0.4 & -0.6 & -1.2 & 2.6 & 0.0 & 0.0 & 1.6 & -2.0 & 0.0 & 0.0 \\ -0.8 & -1.6 & -1.6 & 2.4 & 0.0 & 0.0 & 10.4 & 0.0 & -7.2 & 0.0 & -0.8 & -0.8 \\ -0.8 & -2.0 & 2.4 & -4.0 & 0.0 & 0.0 & 0.0 & 9.2 & 0.0 & -1.2 & -1.6 & -2.0 \\ 0.0 & 0.0 & -1.6 & -2.4 & -0.8 & 1.6 & -7.2 & 0.0 & 10.4 & 0.0 & -0.8 & 0.8 \\ 0.0 & 0.0 & -2.4 & -4.0 & 0.8 & -2.0 & 0.0 & -1.2 & 0.0 & 9.2 & 1.6 & -2.0 \\ 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.8 & -1.6 & -0.8 & 1.6 & 1.6 & 0.0 \\ 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.8 & -2.0 & 0.8 & -2.0 & 0.0 & 4.0 \end{bmatrix}$$

b) Biriktirme Yöntemi ile;

Bu yöntemde doğum noktalarında birleşen elemanların her doğum noktasındaki ortak yer değiştirmeleri toplama örneğinin;

$$K_{33} = k_{33}^1 + k_{11}^2 + k_{11}^1 \quad (\text{Bu doğum noktasında } \textcircled{1}, \textcircled{2} \text{ ve } \textcircled{3})$$

elemanları birleşmektedir (1 nolu elemanın 2. ucu, 2 nolu elemanın 1. ucu ve 3 nolu elemanın 1. ucu)

$$\begin{array}{llll} 1 & \text{nolu} & \text{elemanın} & 2. \text{ ucu} \rightarrow k_{33}^1 \\ 2 & & & 1. \text{ ucu} \rightarrow k_{11}^2 \\ 3 & & & 1. \text{ ucu} \rightarrow k_{11}^1 \end{array}$$

$$K_{9,10} = k_{34}^2 + k_{56}^1 + k_{34}^1 \quad (\text{Bu doğum noktasında } \textcircled{2}, \textcircled{3} \text{ ve } \textcircled{4})$$

elemanları birleşmektedir (2 nolu elemanın 2. ucu, 3 nolu elemanın 3. ucu ve 4 nolu elemanın 2. ucu)

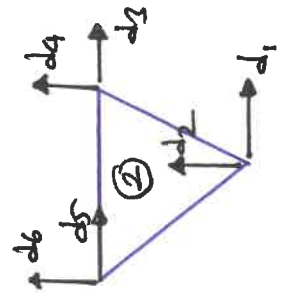
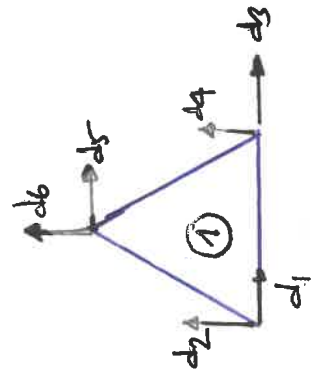
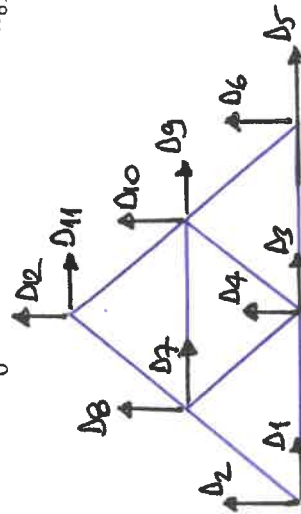
$$\begin{array}{llll} 2 & \text{nolu} & \text{elemanın} & 2. \text{ ucu} \rightarrow k_{34}^2 \\ 3 & & & 3. \text{ ucu} \rightarrow k_{56}^1 \\ 4 & & & 2. \text{ ucu} \rightarrow k_{34}^1 \end{array}$$

$$[K] = \frac{Et}{8(1-0,22^2)}$$

| $k_{11}^1$  | $k_{12}^1$  | $k_{13}^1$  | $k_{14}^1$  | $k_{15}^1$  | $k_{16}^1$  | $k_{17}^1$  | $k_{18}^1$  | $k_{19}^1$  | $k_{110}^1$  | $k_{111}^1$  | $k_{112}^1$  | $k_{113}^1$  | $k_{114}^1$  | $k_{115}^1$  | $k_{116}^1$  | $k_{117}^1$  | $k_{118}^1$  | $k_{119}^1$  | $k_{120}^1$  |
|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|
| $k_{11}^1$  | $k_{12}^1$  | $k_{13}^1$  | $k_{14}^1$  | $k_{15}^1$  | $k_{16}^1$  | $k_{17}^1$  | $k_{18}^1$  | $k_{19}^1$  | $k_{110}^1$  | $k_{111}^1$  | $k_{112}^1$  | $k_{113}^1$  | $k_{114}^1$  | $k_{115}^1$  | $k_{116}^1$  | $k_{117}^1$  | $k_{118}^1$  | $k_{119}^1$  | $k_{120}^1$  |
| $k_{21}^1$  | $k_{22}^1$  | $k_{23}^1$  | $k_{24}^1$  | $k_{25}^1$  | $k_{26}^1$  | $k_{27}^1$  | $k_{28}^1$  | $k_{29}^1$  | $k_{210}^1$  | $k_{211}^1$  | $k_{212}^1$  | $k_{213}^1$  | $k_{214}^1$  | $k_{215}^1$  | $k_{216}^1$  | $k_{217}^1$  | $k_{218}^1$  | $k_{219}^1$  | $k_{220}^1$  |
| $k_{31}^1$  | $k_{32}^1$  | $k_{33}^1$  | $k_{34}^1$  | $k_{35}^1$  | $k_{36}^1$  | $k_{37}^1$  | $k_{38}^1$  | $k_{39}^1$  | $k_{310}^1$  | $k_{311}^1$  | $k_{312}^1$  | $k_{313}^1$  | $k_{314}^1$  | $k_{315}^1$  | $k_{316}^1$  | $k_{317}^1$  | $k_{318}^1$  | $k_{319}^1$  | $k_{320}^1$  |
| $k_{41}^1$  | $k_{42}^1$  | $k_{43}^1$  | $k_{44}^1$  | $k_{45}^1$  | $k_{46}^1$  | $k_{47}^1$  | $k_{48}^1$  | $k_{49}^1$  | $k_{410}^1$  | $k_{411}^1$  | $k_{412}^1$  | $k_{413}^1$  | $k_{414}^1$  | $k_{415}^1$  | $k_{416}^1$  | $k_{417}^1$  | $k_{418}^1$  | $k_{419}^1$  | $k_{420}^1$  |
| $k_{51}^1$  | $k_{52}^1$  | $k_{53}^1$  | $k_{54}^1$  | $k_{55}^1$  | $k_{56}^1$  | $k_{57}^1$  | $k_{58}^1$  | $k_{59}^1$  | $k_{510}^1$  | $k_{511}^1$  | $k_{512}^1$  | $k_{513}^1$  | $k_{514}^1$  | $k_{515}^1$  | $k_{516}^1$  | $k_{517}^1$  | $k_{518}^1$  | $k_{519}^1$  | $k_{520}^1$  |
| $k_{61}^1$  | $k_{62}^1$  | $k_{63}^1$  | $k_{64}^1$  | $k_{65}^1$  | $k_{66}^1$  | $k_{67}^1$  | $k_{68}^1$  | $k_{69}^1$  | $k_{610}^1$  | $k_{611}^1$  | $k_{612}^1$  | $k_{613}^1$  | $k_{614}^1$  | $k_{615}^1$  | $k_{616}^1$  | $k_{617}^1$  | $k_{618}^1$  | $k_{619}^1$  | $k_{620}^1$  |
| $k_{71}^1$  | $k_{72}^1$  | $k_{73}^1$  | $k_{74}^1$  | $k_{75}^1$  | $k_{76}^1$  | $k_{77}^1$  | $k_{78}^1$  | $k_{79}^1$  | $k_{710}^1$  | $k_{711}^1$  | $k_{712}^1$  | $k_{713}^1$  | $k_{714}^1$  | $k_{715}^1$  | $k_{716}^1$  | $k_{717}^1$  | $k_{718}^1$  | $k_{719}^1$  | $k_{720}^1$  |
| $k_{81}^1$  | $k_{82}^1$  | $k_{83}^1$  | $k_{84}^1$  | $k_{85}^1$  | $k_{86}^1$  | $k_{87}^1$  | $k_{88}^1$  | $k_{89}^1$  | $k_{810}^1$  | $k_{811}^1$  | $k_{812}^1$  | $k_{813}^1$  | $k_{814}^1$  | $k_{815}^1$  | $k_{816}^1$  | $k_{817}^1$  | $k_{818}^1$  | $k_{819}^1$  | $k_{820}^1$  |
| $k_{91}^1$  | $k_{92}^1$  | $k_{93}^1$  | $k_{94}^1$  | $k_{95}^1$  | $k_{96}^1$  | $k_{97}^1$  | $k_{98}^1$  | $k_{99}^1$  | $k_{910}^1$  | $k_{911}^1$  | $k_{912}^1$  | $k_{913}^1$  | $k_{914}^1$  | $k_{915}^1$  | $k_{916}^1$  | $k_{917}^1$  | $k_{918}^1$  | $k_{919}^1$  | $k_{920}^1$  |
| $k_{101}^1$ | $k_{102}^1$ | $k_{103}^1$ | $k_{104}^1$ | $k_{105}^1$ | $k_{106}^1$ | $k_{107}^1$ | $k_{108}^1$ | $k_{109}^1$ | $k_{1010}^1$ | $k_{1011}^1$ | $k_{1012}^1$ | $k_{1013}^1$ | $k_{1014}^1$ | $k_{1015}^1$ | $k_{1016}^1$ | $k_{1017}^1$ | $k_{1018}^1$ | $k_{1019}^1$ | $k_{1020}^1$ |
| $k_{111}^1$ | $k_{112}^1$ | $k_{113}^1$ | $k_{114}^1$ | $k_{115}^1$ | $k_{116}^1$ | $k_{117}^1$ | $k_{118}^1$ | $k_{119}^1$ | $k_{1110}^1$ | $k_{1111}^1$ | $k_{1112}^1$ | $k_{1113}^1$ | $k_{1114}^1$ | $k_{1115}^1$ | $k_{1116}^1$ | $k_{1117}^1$ | $k_{1118}^1$ | $k_{1119}^1$ | $k_{1120}^1$ |
| $k_{121}^1$ | $k_{122}^1$ | $k_{123}^1$ | $k_{124}^1$ | $k_{125}^1$ | $k_{126}^1$ | $k_{127}^1$ | $k_{128}^1$ | $k_{129}^1$ | $k_{1210}^1$ | $k_{1211}^1$ | $k_{1212}^1$ | $k_{1213}^1$ | $k_{1214}^1$ | $k_{1215}^1$ | $k_{1216}^1$ | $k_{1217}^1$ | $k_{1218}^1$ | $k_{1219}^1$ | $k_{1220}^1$ |
| $k_{131}^1$ | $k_{132}^1$ | $k_{133}^1$ | $k_{134}^1$ | $k_{135}^1$ | $k_{136}^1$ | $k_{137}^1$ | $k_{138}^1$ | $k_{139}^1$ | $k_{1310}^1$ | $k_{1311}^1$ | $k_{1312}^1$ | $k_{1313}^1$ | $k_{1314}^1$ | $k_{1315}^1$ | $k_{1316}^1$ | $k_{1317}^1$ | $k_{1318}^1$ | $k_{1319}^1$ | $k_{1320}^1$ |
| $k_{141}^1$ | $k_{142}^1$ | $k_{143}^1$ | $k_{144}^1$ | $k_{145}^1$ | $k_{146}^1$ | $k_{147}^1$ | $k_{148}^1$ | $k_{149}^1$ | $k_{1410}^1$ | $k_{1411}^1$ | $k_{1412}^1$ | $k_{1413}^1$ | $k_{1414}^1$ | $k_{1415}^1$ | $k_{1416}^1$ | $k_{1417}^1$ | $k_{1418}^1$ | $k_{1419}^1$ | $k_{1420}^1$ |
| $k_{151}^1$ | $k_{152}^1$ | $k_{153}^1$ | $k_{154}^1$ | $k_{155}^1$ | $k_{156}^1$ | $k_{157}^1$ | $k_{158}^1$ | $k_{159}^1$ | $k_{1510}^1$ | $k_{1511}^1$ | $k_{1512}^1$ | $k_{1513}^1$ | $k_{1514}^1$ | $k_{1515}^1$ | $k_{1516}^1$ | $k_{1517}^1$ | $k_{1518}^1$ | $k_{1519}^1$ | $k_{1520}^1$ |
| $k_{161}^1$ | $k_{162}^1$ | $k_{163}^1$ | $k_{164}^1$ | $k_{165}^1$ | $k_{166}^1$ | $k_{167}^1$ | $k_{168}^1$ | $k_{169}^1$ | $k_{1610}^1$ | $k_{1611}^1$ | $k_{1612}^1$ | $k_{1613}^1$ | $k_{1614}^1$ | $k_{1615}^1$ | $k_{1616}^1$ | $k_{1617}^1$ | $k_{1618}^1$ | $k_{1619}^1$ | $k_{1620}^1$ |
| $k_{171}^1$ | $k_{172}^1$ | $k_{173}^1$ | $k_{174}^1$ | $k_{175}^1$ | $k_{176}^1$ | $k_{177}^1$ | $k_{178}^1$ | $k_{179}^1$ | $k_{1710}^1$ | $k_{1711}^1$ | $k_{1712}^1$ | $k_{1713}^1$ | $k_{1714}^1$ | $k_{1715}^1$ | $k_{1716}^1$ | $k_{1717}^1$ | $k_{1718}^1$ | $k_{1719}^1$ | $k_{1720}^1$ |
| $k_{181}^1$ | $k_{182}^1$ | $k_{183}^1$ | $k_{184}^1$ | $k_{185}^1$ | $k_{186}^1$ | $k_{187}^1$ | $k_{188}^1$ | $k_{189}^1$ | $k_{1810}^1$ | $k_{1811}^1$ | $k_{1812}^1$ | $k_{1813}^1$ | $k_{1814}^1$ | $k_{1815}^1$ | $k_{1816}^1$ | $k_{1817}^1$ | $k_{1818}^1$ | $k_{1819}^1$ | $k_{1820}^1$ |
| $k_{191}^1$ | $k_{192}^1$ | $k_{193}^1$ | $k_{194}^1$ | $k_{195}^1$ | $k_{196}^1$ | $k_{197}^1$ | $k_{198}^1$ | $k_{199}^1$ | $k_{1910}^1$ | $k_{1911}^1$ | $k_{1912}^1$ | $k_{1913}^1$ | $k_{1914}^1$ | $k_{1915}^1$ | $k_{1916}^1$ | $k_{1917}^1$ | $k_{1918}^1$ | $k_{1919}^1$ | $k_{1920}^1$ |
| $k_{201}^1$ | $k_{202}^1$ | $k_{203}^1$ | $k_{204}^1$ | $k_{205}^1$ | $k_{206}^1$ | $k_{207}^1$ | $k_{208}^1$ | $k_{209}^1$ | $k_{2010}^1$ | $k_{2011}^1$ | $k_{2012}^1$ | $k_{2013}^1$ | $k_{2014}^1$ | $k_{2015}^1$ | $k_{2016}^1$ | $k_{2017}^1$ | $k_{2018}^1$ | $k_{2019}^1$ | $k_{2020}^1$ |

$$k_{33} = 4,4 + 16 + 44 = 10,4 \checkmark$$

$$k_{9,10} = 1,2 + 0 - 1,2 = 0 \checkmark$$



## Yük Matrisi

Sisteme etkiyen yükler tekiri yüklerdir ve diğer noktalara etmemektedir. Bu durumda kuvvetler yer değiştikmelere karşı olarak şekilde yük matrisi oluşturulabilir.

$$\begin{Bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_7 \\ D_8 \\ D_9 \\ D_{10} \\ D_{11} \\ D_{12} \end{Bmatrix} \Rightarrow \{Q\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ -10 \\ 0 \\ -10 \\ 0 \\ -10 \end{Bmatrix}$$

## Yer Değiştikmelere Hesabı

Sistemi rijitlik matrisi  $[K]$ , yük matrisi  $\{Q\}$  ve yer değiştikme matrisi  $\{D\}$  ise ;

$$[K][D] = \{Q\} \quad \text{şeklinde yazılacaktır}$$

$$\begin{array}{c}
 \frac{Et}{8(1-0,2^2)} \\
 \begin{bmatrix}
 44 & 12 & -36 & -0,4 & 0 & 0 & -0,8 & -0,8 & 0 & 0 & 0 & 0 \\
 12 & 26 & 0,4 & -0,6 & 0 & 0 & -16 & -20 & 0 & 0 & 0 & 0 \\
 -36 & 0,4 & 104 & 0 & -36 & -0,4 & -16 & 24 & -1,6 & -24 & 0 & 0 \\
 -0,4 & -0,6 & 0 & 9,2 & 0,4 & -0,6 & 24 & -40 & -24 & -40 & 0 & 0 \\
 0 & 0 & -36 & 0,4 & 44 & -12 & 0 & 0 & -0,8 & 0,8 & 0 & 0 \\
 0 & 0 & -0,4 & -0,6 & -12 & 26 & 0 & 0 & 1,6 & -20 & 0 & 0 \\
 -0,8 & -16 & -1,6 & 24 & 0 & 0 & 104 & 0 & -32 & 0 & -0,8 & -0,8 \\
 -0,8 & -20 & 24 & -40 & 0 & 0 & 0 & 9,2 & 0 & -1,2 & -1,6 & -20 \\
 0 & 0 & -1,6 & -24 & -0,8 & 1,6 & -32 & 0 & 104 & 0 & -0,8 & 0,8 \\
 0 & 0 & -24 & -40 & 0,8 & -20 & 0 & -1,2 & 0 & 9,2 & 1,6 & -20 \\
 0 & 0 & 0 & 0 & 0 & 0 & -0,8 & -1,6 & -0,8 & 1,6 & 1,6 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & -0,8 & -20 & 0,8 & -20 & 0 & 4,0
 \end{bmatrix}
 \begin{bmatrix}
 D_1 \\
 D_2 \\
 D_3 \\
 D_4 \\
 D_5 \\
 D_6 \\
 D_7 \\
 D_8 \\
 D_9 \\
 D_{10} \\
 D_{11} \\
 D_{12}
 \end{bmatrix}
 =
 \begin{bmatrix}
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 -10 \\
 0 \\
 -10 \\
 0 \\
 -10
 \end{bmatrix}
 \end{array}$$

Sınır koşulları

Sabit mesnet koşulu  $\rightarrow D_1 = D_2 = D_5 = D_6 = 0$

Simetre koşulu  $\rightarrow D_3 = D_{11} = 0 ; D_7 = -D_9 ; D_8 = D_{10}$

Sistemin çözümleri;

$$D_3 = 0$$

$$D_4 = -5,52 \frac{8(1-0,2^2)}{Et}$$

$$D_7 = 0,37 \frac{8(1-0,2^2)}{Et}$$

$$D_8 = -6,13 \frac{8(1-0,2^2)}{Et}$$

$$D_9 = -0,37 \frac{8(1-0,2^2)}{Et}$$

$$D_{10} = -6,13 \frac{8(1-0,2^2)}{Et}$$

$$D_{11} = 0$$

$$D_{12} = -8,49 \frac{8(1-0,2^2)}{Et}$$

## Geometrisch Hesabi

$$\begin{Bmatrix} G_x \\ G_y \\ T_{xy} \end{Bmatrix}_{1.\text{element}} = \frac{8(1-\nu^2)}{Et} \frac{E}{4(1-\nu^2)} \begin{bmatrix} -2 & -0,2 & 2 & -0,2 & 0 & 0,4 \\ -0,4 & -1 & 0,4 & -1 & 0 & 2 \\ -0,4 & -0,8 & -0,4 & 0,8 & 0,8 & 0 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ 0 \\ -5,52 \\ 0,37 \\ -6,13 \end{bmatrix} \begin{matrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_7 \\ D_8 \end{matrix}$$

$$\begin{Bmatrix} G_x \\ G_y \\ T_{xy} \end{Bmatrix}_{1.\text{element}} = \frac{1}{t} \begin{bmatrix} -2,696 \\ -13,480 \\ -8,240 \end{bmatrix} \text{ kN/m}^2$$

$$\begin{Bmatrix} G_x \\ G_y \\ T_{xy} \end{Bmatrix}_{2.\text{element}} = \frac{2}{t} \begin{bmatrix} 0 & -0,4 & 2 & 0,2 & -2 & 0,2 \\ 0 & -2 & 0,4 & 1 & -0,4 & 1 \\ -0,8 & 0 & 0,4 & 0,8 & 0,4 & -0,8 \end{bmatrix} \begin{bmatrix} 0 \\ -5,52 \\ -0,37 \\ -6,13 \\ 0,37 \\ -6,13 \end{bmatrix} \begin{matrix} D_3 \\ D_4 \\ D_9 \\ D_{10} \\ D_7 \\ D_8 \end{matrix}$$

$$\begin{Bmatrix} G_x \\ G_y \\ T_{xy} \end{Bmatrix}_{2.\text{element}} = \frac{1}{t} \begin{bmatrix} -3,448 \\ -3,032 \\ 0 \end{bmatrix} \text{ kN/m}^2$$

$$\begin{Bmatrix} G_x \\ G_y \\ T_{xy} \end{Bmatrix}_{3.\text{element}} = \frac{2}{t} \begin{bmatrix} -2 & -0,2 & 2 & -0,2 & 0 & 0,4 \\ -0,4 & -1 & 0,4 & -1 & 0 & 2 \\ -0,4 & -0,8 & -0,4 & 0,8 & 0,8 & 0 \end{bmatrix} \begin{bmatrix} 0 \\ -5,52 \\ 0 \\ 0 \\ -0,37 \\ -6,13 \end{bmatrix} \begin{matrix} D_3 \\ D_4 \\ D_5 \\ D_6 \\ D_9 \\ D_{10} \end{matrix}$$

$$\begin{Bmatrix} G_x \\ G_y \\ T_{xy} \end{Bmatrix}_{3.\text{element}} = \frac{1}{t} \begin{bmatrix} \phantom{0} \\ \phantom{0} \\ \phantom{0} \end{bmatrix} \text{ kN/m}^2$$

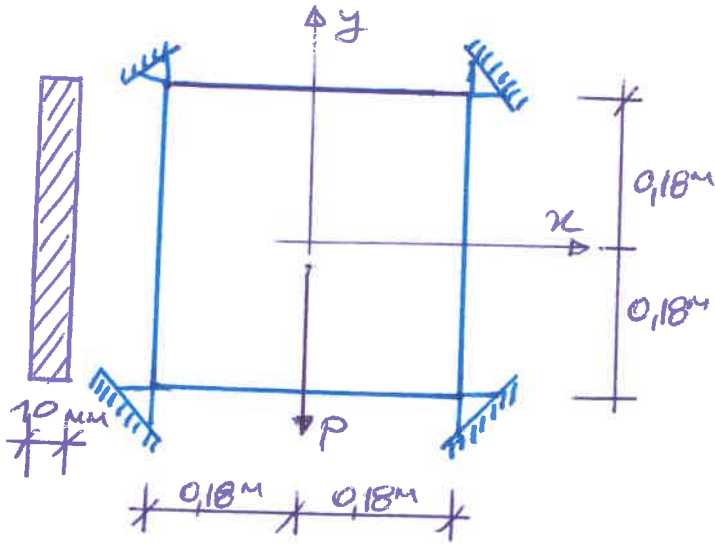
$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_{4\text{ element}} = \frac{2}{t} \begin{bmatrix} -2 & -92 & 2 & -92 & 0 & 94 \\ -94 & -1 & 94 & -1 & 0 & 2 \\ -94 & -98 & -94 & 98 & 98 & 0 \end{bmatrix} \begin{Bmatrix} 0,37 \\ -413 \\ -0,37 \\ -413 \\ 0 \\ -849 \end{Bmatrix} \begin{matrix} D_7 \\ D_8 \\ D_9 \\ D_{10} \\ D_{11} \\ D_{12} \end{matrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_{4\text{ element}} = \frac{1}{t} \begin{bmatrix} -4,848 \\ -10,032 \\ 0 \end{bmatrix} \text{ kN/m}^2$$

4

## ÖRNEK 2)

Şekilde yukarıya durumu ve boyutları verilen levhaya üçgen ve dörtgen şekil fonksiyonları ile çözerek gerilmeleri ve yer değiştirmeleri hesaplayınız.



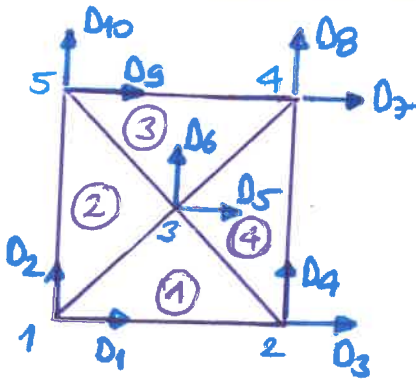
$$E = 210000 \text{ MPa}$$

$$\nu = 0.25$$

$$t = 10 \text{ mm}$$

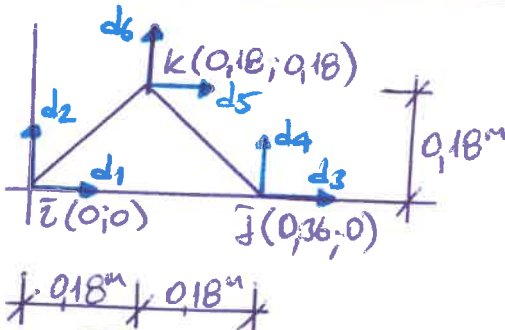
$$P = 420 \text{ kN}$$

### a) Üçgen elemanlarla çözüm



| Koordinatlar | X(m) | Y(m) |
|--------------|------|------|
| 1            | 0    | 0    |
| 2            | 0,36 | 0    |
| 3            | 0,18 | 0,18 |
| 4            | 0,36 | 0,36 |
| 5            | 0    | 0,36 |

### 1 nolu eleman



$$a_i = 0,36 \times 0,18 = 0,0648$$

$$b_i = -0,18$$

$$c_i = 0,18 - 0,36 = -0,18$$

$$a_k = 0 - 0,36 \times 0 = 0$$

$$b_k = 0 - 0 = 0$$

$$c_k = 0,36 - 0 = 0,36$$

$$a_j = 0,18 \times 0 - 0,18 \times 0 = 0$$

$$b_j = 0,18 - 0 = 0,18$$

$$c_j = -0,18$$

$$b_i + b_j + b_k = 0$$

$$c_i + c_j + c_k = 0$$

$$\Delta = \frac{0,36 \times 0,18}{2} = 0,0324$$



$$\{\varepsilon\} = \frac{1}{2 \times 0,0324} \begin{bmatrix} -0,18 & 0 & 0,18 & 0 & 0 & 0 \\ 0 & -0,18 & 0 & -0,18 & 0 & 0,36 \\ -0,18 & -0,18 & -0,18 & 0,18 & 0,36 & 0 \end{bmatrix} \{d\}$$

$$\{\sigma\} = \frac{E}{2 \times 0,0324 (1 - 0,25^2)} \begin{bmatrix} 1 & 0,25 & 0 \\ 0,25 & 1 & 0 \\ 0 & 0 & \frac{1-0,25}{2} \end{bmatrix} \begin{bmatrix} -0,18 & 0 & 0,18 & 0 & 0 & 0 \\ 0 & -0,18 & 0 & -0,18 & 0 & 0,36 \\ -0,18 & -0,18 & -0,18 & 0,18 & 0,36 & 0 \end{bmatrix} \{d\}$$

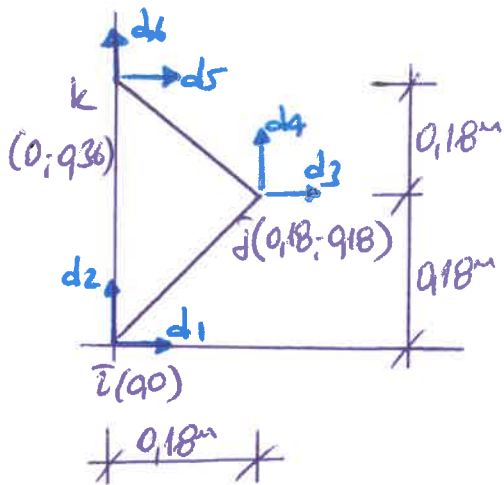
$$\{\sigma\} = \frac{E}{0,0648 (1 - 0,25^2)} \begin{bmatrix} -0,18 & -0,045 & 0,18 & -0,045 & 0 & 0,09 \\ -0,045 & -0,18 & 0,045 & -0,18 & 0 & 0,36 \\ -0,0675 & -0,0675 & -0,0675 & 0,0675 & 0,135 & 0 \end{bmatrix} \{d\}$$

$$[k_{e1}] = \frac{Et}{4\Delta^2(1-\nu^2)} \iint \begin{bmatrix} -0,18 & 0 & -0,18 \\ 0 & -0,18 & -0,18 \\ 0,18 & 0 & -0,18 \\ 0 & -0,18 & 0,18 \\ 0 & 0 & 0,36 \\ 0 & 0,36 & 0 \end{bmatrix} \begin{bmatrix} -0,18 & -0,045 & 0,18 & -0,045 & 0 & 0,09 \\ -0,045 & -0,18 & 0,045 & -0,18 & 0 & 0,36 \\ -0,0675 & -0,0675 & -0,0675 & 0,0675 & 0,135 & 0 \end{bmatrix} dx dy$$

$t = 0,01 \text{ m}^2, \Delta = 0,0324 \text{ m}^2$   
 $\nu = 0,25$

$$[k_{e1}] = \frac{Et}{0,1296(1-0,25^2)} \begin{bmatrix} 0,0445 & 0,02025 & -0,02025 & -0,00405 & -0,0243 & -0,0162 \\ 0,02025 & 0,0445 & 0,00405 & 0,02025 & -0,0243 & -0,0648 \\ -0,02025 & 0,00405 & 0,0445 & -0,02025 & -0,0243 & 0,0162 \\ -0,00405 & 0,02025 & -0,02025 & 0,0445 & 0,0243 & -0,0648 \\ -0,0243 & -0,0243 & -0,0243 & 0,0243 & 0,0486 & 0 \\ -0,0162 & -0,0648 & 0,0162 & -0,0648 & 0 & 0,1296 \end{bmatrix}$$

2 ndu elemen



$$\begin{array}{c|c|c} a_i = 0,0648 & a_j = 0 & a_k = 0 \\ b_i = -0,18 & b_j = 0,36 & b_k = -0,18 \\ c_i = -0,18 & c_j = 0 & c_k = 0,18 \end{array}$$

$$\iint dx dy = 0,0324$$

$$\{\epsilon\} = \frac{1}{2 \times 0,0324} \begin{bmatrix} -0,18 & 0 & 0,36 & 0 & -0,18 & 0 \\ 0 & -0,18 & 0 & 0 & 0 & 0,18 \\ -0,18 & -0,18 & 0 & 0,36 & 0,18 & -0,18 \end{bmatrix} \{d\}$$

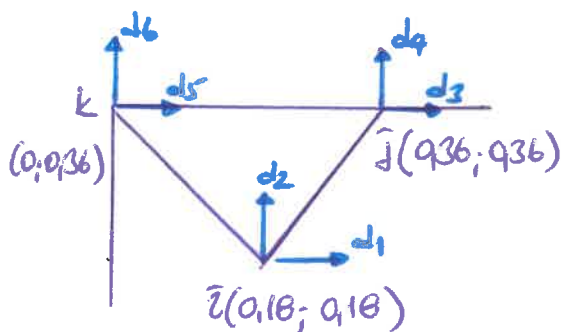
$$\{\sigma\} = \frac{E}{2 \times 0,0324 (1 - 0,25^2)} \begin{bmatrix} 1 & 0,25 & 0 \\ 0,25 & 1 & 0 \\ 0 & 0 & 0,375 \end{bmatrix} \begin{bmatrix} -0,18 & 0 & 0,36 & 0 & -0,18 & 0 \\ 0 & -0,18 & 0 & 0 & 0 & 0,18 \\ -0,18 & -0,18 & 0 & 0,36 & 0,18 & -0,18 \end{bmatrix} \{d\}$$

$$\{\sigma\} = \frac{E}{0,0648 (1 - 0,25^2)} \begin{bmatrix} -0,18 & -0,045 & 0,36 & 0 & -0,18 & 0,045 \\ -0,045 & -0,18 & 0,09 & 0 & -0,045 & 0,18 \\ -0,0675 & -0,0675 & 0 & 0,135 & 0,0675 & -0,0675 \end{bmatrix} \{d\}$$

$$[k_{e2}] = \frac{Et}{4 \Delta^2 (1 - 0,25^2)} \iint \begin{bmatrix} -0,18 & 0 & -0,18 \\ 0 & -0,18 & -0,18 \\ 0,36 & 0 & 0 \\ 0 & 0 & 0,36 \\ -0,18 & 0 & 0,18 \\ 0 & 0,18 & -0,18 \end{bmatrix} \begin{bmatrix} -0,18 & -0,045 & 0,36 & 0 & -0,18 & 0,045 \\ -0,045 & -0,18 & 0,09 & 0 & -0,045 & 0,18 \\ -0,0675 & -0,0675 & 0 & 0,135 & 0,0675 & -0,0675 \end{bmatrix} dx dy$$

$$[K_e] = \frac{Et}{0,1296(1-0,25^2)} \begin{bmatrix} 0,04455 & 0,02025 & -0,0648 & -0,0243 & 0,02025 & 0,00405 \\ 0,02025 & 0,04455 & -0,0162 & -0,0243 & -0,00405 & -0,02025 \\ -0,0648 & -0,0162 & 0,1296 & 0 & -0,0648 & 0,0162 \\ -0,0243 & -0,0243 & 0 & 0,0486 & 0,0243 & -0,0243 \\ 0,02025 & -0,00405 & -0,0648 & 0,0243 & 0,04455 & -0,02025 \\ 0,00405 & -0,02025 & 0,0162 & -0,0243 & -0,02025 & 0,04455 \end{bmatrix}$$

3 node elemen



$$a_i = 0,1296$$

$$b_i = 0$$

$$c_i = -0,36$$

$$a_j = -0,0648$$

$$b_j = 0,18$$

$$c_j = 0,18$$

$$a_k = 0$$

$$b_k = -0,18$$

$$c_k = 0,18$$

$$\{F\} = \frac{1}{0,0648} \begin{bmatrix} 0 & 0 & 0,18 & 0 & -0,18 & 0 \\ 0 & -0,36 & 0 & 0,18 & 0 & 0,18 \\ -0,36 & 0 & 0,18 & 0,18 & 0,18 & -0,18 \end{bmatrix} \{d\}$$

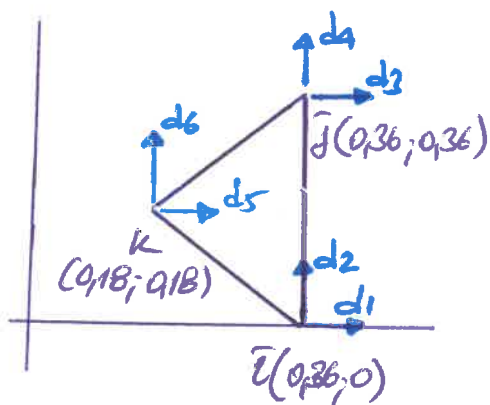
$$\{\sigma\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} 1 & 0,25 & 0 \\ 0,25 & 1 & 0 \\ 0 & 0 & 0,375 \end{bmatrix} \begin{bmatrix} 0 & 0 & 0,18 & 0 & -0,18 & 0 \\ 0 & -0,36 & 0 & 0,18 & 0 & 0,18 \\ -0,36 & 0 & 0,18 & 0,18 & 0,18 & -0,18 \end{bmatrix} \{d\}$$

$$\{\sigma\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} 0 & -0,09 & 0,18 & 0,045 & -0,18 & 0,045 \\ 0 & -0,36 & 0,045 & 0,18 & -0,045 & 0,18 \\ -0,135 & 0 & 0,0675 & 0,0675 & 0,0675 & -0,0675 \end{bmatrix} \{d\}$$

$$[K_3] = \frac{Et}{4\Delta^2(1-0,25^2)} \iint \begin{bmatrix} 0 & 0 & -0,36 \\ 0 & -0,36 & 0 \\ 0,18 & 0 & 0,18 \\ 0 & 0,18 & 0,18 \\ -0,18 & 0 & 0,18 \\ 0 & 0,18 & -0,18 \end{bmatrix} \begin{bmatrix} 0 & -0,09 & 0,18 & 0,045 & -0,18 & 0,045 \\ 0 & -0,36 & 0,045 & 0,18 & -0,045 & 0,18 \\ -0,135 & 0 & 0,0675 & 0,0675 & 0,0675 & -0,0675 \end{bmatrix} d\Omega$$

$$[K_3] = \frac{Et}{0,1296(1-0,25^2)} \begin{bmatrix} 0,0486 & 0 & -0,0243 & -0,0243 & -0,0243 & 0,0243 \\ 0 & 0,1296 & -0,0162 & -0,0648 & 0,0162 & -0,0648 \\ -0,0243 & -0,0162 & 0,04455 & 0,02025 & -0,02025 & -0,00405 \\ -0,0243 & -0,0648 & 0,02025 & 0,04455 & 0,00405 & 0,02025 \\ -0,0243 & 0,0162 & -0,02025 & 0,00405 & 0,04455 & -0,02025 \\ 0,0243 & -0,0648 & -0,00405 & 0,02025 & -0,02025 & 0,04455 \end{bmatrix}$$

4 node elemen



$$a_i = 0$$

$$b_i = 0,18$$

$$c_i = -0,18$$

$$a_j = -0,0648$$

$$b_j = 0,18$$

$$c_j = 0,18$$

$$a_k = 0,1296$$

$$b_k = -0,36$$

$$c_k = 0$$

$$\{F\} = \frac{1}{0,0648} \begin{bmatrix} 0,18 & 0 & 0,18 & 0 & -0,36 & 0 \\ 0 & -0,18 & 0 & 0,18 & 0 & 0 \\ -0,18 & 0,18 & 0,18 & 0,18 & 0 & -0,36 \end{bmatrix} \{d\}$$

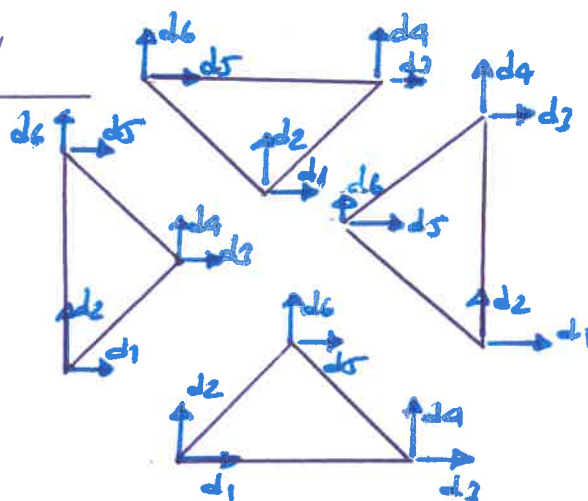
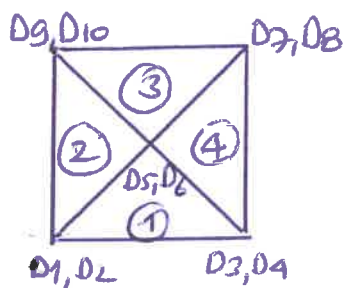
$$\{f\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} 1 & 0,25 & 0 \\ 0,25 & 1 & 0 \\ 0 & 0 & 0,75 \end{bmatrix} \begin{bmatrix} 0,18 & 0 & 0,18 & 0 & -0,36 & 0 \\ 0 & -0,18 & 0 & 0,18 & 0 & 0 \\ -0,18 & 0,18 & 0,18 & 0,18 & 0 & -0,36 \end{bmatrix} \{d\}$$

$$\{f\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} 0,18 & -0,045 & 0,18 & 0,045 & -0,36 & 0 \\ 0,045 & -0,18 & 0,045 & 0,18 & -0,09 & 0 \\ -0,0675 & 0,0675 & 0,0675 & 0,0675 & 0 & -0,135 \end{bmatrix} \{d\}$$

$$[k_4] = \frac{Et}{4\Delta^2(1-0,25^2)} \begin{bmatrix} 0,18 & 0 & -0,18 \\ 0 & -0,18 & 0,18 \\ 0,18 & 0 & 0,18 \\ 0 & 0,18 & 0,18 \\ -0,36 & 0 & 0 \\ 0 & 0 & -0,36 \end{bmatrix} \begin{bmatrix} 0,18 & -0,045 & 0,18 & 0,045 & -0,36 & 0 \\ 0,045 & -0,18 & 0,045 & 0,18 & -0,09 & 0 \\ -0,0675 & 0,0675 & 0,0675 & 0,0675 & 0 & -0,135 \end{bmatrix} dx dy$$

$$[k_4] = \frac{Et}{0,1296(1-0,25^2)^2} \begin{bmatrix} 0,04455 & -0,02025 & 0,02025 & -0,00405 & -0,0648 & 0,0243 \\ -0,02025 & 0,04455 & 0,00405 & -0,02025 & 0,0162 & -0,0243 \\ 0,02025 & 0,00405 & 0,04455 & 0,02025 & -0,0648 & -0,0243 \\ -0,00405 & -0,02025 & 0,02025 & 0,04455 & -0,0162 & -0,0243 \\ -0,0648 & 0,0162 & -0,0648 & -0,0162 & 0,1296 & 0 \\ 0,0243 & -0,0243 & -0,0243 & -0,0243 & 0 & 0,0486 \end{bmatrix}$$

Gesamte Matrixkei



$$[Ce_1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[Ce_2] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[Ce_3] = \begin{bmatrix} 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[Ce_4] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix}$$

Buradan elemen ri'lik matrisleri hesaplanir;

$$[K_{e1}] = [C_{e1}]^T [K_{e1}] [C_{e1}]$$

$$[K_{e2}] = [C_{e2}]^T [K_{e2}] [C_{e2}]$$

$$[K_{e3}] = [C_{e3}]^T [K_{e3}] [C_{e3}]$$

$$[K_{e4}] = [C_{e4}]^T [K_{e4}] [C_{e4}]$$

$$[Ke_1] = \begin{bmatrix} 0.04455 & 0.02025 & -0.02025 & -0.00405 & -0.0243 & -0.0162 & 0 & 0 & 0 & 0 \\ 0.02025 & 0.04455 & 0.00405 & 0.02025 & -0.0243 & -0.0648 & 0 & 0 & 0 & 0 \\ -0.02025 & 0.00405 & 0.04455 & -0.02025 & -0.0243 & 0.0162 & 0 & 0 & 0 & 0 \\ -0.00405 & 0.02025 & -0.02025 & 0.04455 & 0.0243 & -0.0648 & 0 & 0 & 0 & 0 \\ -0.0243 & -0.0243 & -0.0243 & 0.0243 & 0.0486 & 0 & 0 & 0 & 0 & 0 \\ -0.0162 & -0.0648 & 0.0162 & -0.0648 & 0 & 0.1296 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[Ke_2] = \begin{bmatrix} 0.04455 & 0.02025 & 0 & 0 & -0.0648 & -0.0243 & 0 & 0 & 0.02025 & 0.00405 \\ 0.02025 & 0.04455 & 0 & 0 & -0.0162 & -0.0243 & 0 & 0 & -0.00405 & -0.02025 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ -0.0648 & -0.0162 & 0 & 0 & 0.1296 & 0 & 0 & 0 & -0.0648 & 0.0162 \\ -0.0243 & -0.0243 & 0 & 0 & 0 & 0.0486 & 0 & 0 & 0.0243 & -0.0243 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0.02025 & -0.00405 & 0 & 0 & -0.0648 & 0.0243 & 0 & 0 & 0.04455 & -0.02025 \\ 0.00405 & -0.02025 & 0 & 0 & 0.0162 & -0.0243 & 0 & 0 & -0.02025 & 0.04455 \end{bmatrix}$$

$$[Ke_3] = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0.0486 & 0 & -0.0243 & -0.0243 & -0.0243 & 0.0243 \\ 0 & 0 & 0 & 0 & 0 & 0.1296 & -0.0162 & -0.0648 & 0.0162 & -0.0648 \\ 0 & 0 & 0 & 0 & -0.0243 & -0.0162 & 0.04455 & 0.02025 & -0.02025 & -0.00405 \\ 0 & 0 & 0 & 0 & -0.0243 & -0.0648 & 0.02025 & 0.04455 & 0.00405 & 0.02025 \\ 0 & 0 & 0 & 0 & -0.0243 & 0.0162 & -0.02025 & 0.00405 & 0.04455 & -0.02025 \\ 0 & 0 & 0 & 0 & 0.0243 & -0.0648 & -0.00405 & 0.02025 & -0.02025 & 0.04455 \end{bmatrix}$$

$$[Ke_4] = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0.04455 & -0.02025 & -0.0648 & 0.0243 & 0.02025 & -0.00405 & 0 & 0 \\ 0 & 0 & -0.02025 & 0.04455 & 0.0162 & -0.0243 & 0.00405 & -0.02025 & 0 & 0 \\ 0 & 0 & -0.0648 & 0.0162 & 0.1296 & 0 & -0.0648 & -0.0162 & 0 & 0 \\ 0 & 0 & 0.0243 & -0.0243 & 0 & 0.0486 & -0.0243 & -0.0243 & 0 & 0 \\ 0 & 0 & 0.02025 & 0.00405 & -0.0648 & -0.0243 & 0.04455 & 0.02025 & 0 & 0 \\ 0 & 0 & -0.00405 & -0.02025 & -0.0162 & -0.0243 & 0.02025 & 0.04455 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

matrisleri elde edilir. Toplam Rijitlik matrisi;

$$[K_e] = [K_{e1}] + [K_{e2}] + [K_{e3}] + [K_{e4}] \text{ şeklinde.}$$



$$\begin{array}{c}
 \text{K} \\
 \hline
 0,1296/1-
 \end{array}
 \begin{bmatrix}
 0.0891 & 0.0405 & -0.02025 & -0.00405 & -0.0891 & -0.0405 & 0 & 0.02025 & 0.00405 & D1 \\
 0.0405 & 0.0891 & 0.00405 & 0.02025 & -0.0405 & -0.0891 & 0 & -0.00405 & -0.02025 & D2 \\
 -0.02025 & 0.00405 & 0.0891 & -0.0405 & -0.0891 & 0.0405 & 0.02025 & -0.00405 & 0 & D3 \\
 -0.00405 & 0.02025 & -0.0405 & 0.0891 & 0.0405 & -0.0891 & 0.00405 & -0.02025 & 0 & D4 \\
 -0.0891 & -0.0405 & -0.0891 & 0.0405 & 0.3564 & 0 & -0.0891 & -0.0405 & 0.0405 & D5 \\
 -0.0405 & -0.0891 & 0.0405 & -0.0891 & 0 & 0.3564 & -0.0405 & -0.0891 & -0.0891 & D6 \\
 0 & 0 & 0.02025 & 0.00405 & -0.0891 & -0.0405 & 0.0891 & 0.0405 & -0.02025 & D7 \\
 0 & 0 & -0.00405 & -0.02025 & -0.0405 & -0.0891 & 0.0405 & 0.0891 & 0.00405 & D8 \\
 0.02025 & -0.00405 & 0 & 0 & -0.0891 & 0.0405 & -0.0203 & 0.00405 & 0.0891 & D9 \\
 0.00405 & -0.02025 & 0 & 0 & 0.0405 & -0.0891 & -0.0041 & 0.02025 & -0.0405 & D10
 \end{bmatrix}
 =
 \begin{bmatrix}
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 -420 \\
 0 \\
 0 \\
 0 \\
 0
 \end{bmatrix}$$

$$\begin{bmatrix}
 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0.3564 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0.3564 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1
 \end{bmatrix}
 \begin{bmatrix}
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0
 \end{bmatrix}
 =
 \begin{bmatrix}
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 -420 \\
 0 \\
 0 \\
 0 \\
 0
 \end{bmatrix}$$

Berada  $D5=0$

$$D6 = - \frac{420 \times 0,1296(1-0,125^2)}{21 \times 10^7 \times 0,1296 \times 0,3564} = -6,82 \times 10^{-5} \text{ m.}$$

Geriölçmeleri hesabı ;

1 nolu eleman

$$\{\sigma\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} -0,18 & -0,045 & 0,18 & -0,045 & 0 & 0,09 \\ -0,045 & -0,18 & 0,045 & -0,18 & 0 & 0,36 \\ -0,0675 & -0,0675 & -0,0675 & 0,0675 & 0,135 & 0 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ -6,62 \cdot 10^{-5} \end{bmatrix}$$

$$\sigma_x^1 = \frac{E}{0,0648(1-0,25^2)} (0,09) (-6,62 \cdot 10^{-5}) \cdot 10^3 = -21,22 \text{ N/mm}^2$$

$$\sigma_y^1 = \frac{E}{0,0648(1-0,25^2)} (0,36) (-6,62 \cdot 10^{-5}) \cdot 10^3 = -84,88 \text{ N/mm}^2$$

$$\tau_{xy}^1 = 0$$

2 ve 4 nolu elemanlar

$$\{\sigma\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} -0,18 & -0,045 & 0,36 & 0 & -0,18 & 0,045 \\ -0,045 & -0,18 & 0,09 & 0 & -0,045 & 0,18 \\ -0,0675 & -0,0675 & 0 & 0,135 & 0,0675 & -0,0675 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ 0 \\ -6,62 \cdot 10^{-5} \\ 0 \\ 0 \end{bmatrix}$$

$$\sigma_x^{2,4} = \frac{E}{0,0648(1-0,25^2)} (0) (-6,62 \cdot 10^{-5}) \cdot 10^3 = 0$$

$$\sigma_y^{2,4} = 0$$

$$\tau_{xy} = \frac{E}{0,0648(1-0,25^2)} (0,135) (-6,62 \cdot 10^{-5}) \cdot 10^3 = -31,62 \text{ N/mm}^2$$

3 nolu eleman

$$\{\sigma\} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} 0 & -0,09 & 0,18 & 0,045 & -0,18 & 0,045 \\ 0 & -0,36 & 0,045 & 0,18 & -0,045 & 0,18 \\ 0,135 & 0 & 0,0675 & 0,0675 & 0,0675 & -0,0675 \end{bmatrix} \begin{bmatrix} 0 \\ -6,62 \cdot 10^{-5} \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$

$$\sigma_x^3 = \frac{E}{0,0648(1-0,25^2)} (-0,09) (-6,62 \cdot 10^{-5}) \cdot 10^3 = 21,22 \text{ N/mm}^2$$

$$\sigma_y^3 = \frac{E}{0,0648(1-0,25^2)} (-0,36) ( \quad ) \cdot 10^3 = 84,88 \text{ N/mm}^2 \quad \tau_{xy}^3 = 0$$

Satırı incelediğimizde yalnızca  $D_6$  yer değiştirmesinin olduğu, diğer yer değiştirmelerin sıfır olduğu görülmektedir. Bu durumda 1 elemanında  $k_{66}$ , 2 elemanında  $k_{44}$ , 3 elemanında  $k_{22}$  ve 4 elemanında  $k_{66}$  rijitlik değerlerini alabiliriz. Üçgen elemanlar için hazırlanan (30) nolu tablodan;

$$k_{22}^3 = \frac{Et}{4\Delta(1-\nu^2)} \left[ \left( \frac{1-\nu}{2} \right) b_i^2 + c_i^2 \right] = \frac{Et}{4\Delta(1-\nu^2)} 0,1296 \quad \left\| \begin{array}{l} b_i = 0 \\ c_i = -0,36 \end{array} \right.$$

$$k_{44}^2 = \frac{Et}{4\Delta(1-\nu^2)} \left[ \left( \frac{1-\nu}{2} \right) b_j^2 + c_j^2 \right] = \frac{Et}{4\Delta(1-\nu^2)} 0,0486 \quad \left\| \begin{array}{l} b_j = 0,36 \\ c_j = 0 \end{array} \right.$$

$$k_{66}^1 = \frac{Et}{4\Delta(1-\nu^2)} \left[ \left( \frac{1-\nu}{2} \right) b_k^2 + c_k^2 \right] = \frac{Et}{4\Delta(1-\nu^2)} 0,1296 \quad \left\| \begin{array}{l} b_k = 0 \\ c_k = 0,36 \end{array} \right.$$

$$k_{66}^4 = \frac{Et}{4\Delta(1-\nu^2)} \left[ \left( \frac{1-\nu}{2} \right) b_k^2 + c_k^2 \right] = \frac{Et}{4\Delta(1-\nu^2)} 0,0486 \quad \left\| \begin{array}{l} b_k = -0,36 \\ c_k = 0 \end{array} \right.$$

$$K_{66} = \frac{Et}{4\Delta(1-\nu^2)} (0,1296 \times 2 + 0,0486 \times 2) = \frac{Et}{4\Delta(1-\nu^2)} 0,3564$$

$$K_{66} \times D_6 = -Q$$

$$\frac{Et}{4\Delta(1-\nu^2)} 0,3564 D_6 = -420$$

$$D_6 = - \frac{420 \times 0,1296 \times (1-0,015^2)}{21 \times 10^2 \times 0,01 \times 0,3564} = -6,82 \times 10^{-5} \text{ m.}$$

Benzer şekilde gerilmeler hesaplanırsa;

1 nolu eleman için;

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \frac{E}{0,0648(1-0,25^2)} \begin{bmatrix} -0,18 & -0,045 & 0,18 & -0,045 & 0 & 0,09 \\ -0,045 & -0,18 & 0,045 & -0,18 & 0 & 0,36 \\ -0,0675 & -0,0675 & -0,0675 & 0,0675 & 0,135 & 0 \end{bmatrix} \begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{bmatrix}$$

\* 1 nolu eleman için 06'ya karşı gelen yer değiştirmeye  $d_6$ 'dır. Dolayısıyla her sahada  $d_6$ 'ya karşı gelen katsayılar ele alınacaktır.

$$\begin{aligned} \sigma_x^1 &= \frac{E}{0,0648(1-0,25^2)} (0,09) (-6,82 \times 10^{-5}) \times 10^3 = -21,22 \text{ N/mm}^2 \\ \sigma_y^1 &= \frac{E}{0,0648(1-0,25^2)} (0,36) (-6,82 \times 10^{-5}) \times 10^3 = -84,88 \text{ N/mm}^2 \\ \tau_{xy}^1 &= 0 \end{aligned}$$

2 ve 4 nolu elemanlar için;

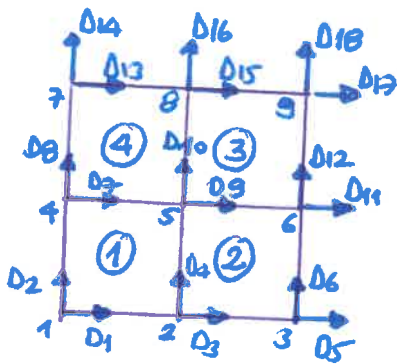
\* 2 ve 4 nolu elemanlarda 06'ya karşı gelen yer değiştirmeye  $d_4$ 'tür

$$\begin{aligned} \sigma_x^{24} &= \frac{E}{0,0648(1-0,25^2)} (0) (-6,82 \times 10^{-5}) \times 10^3 = 0 \\ \sigma_y^{24} &= 0 \\ \tau_{xy}^{24} &= \frac{E}{0,0648(1-0,25^2)} (0,135) (-6,82 \times 10^{-5}) \times 10^3 = -31,82 \text{ N/mm}^2 \end{aligned}$$

3 nolu eleman için;

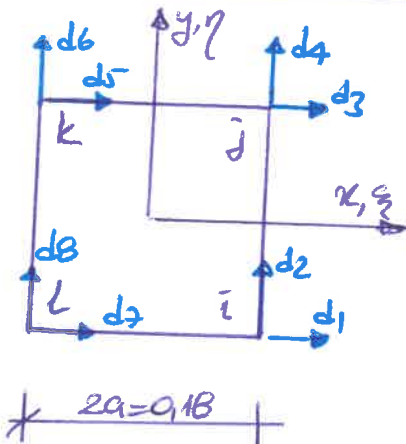
$$\begin{aligned} \sigma_x^3 &= \frac{E}{0,0648(1-0,25^2)} (-0,09) (-6,82 \times 10^{-5}) \times 10^3 = 21,22 \text{ N/mm}^2 \\ \sigma_y^3 &= \frac{E}{0,0648(1-0,25^2)} (-0,36) (-6,82 \times 10^{-5}) \times 10^3 = 84,88 \text{ N/mm}^2 \\ \tau_{xy}^3 &= 0 \end{aligned}$$

## b) Dörtgen elemanlara gözüm



| Koordinatlar | x(m) | y(m) |
|--------------|------|------|
| 1            | 0    | 0    |
| 2            | 0,18 | 0    |
| 3            | 0,36 | 0    |
| 4            | 0    | 0,18 |
| 5            | 0,18 | 0,18 |
| 6            | 0,36 | 0,18 |
| 7            | 0    | 0,36 |
| 8            | 0,18 | 0,36 |
| 9            | 0,36 | 0,36 |

## Elemanlerin formasyonun hesabı



$$\xi = \frac{x}{a} \quad \eta = \frac{y}{b}$$

$$D = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}$$

$$D_{11} = D_{22} = \frac{E}{1-\nu^2} = \frac{21 \cdot 10^7}{1-0,25^2} = 22,4 \cdot 10^7$$

$$D_{12} = D_{21} = \frac{E}{1-\nu^2} \nu = \frac{21 \cdot 10^7}{1-0,25^2} \cdot 0,25 = 5,6 \cdot 10^7$$

$$D_{33} = \frac{E}{(1+\nu)2} = \frac{21 \cdot 10^7}{(1+0,25)2} = 8,4 \cdot 10^7$$

$$[k_e] = \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} & k_{15} & k_{16} & k_{17} & k_{18} \\ k_{21} & k_{22} & k_{23} & k_{24} & k_{25} & k_{26} & k_{27} & k_{28} \\ k_{31} & k_{32} & k_{33} & k_{34} & k_{35} & k_{36} & k_{37} & k_{38} \\ k_{41} & k_{42} & k_{43} & k_{44} & k_{45} & k_{46} & k_{47} & k_{48} \\ k_{51} & k_{52} & k_{53} & k_{54} & k_{55} & k_{56} & k_{57} & k_{58} \\ k_{61} & k_{62} & k_{63} & k_{64} & k_{65} & k_{66} & k_{67} & k_{68} \\ k_{71} & k_{72} & k_{73} & k_{74} & k_{75} & k_{76} & k_{77} & k_{78} \\ k_{81} & k_{82} & k_{83} & k_{84} & k_{85} & k_{86} & k_{87} & k_{88} \end{bmatrix}$$

$$\xi = \frac{b}{a} = 1$$

$$k_{11} = \frac{0,01}{4} (22,4 \times 10^7 + 8,4 \times 10^7) = 77 \times 10^4$$

$$k_{21} = \frac{0,01}{4} (5,6 \times 10^7 + 8,4 \times 10^7) = 35 \times 10^4$$

$$k_{22} = \frac{0,01}{3} (22,4 \times 10^7 + 8,4 \times 10^7) = 102,67 \times 10^4$$

$$k_{31} = \frac{0,01}{4} \left( \frac{22,4}{2} \times 10^7 - 8,4 \times 10^7 \right) = 7 \times 10^4$$

$$k_{32} = \frac{0,01}{4} (5,6 \times 10^7 - 8,4 \times 10^7) = -7 \times 10^4$$

$$k_{33} = \frac{0,01}{3} (22,4 \times 10^7 + 8,4 \times 10^7) = 102,67 \times 10^4$$

$$k_{41} = \frac{0,01}{4} (-5,6 \times 10^7 + 8,4 \times 10^7) = 7 \times 10^4$$

$$k_{42} = \frac{0,01}{3} \left( -22,4 \times 10^7 + \frac{8,4}{2} \times 10^7 \right) = -60,67 \times 10^4$$

$$k_{43} = \frac{0,01}{4} (-5,6 \times 10^7 - 8,4 \times 10^7) = -35 \times 10^4$$

$$k_{44} = \frac{0,01}{3} (22,4 \times 10^7 + 8,4 \times 10^7) = 102,67 \times 10^4$$

$$k_{51} = \frac{0,01}{3} \left( -22,4 \times 10^7 + \frac{8,4}{2} \times 10^7 \right) = -60,67 \times 10^4$$

$$k_{52} = \frac{0,01}{4} (-5,6 \times 10^7 + 8,4 \times 10^7) = 7 \times 10^4$$

$$k_{53} = \frac{0,01}{6} (-22,4 \times 10^7 - 8,4 \times 10^7) = -51,33 \times 10^4$$

$$k_{54} = \frac{0,01}{4} (5,6 \times 10^7 + 8,4 \times 10^7) = 35 \times 10^4$$

$$k_{55} = \frac{0,01}{3} (224 \times 10^7 + 84 \times 10^7) = 10267 \times 10^4$$

$$k_{61} = \frac{0,01}{4} (5,6 \times 10^7 - 8,4 \times 10^7) = -7 \times 10^4$$

$$k_{62} = \frac{0,01}{3} \left( \frac{224}{2} \times 10^7 - 84 \times 10^7 \right) = 9,33 \times 10^4$$

$$k_{63} = \frac{0,01}{4} (5,6 \times 10^7 + 8,4 \times 10^7) = 35 \times 10^4$$

$$k_{64} = \frac{0,01}{6} (-224 \times 10^7 - 84 \times 10^7) = -51,33 \times 10^4$$

$$k_{65} = \frac{0,01}{4} (-5,6 \times 10^7 - 8,4 \times 10^7) = -35 \times 10^4$$

$$k_{66} = \frac{0,01}{3} (224 \times 10^7 + 84 \times 10^7) = 10267 \times 10^4$$

$$k_{71} = \frac{0,01}{6} (-224 \times 10^7 - 84 \times 10^7) = -51,33 \times 10^4$$

$$k_{72} = \frac{0,01}{4} (-5,6 \times 10^7 - 8,4 \times 10^7) = -35 \times 10^4$$

$$k_{73} = \frac{0,01}{3} \left( -224 \times 10^7 + \frac{84}{2} \times 10^7 \right) = -60,67 \times 10^4$$

$$k_{74} = \frac{0,01}{4} (5,6 \times 10^7 - 8,4 \times 10^7) = -7 \times 10^4$$

$$k_{75} = \frac{0,01}{3} \left( \frac{224}{2} \times 10^7 - 84 \times 10^7 \right) = 9,33 \times 10^4$$

$$k_{76} = \frac{0,01}{4} (-5,6 \times 10^7 + 8,4 \times 10^7) = 7 \times 10^4$$

$$k_{77} = \frac{0,01}{3} (224 \times 10^7 + 84 \times 10^7) = 10267 \times 10^4$$



$$k_{\theta 1} = \frac{0,01}{4} (-5,6 \times 10^7 - 8,4 \times 10^7) = -35 \times 10^4$$

$$k_{\theta 2} = \frac{0,01}{6} (-22,4 \times 10^7 - 8,4 \times 10^7) = -51,33 \times 10^4$$

$$k_{\theta 3} = \frac{0,01}{4} (-5,6 \times 10^7 + 8,4 \times 10^7) = 7 \times 10^4$$

$$k_{\theta 4} = \frac{0,01}{3} \left( \frac{22,4}{2} \times 10^7 - 8,4 \times 10^7 \right) = 9,33 \times 10^4$$

$$k_{\theta 5} = \frac{0,01}{4} (5,6 \times 10^7 - 8,4 \times 10^7) = -7 \times 10^4$$

$$k_{\theta 6} = \frac{0,01}{3} \left( -22,4 \times 10^7 + \frac{8,4}{2} \times 10^7 \right) = -6067 \times 10^4$$

$$k_{\theta 7} = \frac{0,01}{4} (5,6 \times 10^7 + 8,4 \times 10^7) = 35 \times 10^4$$

$$k_{\theta 8} = \frac{0,01}{3} (22,4 \times 10^7 + 8,4 \times 10^7) = 10267 \times 10^4$$

|     |         |         |         |         |         |         |         |         |
|-----|---------|---------|---------|---------|---------|---------|---------|---------|
| ke1 | 770000  | 350000  | 70000   | 70000   | -606700 | -70000  | -513300 | -350000 |
|     | 350000  | 1026700 | -70000  | -606700 | 70000   | 93300   | -350000 | -513300 |
|     | 70000   | -70000  | 1026700 | -350000 | -513300 | 350000  | -606700 | 70000   |
|     | 70000   | -606700 | -350000 | 1026700 | 350000  | -513300 | -70000  | 93300   |
|     | -606700 | 70000   | -513300 | 350000  | 1026700 | -350000 | 93300   | -70000  |
|     | -70000  | 93300   | 350000  | -513300 | -350000 | 1026700 | 70000   | -606700 |
|     | -513300 | -350000 | -606700 | -70000  | 93300   | 70000   | 1026700 | 350000  |
|     | -350000 | -513300 | 70000   | 93300   | -70000  | -606700 | 350000  | 1026700 |

|     |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |
|-----|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
|     | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| ce1 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

|     |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |
|-----|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
|     | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| ce2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

|     |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |
|-----|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |
| ce3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

|     |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |
|-----|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 |
| ce4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|     | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

$$ke_1 = [c_{e1}]^T [k_e] [c_{e1}]$$

$$ke_2 = [c_{e2}]^T [k_e] [c_{e2}]$$

$$ke_3 = [c_{e3}]^T [k_e] [c_{e3}]$$

$$ke_4 = [c_{e4}]^T [k_e] [c_{e4}]$$





[illegible]



[illegible]





[illegible]

1 elemanı 1 noktası :  $\xi=-1$   $\eta=-1$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = -\frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0 & 0 & 0.18 & 0 & 0 & 0 \\ 0 & 0 & 0.045 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0.0675 & 0.0675 & 0 \end{bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} -0.0000045 \\ -0.000018 \\ -0.000015525 \end{Bmatrix} = \begin{Bmatrix} -31.1112 \\ -124.444 \\ -107.333 \end{Bmatrix}$$

$$\begin{bmatrix} 0 & -0.00013 & 0 & -0.00023 & 0 & -0.0001 \\ -0.00013 & 0 & -0.00023 & 0 & -0.0001 & 0 \\ 0 & -0.00023 & 0 & -0.0001 & 0 & 0 \end{bmatrix}$$

$$\begin{bmatrix} 0.045 & -0.18 & -0.045 & -0.0675 \\ 0.18 & -0.045 & -0.18 & -0.0675 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

1 elemanı 2 noktası :  $\xi=0$   $\eta=-1$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = -\frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0 & 0.0225 & 0.18 & -0.0225 & 0 & 0.0225 \\ 0 & 0.09 & 0.045 & -0.09 & 0 & 0.09 \\ 0.03375 & 0 & -0.03375 & 0.0675 & 0.03375 & 0 \end{bmatrix}$$

$$\begin{bmatrix} 0.0225 & -0.18 & -0.0225 \\ 0.09 & -0.045 & -0.09 \\ 0 & -0.03375 & -0.0675 \end{bmatrix}$$

$$\begin{bmatrix} 0 & -0.00013 & 0 & -0.00023 & 0 & -0.0001 \\ -0.00013 & 0 & -0.00023 & 0 & -0.0001 & 0 \\ 0 & -0.00023 & 0 & -0.0001 & 0 & 0 \end{bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ 0 \\ -0.000015525 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ -107.334 \end{Bmatrix}$$

1 Elemanı 5 noktası :  $\xi=0$   $\eta=0$

$$\begin{Bmatrix} U_x \\ U_y \\ Z_{xy} \end{Bmatrix} = \frac{e}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0.0225 & 0.09 & -0.0225 & 0.09 & -0.0225 \\ 0.0225 & 0.09 & 0.0225 & -0.09 & 0.0225 & -0.09 \\ 0.03375 & 0.03375 & -0.03375 & 0.03375 & -0.03375 & -0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ -0.000008775 \end{Bmatrix}$$

$$\begin{Bmatrix} U_x \\ U_y \\ Z_{xy} \end{Bmatrix} = \begin{Bmatrix} 6913600 \\ 0 \\ -0.000008775 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ -69667 \end{Bmatrix}$$

$$\begin{bmatrix} 0 \\ -0.00013 \\ 0 \\ -0.00023 \\ 0 \\ -0.0001 \\ 0 \\ 0 \end{bmatrix}$$

1 Elemanı 4 noktası :  $\xi=-1$   $\eta=0$

$$\begin{Bmatrix} U_x \\ U_y \\ Z_{xy} \end{Bmatrix} = \frac{e}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0 & 0.09 & 0 & 0.09 & 0 \\ 0.0225 & 0 & 0.0225 & 0 & 0.0225 & 0 \\ 0 & 0.03375 & 0 & 0.03375 & 0 & 0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ -0.000008775 \end{Bmatrix}$$

$$\begin{Bmatrix} U_x \\ U_y \\ Z_{xy} \end{Bmatrix} = \begin{Bmatrix} -0.0000045 \\ 6913600 \\ -0.000008775 \end{Bmatrix} = \begin{Bmatrix} -31.1112 \\ -124.44 \\ -60.6668 \end{Bmatrix}$$

$$\begin{bmatrix} 0 \\ -0.00013 \\ 0 \\ -0.00023 \\ -0.0001 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$

2 Elemanı 6 noktası :  $\xi=+1$   $\eta=0$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0.045 & 0.09 & -0.045 & -0.09 & 0 \\ 0.0225 & 0.18 & 0.0225 & -0.18 & -0.0225 & 0 \\ 0.0675 & 0.03375 & -0.0675 & 0.03375 & 0 & -0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -0.0001 \\ 0 \\ -0.00023 \\ 0 \\ -0.00013 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0.0000045 \\ 0.000018 \\ 0.000008775 \end{Bmatrix} = \begin{Bmatrix} 31.1112 \\ 124.4448 \\ 60.66684 \end{Bmatrix}$$

2 Elemanı 5 noktası :  $\xi=0$   $\eta=0$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0.0225 & 0.09 & -0.0225 & -0.09 & 0.0225 \\ 0.0225 & 0.09 & 0.0225 & -0.09 & -0.0225 & -0.09 \\ 0.03375 & 0.03375 & -0.03375 & 0.03375 & -0.03375 & -0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -0.0001 \\ 0 \\ -0.00023 \\ 0 \\ -0.00013 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ 0 \\ 0.000008775 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 60.66684 \end{Bmatrix}$$

2 Elemanı 2 noktası :  $\xi=0$   $\eta=-1$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0 & 0.0225 & 0.18 & -0.0225 & 0 & 0.0225 & -0.18 & -0.0225 \\ 0 & 0.09 & 0.045 & -0.09 & 0 & 0.09 & -0.045 & -0.09 \\ 0.03375 & 0 & -0.03375 & 0.0675 & 0.03375 & 0 & -0.03375 & -0.0675 \end{bmatrix} \begin{Bmatrix} D5 \\ D6 \\ D11 \\ D12 \\ D9 \\ D10 \\ D3 \\ D4 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ 0 \\ 0.000002025 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 14.00004 \end{Bmatrix}$$

2 Elemanı 3 noktası :  $\xi=+1$   $\eta=-1$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0 & 0.045 & 0.18 & -0.045 & 0 & 0 & -0.18 & 0 \\ 0 & 0.18 & 0.045 & -0.18 & 0 & 0 & -0.045 & 0 \\ 0.0675 & 0 & -0.0675 & 0.0675 & 0 & 0 & 0 & -0.0675 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -0.0001 \\ 0 \\ -0.00023 \\ 0 \\ -0.00013 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0.00000045 \\ 0.000018 \\ 0.000002025 \end{Bmatrix} = \begin{Bmatrix} 31.1112 \\ 124.4448 \\ 14.00004 \end{Bmatrix}$$

3 Elemanı 6 noktası :  $\xi=+1$   $\eta=0$

$$\begin{Bmatrix} U^x \\ U^y \\ Z_{xy} \end{Bmatrix} = \frac{E}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0.045 & 0.09 & -0.045 & -0.09 & 0 & -0.09 & 0 \\ 0.0225 & 0.18 & 0.0225 & -0.18 & -0.0225 & 0 & -0.0225 & 0 \\ 0.0675 & 0.03375 & -0.0675 & 0.03375 & 0 & -0.03375 & 0 & -0.03375 \end{bmatrix} \begin{Bmatrix} D_{11} \\ D_{12} \\ D_{17} \\ D_{18} \\ D_{15} \\ D_{16} \\ D_9 \\ D_{10} \end{Bmatrix}$$

$$\begin{Bmatrix} U^x \\ U^y \\ Z_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} -0.00000045 \\ -0.0000018 \\ 0.0000008775 \end{Bmatrix} = \begin{Bmatrix} -31.1112 \\ -124.445 \\ 60.66684 \end{Bmatrix}$$

3 Elemanı 9 noktası :  $\xi=+1$   $\eta=+1$

$$\begin{Bmatrix} U^x \\ U^y \\ Z_{xy} \end{Bmatrix} = \frac{E}{4ab(1-\nu^2)} \begin{bmatrix} 0.18 & 0.045 & 0 & -0.045 & -0.18 & 0 & 0 & 0 \\ 0.045 & 0.18 & 0 & -0.18 & -0.045 & 0 & 0 & 0 \\ 0.0675 & 0.0675 & -0.0675 & 0 & 0 & -0.0675 & 0 & 0 \end{bmatrix} \begin{Bmatrix} D_{11} \\ D_{12} \\ D_{17} \\ D_{18} \\ D_{15} \\ D_{16} \\ D_9 \\ D_{10} \end{Bmatrix}$$

$$\begin{Bmatrix} U^x \\ U^y \\ Z_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} -0.00000045 \\ -0.0000018 \\ 0.0000002025 \end{Bmatrix} = \begin{Bmatrix} -31.1112 \\ -124.445 \\ 14.00004 \end{Bmatrix}$$



3 Elemanı 8 noktası :  $\xi=0$   $\eta=+1$

$$\begin{Bmatrix} U_x \\ U_y \\ U_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0.18 & 0.0225 & 0 & -0.0225 & -0.18 & 0.0225 & 0 & -0.0225 \\ 0.045 & 0.09 & 0 & -0.09 & -0.045 & 0.09 & 0 & -0.09 \\ 0.03375 & 0.0675 & -0.03375 & 0 & 0.03375 & -0.0675 & -0.03375 & 0 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} U_x \\ U_y \\ U_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ 0 \\ 0.000002025 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 14.00004 \end{Bmatrix}$$

3 Elemanı 5 noktası :  $\xi=0$   $\eta=0$

$$\begin{Bmatrix} U_x \\ U_y \\ U_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0.0225 & 0.09 & -0.0225 & -0.09 & 0.0225 & -0.09 & -0.0225 \\ 0.0225 & 0.09 & 0.0225 & -0.09 & -0.0225 & 0.09 & -0.0225 & -0.09 \\ 0.03375 & 0.03375 & -0.03375 & 0.03375 & 0.03375 & -0.03375 & -0.03375 & 0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ -0.0001 \\ 0 \\ 0 \\ 0 \\ -0.00013 \\ 0 \\ -0.00023 \end{Bmatrix}$$

$$\begin{Bmatrix} U_x \\ U_y \\ U_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ 0 \\ 0.000008775 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 60.66684 \end{Bmatrix}$$

4 Elemanı 5 noktası :  $\xi=0$   $\eta=0$

$$\begin{Bmatrix} G_x \\ G_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0.0225 & 0.09 & -0.0225 & -0.09 & 0.0225 & -0.0225 & -0.0225 \\ 0.0225 & 0.09 & 0.0225 & -0.09 & -0.0225 & 0.09 & -0.0225 & -0.09 \\ 0.03375 & 0.03375 & -0.03375 & 0.03375 & 0.03375 & -0.03375 & -0.03375 & -0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ -0.000008775 \\ 0 \\ 0 \\ 0 \\ 0 \\ -0.0001 \end{Bmatrix}$$

$$\begin{Bmatrix} G_x \\ G_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ 0 \\ -0.000008775 \\ 0 \\ 0 \\ 0 \\ 0 \\ -60.66684 \end{Bmatrix}$$

4 elemanı 7 noktası :  $\xi=-1$   $\eta=+1$

$$\begin{Bmatrix} G_x \\ G_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{4ab(1-\nu^2)} \begin{bmatrix} 0.18 & 0 & 0 & 0 & 0 & 0.045 & -0.045 & 0 \\ 0.045 & 0 & 0 & 0 & 0 & 0.18 & -0.18 & 0 \\ 0 & 0.0675 & 0 & 0 & 0.0675 & -0.0675 & -0.0675 & 0 \end{bmatrix} \begin{Bmatrix} 0 \\ -0.00023 \\ 0 \\ -0.00013 \\ 0 \\ 0 \\ 0 \\ -0.0001 \end{Bmatrix}$$

$$\begin{Bmatrix} G_x \\ G_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0.0000045 \\ 0.000018 \\ -0.000015525 \\ 31.1112 \\ 124.4448 \\ -107.3336 \\ 0 \\ 0 \end{Bmatrix}$$

4 elemanı 8 noktası :  $\xi=0$   $\eta=+1$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0.18 & 0.0225 & 0 & -0.0225 & -0.18 & 0.0225 & 0 & -0.0225 \\ 0.045 & 0.09 & 0 & -0.09 & -0.045 & 0.09 & 0 & -0.09 \\ 0.03375 & 0.0675 & -0.03375 & 0 & 0.03375 & -0.0675 & -0.03375 & 0 \end{bmatrix} \begin{Bmatrix} 0 \\ -0.00023 \\ 0 \\ -0.00013 \\ 0 \\ 0 \\ 0 \\ -0.0001 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0 \\ -0.00015525 \\ -107.3336 \end{Bmatrix}$$

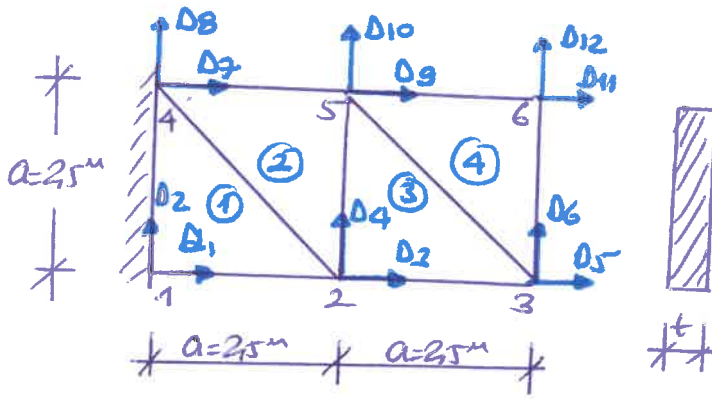
4 Elemanı 4 noktası :  $\xi=-1$   $\eta=0$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{\epsilon}{4ab(1-\nu^2)} \begin{bmatrix} 0.09 & 0 & 0.09 & 0 & -0.09 & 0.045 & -0.09 & -0.045 \\ 0.0225 & 0 & 0.0225 & 0 & -0.0225 & 0.18 & -0.0225 & -0.18 \\ 0 & 0.03375 & 0 & 0.03375 & 0.0675 & -0.03375 & -0.0675 & -0.03375 \end{bmatrix} \begin{Bmatrix} 0 \\ -0.00023 \\ 0 \\ -0.00013 \\ 0 \\ 0 \\ 0 \\ -0.0001 \end{Bmatrix}$$

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = 6913600 \begin{Bmatrix} 0.0000045 \\ 0.0000018 \\ -0.000008775 \end{Bmatrix}$$

|                     |  |                |              |  |                   |
|---------------------|--|----------------|--------------|--|-------------------|
| 31<br>124<br>-107   |  | 0<br>0<br>-107 | 0<br>0<br>14 |  | -31<br>-124<br>14 |
| 31<br>124<br>-60    |  | 0<br>0<br>-60  | 0<br>0<br>60 |  | -31<br>-124<br>60 |
| -31<br>-124<br>-60  |  | 0<br>0<br>-60  | 0<br>0<br>60 |  | 31<br>124<br>60   |
| -31<br>-124<br>-107 |  | 0<br>0<br>-107 | 0<br>0<br>14 |  | 31<br>124<br>14   |

Örnek: 3) Şekilde boyutları veritmiş olan levhanın 6 nolu  
 düğüm noktası düzey yönde 3 cm ötkme yapabilmesi durumunda  
 taşıyabileceği düğüm noktası yüklemni bulunuz. 3 numaralı  
 alanın  $\Delta t = 30^\circ\text{C}$  ısınnması durumunda düğüm noktası yüklemni  
 hesaplayınız.



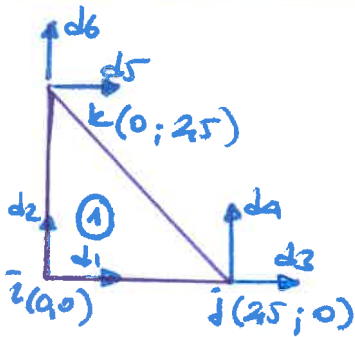
$$E = 210000 \text{ N/mm}^2 = 21 \times 10^7 \text{ kN/m}^2$$

$$\nu = 0.30, t = 0.03 \text{ m}$$

$$\alpha_t = 1.2 \times 10^{-4} \text{ 1/}^\circ\text{C}$$

$$\delta_{6y} = 0.02 \text{ m}$$

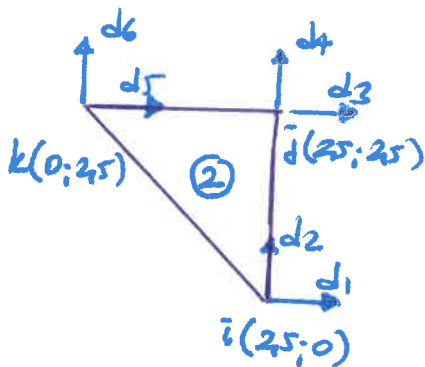
1 nolu eleman



|              |            |            |
|--------------|------------|------------|
| $a_i = 6.25$ | $a_j = 0$  | $a_k = 0$  |
| $b_i = -25$  | $b_j = 25$ | $b_k = 0$  |
| $c_i = -25$  | $c_j = 0$  | $c_k = 25$ |

$$\Delta = \frac{25 \times 25}{2} = 3.125$$

2 nolu eleman



|              |               |              |
|--------------|---------------|--------------|
| $a_i = 6.25$ | $a_j = -6.25$ | $a_k = 6.25$ |
| $b_i = 0$    | $b_j = +25$   | $b_k = -25$  |
| $c_i = -25$  | $c_j = 0$     | $c_k = 0$    |

$$\Delta = 3.125$$

1 node element

$$\{ \epsilon \} = \frac{1}{2 \times 3.125} \begin{bmatrix} -2.5 & 0 & 2.5 & 0 & 0 & 0 \\ 0 & -2.5 & 0 & 0 & 0 & 2.5 \\ -2.5 & -2.5 & 0 & 2.5 & 2.5 & 0 \end{bmatrix} \{ d \}$$

$$\{ \sigma \} = \frac{E}{6.25(1-0.3^2)} \begin{bmatrix} 1 & 0.3 & 0 \\ 0.3 & 1 & 0 \\ 0 & 0 & 0.35 \end{bmatrix} * \begin{bmatrix} -2.5 & 0 & 2.5 & 0 & 0 & 0 \\ 0 & -2.5 & 0 & 0 & 0 & 2.5 \\ -2.5 & -2.5 & 0 & 2.5 & 2.5 & 0 \end{bmatrix} \{ d \}$$

$$\{ \sigma \} = \frac{E}{6.25(1-0.3^2)} \begin{bmatrix} -2.5 & -0.75 & 2.5 & 0 & 0 & 0.75 \\ -0.75 & -2.5 & 0.75 & 0 & 0 & 2.5 \\ -0.875 & -0.875 & 0 & 0.875 & 0.875 & 0 \end{bmatrix} \{ d \}$$

$$[k_e] = \frac{Et}{4(\Delta^2)(1q^2)} \int \begin{bmatrix} -2.5 & 0 & -2.5 \\ 0 & -2.5 & -2.5 \\ 2.5 & 0 & 0 \\ 0 & 0 & 2.5 \\ 0 & 0 & 2.5 \\ 0 & 2.5 & 0 \end{bmatrix} * \begin{bmatrix} -2.5 & -0.75 & 2.5 & 0 & 0 & 0.75 \\ -0.75 & -2.5 & 0.75 & 0 & 0 & 2.5 \\ -0.875 & -0.875 & 0 & 0.875 & 0.875 & 0 \end{bmatrix} dx dy$$

$$\int dx dy = A$$

$$[k_e] = \frac{Et}{4 \times 3.125(1-0.3^2)} \begin{bmatrix} 8.438 & 4.0625 & -6.25 & -2.1875 & -2.188 & -1.875 \\ 4.063 & 8.4375 & -1.875 & -2.1875 & -2.188 & -6.25 \\ -6.25 & -1.875 & 6.25 & 0 & 0 & 1.875 \\ -2.188 & -2.1875 & 0 & 2.1875 & 2.1875 & 0 \\ -2.188 & -2.1875 & 0 & 2.1875 & 2.1875 & 0 \\ -1.875 & -6.25 & 1.875 & 0 & 0 & 6.25 \end{bmatrix}$$

$$[ce1] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[ce3] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$\{ \varepsilon \} = \frac{1}{2 \times 3/125} \begin{bmatrix} 0 & 0 & 2.5 & 0 & -2.5 & 0 \\ 0 & -2.5 & 0 & 2.5 & 0 & 0 \\ -2.5 & 0 & 2.5 & 2.5 & 0 & -2.5 \end{bmatrix} \{ d \}$$

$$\{ \sigma \} = \frac{E}{6.25/(1-0.3^2)} \begin{bmatrix} 1 & 0.3 & 0 \\ 0.3 & 1 & 0 \\ 0 & 0 & 0.35 \end{bmatrix} * \begin{bmatrix} 0 & 0 & 2.5 & 0 & -2.5 & 0 \\ 0 & -2.5 & 0 & 2.5 & 0 & 0 \\ -2.5 & 0 & 2.5 & 2.5 & 0 & -2.5 \end{bmatrix} \{ d \}$$

$$\{ \sigma \} = \frac{E}{6.25/(1-0.3^2)} \begin{bmatrix} 0 & -0.75 & 2.5 & 0.75 & -2.5 & 0 \\ 0 & -2.5 & 0.75 & 2.5 & -0.75 & 0 \\ -0.875 & 0 & 0.875 & 0.875 & 0 & -0.875 \end{bmatrix} \{ d \}$$

$$[k_2] = \frac{Et}{4(\Delta^2)/(1-0.3^2)} \begin{bmatrix} 0 & 0 & -2.5 \\ 0 & -2.5 & 0 \\ 2.5 & 0 & 2.5 \\ 0 & 2.5 & 2.5 \\ -2.5 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} * \begin{bmatrix} 0 & -0.75 & 2.5 & 0.75 & -2.5 & 0 \\ 0 & -2.5 & 0.75 & 2.5 & -0.75 & 0 \\ -0.875 & 0 & 0.875 & 0.875 & 0 & -0.875 \end{bmatrix} dx dy$$

$$[k_2] = \frac{Et}{4 \times 3/125(1-0.3^2)} \begin{bmatrix} 2.1875 & 0 & -2.188 & -2.188 & 0 & 2.1875 \\ 0 & 6.25 & -1.875 & -6.25 & 1.875 & 0 \\ -2.1875 & -1.875 & 8.438 & 4.0625 & -6.25 & -2.1875 \\ -2.1875 & -6.25 & 4.063 & 8.4375 & -1.875 & -2.1875 \\ 0 & 1.875 & -6.25 & -1.875 & 6.25 & 0 \\ 2.1875 & 0 & -2.188 & -2.188 & 0 & 2.1875 \end{bmatrix}$$

$$[ce2] = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[ce4] = \begin{bmatrix} 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$



[illegible]

[illegible]

$$[K_1] = \frac{Et}{125(10^3)} \begin{bmatrix} 8.4375 & 4.0625 & -6.25 & -2.1875 & 0 & 0 & -2.1875 & -1.875 & 0 & 0 & 0 & 0 \\ 4.0625 & 8.4375 & -1.875 & -2.1875 & 0 & 0 & -2.1875 & -6.25 & 0 & 0 & 0 & 0 \\ -6.25 & -1.875 & 6.25 & 0 & 0 & 0 & 0 & 1.875 & 0 & 0 & 0 & 0 \\ -2.1875 & -2.1875 & 0 & 2.1875 & 0 & 0 & 2.1875 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[K_2] = \frac{Et}{125(10^3)} \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 2.1875 & 0 & 6.25 & 0 & 0 & 2.1875 & -2.1875 & -6.25 & 0 & 0 \\ 0 & 0 & 0 & 6.25 & 0 & 0 & 1.875 & 0 & -1.875 & -2.1875 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1.875 & 6.25 & 0 & 6.25 & 0 & -6.25 & -1.875 & 0 & 0 \\ 0 & 0 & 2.1875 & 0 & 0 & 0 & 0 & 2.1875 & -2.1875 & -6.25 & -1.875 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[K] = [K_1] + [K_2] + [K_3] + [K_4]$$

$$[K][\Delta] = [Q]$$



بر یس حسابی

$$\{q\}_T = -\alpha_t (\Delta t) \left( \int [G]^T dV \right) \{D\}_T$$

$$[G]^T = \frac{1}{2\Delta} \begin{bmatrix} b_i & 0 & c_i \\ 0 & c_i & b_i \\ b_j & 0 & c_j \\ 0 & c_j & b_j \\ b_k & 0 & c_k \\ 0 & c_k & b_k \end{bmatrix}$$

$$\{D\}_T = \frac{\epsilon}{1-\nu} \begin{bmatrix} 1 \\ 1 \\ 0 \end{bmatrix}$$

$$dV = t \cdot \Delta \iint dx dy = \Delta$$

$$\{q\}_T = -\alpha_T (\Delta T) \frac{1}{2\Delta} \begin{bmatrix} b_i & 0 & c_i \\ 0 & c_i & b_i \\ b_j & 0 & c_j \\ 0 & c_j & b_j \\ b_k & 0 & c_k \\ 0 & c_k & b_k \end{bmatrix} \Delta t \cdot \frac{\epsilon}{1-\nu} \begin{bmatrix} 1 \\ 1 \\ 0 \end{bmatrix}$$

$$\{q\}_T = -\alpha_T (\Delta T) \frac{t}{2(1-\nu)} \begin{bmatrix} b_i \\ c_i \\ b_j \\ c_j \\ b_k \\ c_k \end{bmatrix}$$

$$\{q\}_T \quad \{Q\} = -\sum q$$

$$\{q\}_T = -0,00012 \times (-30) \frac{21 \times 10^3 \times 0,03}{2(1-0,30)}$$

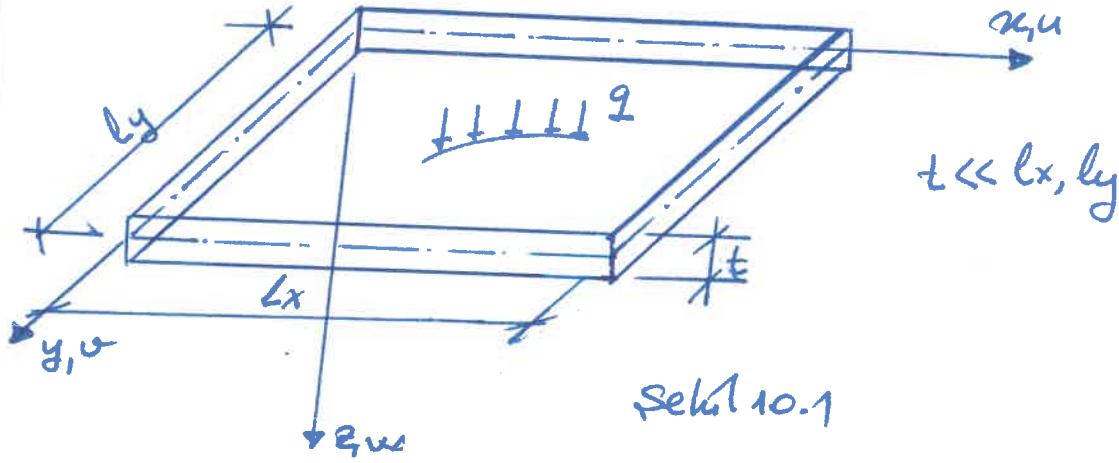
$$\begin{bmatrix} -25 \\ -25 \\ 25 \\ 0 \\ 0 \\ 25 \end{bmatrix} =$$

$$\begin{bmatrix} -40500 \\ -40500 \\ 40500 \\ 0 \\ 0 \\ 40500 \end{bmatrix}$$

$$\begin{aligned} Q_3 &= 40500 \\ Q_4 &= 40500 \\ Q_5 &= -40500 \\ Q_6 &= 0 \\ Q_9 &= 0 \\ Q_{10} &= -40500 \end{aligned}$$

#### 10.4) İnce Plâklar

Her üç boyutu yanında kalınlığı çok küçük olan ve yükleri orta düzlemine dik olan taşıyıcı elemana plâk denir (Şekil 10.1)



##### 10.4.1) İnce plâklarda yapılan kabûller

a) Plâk geometrisi açısından yapılan kabûller;

- 1) Plâk kalınlığı diğer üç boyutu yanında oldukça küçüktür.
- 2) Plâğa etkiyen yükler plâk orta düzlemine diktir.
- 3) Yer değiştirmeler plâk kalınlığına oranla çok küçüktür.
- 4) Plâk kalınlığına bütün orta noktadaki geometrik yeri yine bir düzlemdir.

b) Malzeme açısından yapılan kabûller;

1) Malzeme homojendir.

2) Malzeme her doğrultuda aynı davranışı gösteren izotrop bir malzemedir.

3) Malzeme lineer elastiktir ve Hooke Kanunu geçerlidir.

c) Hesaplar açısından yapılan kabûller;

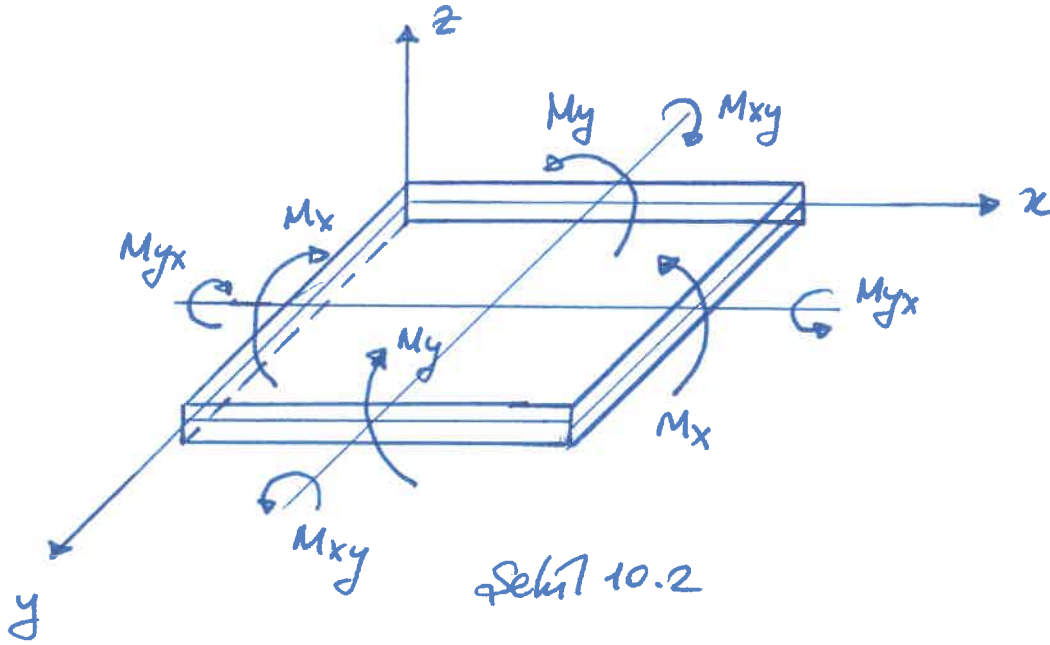
1) Plâk orta düzlemine dik doğrultudaki başka bir değişik plâk kalınlığı boyunca meydana gelen gerilmeler ( $\sigma_z$ ) çok küçük olduğundan ihmal edilebilir (Düzlem Gerilme Hali).

2) Plâk kalınlığı boyunca oluşan yer değiştirmelerinde plâk kalınlığında bir değişim olmaz.

3) Plâk kesitinin orta düzleminde şekil değişikliği meydana gelmeyecektir.

4) Bernoulli Hipotezi' geçerli olan Düzlem kesitler şekil değiştikdikten sonra da düzlem kalırlar ve eğrilme eksenine diktirler.

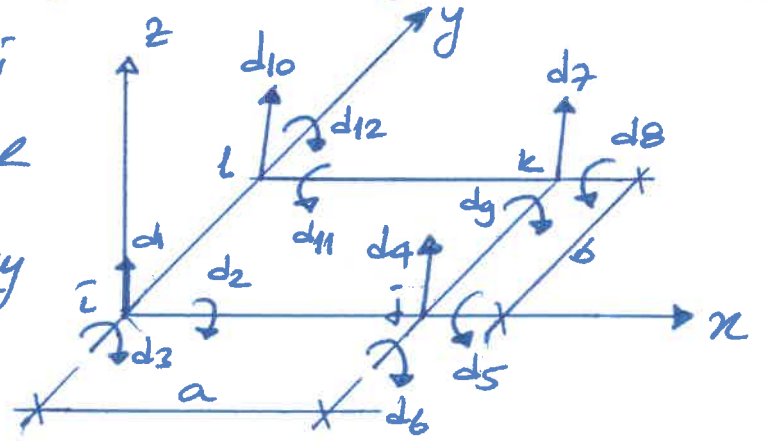
Şekil 10.2'de plâk elemanında meydana gelen kesit tensörleri' verilmiştir.



Şekil 10.2

#### 10.4.2) İnce Plâklerin Dikdörtgen Şekil Fonksiyonları ile Hesabı

Şekil 10.3'de de verildiği gibi dikdörtgen plâk elemanın her düğüm noktasında bir dikey yer değiştirme ve her iki doğrultuda dönme olmak üzere toplam 12 serbestliği bulunmaktadır.



Şekil 10.3



Doğru noktası serbestlik kime göre eleman deplasman parametreleri;

$$\{d\}^T = [w_i, \theta_{x_i}, \theta_{y_i}, w_j, \theta_{x_j}, \theta_{y_j}, w_k, \theta_{x_k}, \theta_{y_k}, w_l, \theta_{x_l}, \theta_{y_l}]$$

$$= [d_1, d_2, d_3; d_4, d_5, d_6; d_7, d_8, d_9; d_{10}, d_{11}, d_{12}]$$

Pascal polinomlarından faydalanarak deplasman fonksiyonu seçilecek;

$$w = a_1 + a_2x + a_3y + a_4x^2 + a_5xy + a_6y^2 + a_7x^3 + a_8x^2y + a_9xy^2$$

$$+ a_{10}y^3 + a_{11}x^3y + a_{12}xy^3$$

olarak belirlenir. Deplasman fonksiyonundan faydalanarak  $x$  ve  $y$  doğrultülerindeki dönmeles;

$$\theta_x = \frac{\partial w}{\partial y} = a_3 + a_5x + 2a_6y + a_8x^2 + 2a_9xy + 3a_{10}y^2 + a_{11}x^3 + 3a_{12}y^2$$

$$\theta_y = -\frac{\partial w}{\partial x} = -a_2 - 2a_4x - a_5y - 3a_7x^2 - 2a_8xy - a_9y^2 - 3a_{11}x^2y - a_{12}y^3$$

şeklinde ekle edilir.

### 10.4.3) Şekil Farklılıklarının Belirlenmesi

Deplasman fonksiyonu  $w$ , önceden de verildiği gibi

$w = [\phi(x,y)]\{a\}$  şeklinde yazılabilir.

$$w = [1, x, y, x^2, xy, y^2, x^3, x^2y, xy^2, y^3, x^3y, xy^3]$$

$$[\phi(x,y)] = [1, x, y, x^2, xy, y^2, x^3, x^2y, xy^2, y^3, x^3y, xy^3]$$

$$\begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ \vdots \\ a_8 \\ a_9 \\ a_{10} \\ a_{11} \\ a_{12} \end{Bmatrix}$$

veya

$$w = [N]\{d\} = [N_1, N_2, N_3, \dots, N_9, N_{10}, N_{11}, N_{12}]$$

$$\begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ \vdots \\ a_{12} \end{Bmatrix}$$

yazılabilir.

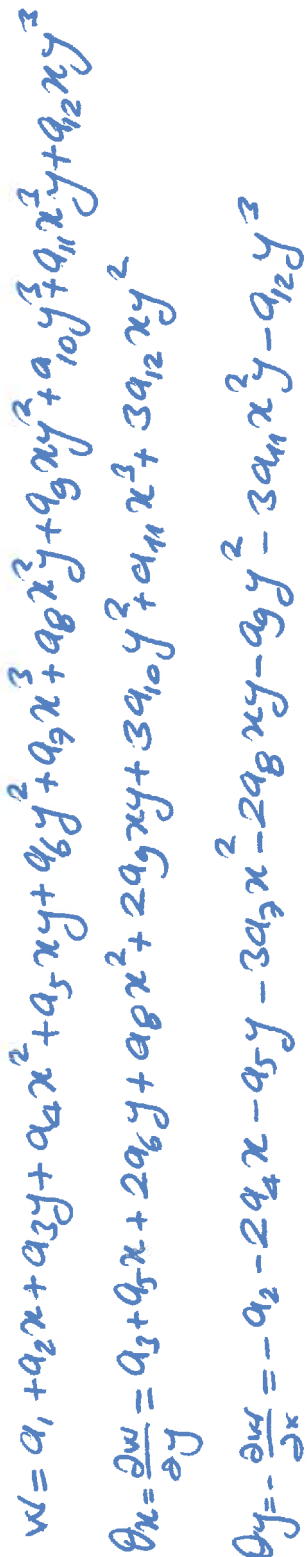
$w = [\phi(x,y)]\{a\}$  de ve  $\{a\} = [B]\{d\}$  olduğundan;

$$w = \underbrace{[\phi(x,y)][B]}_{[N]}\{d\}$$

$[N] = [\phi(x,y)][B] \rightarrow$  şekil fonksiyonu verir.

$[A]$  bağ matrisini ve dolayısıyla  $[B]$  matrisini elde etmek için düğüm noktaları koordinatlarından;

$$i(0,0), j(a,0), k(a,b), l(0,b)$$



$$\partial y = -\frac{x}{2} = -a_2 - 2a_4x - a_5y - 3a_3x^2 - 2a_8xy - a_9y^2 - 3a_{11}x^3 - a_{12}y^3$$

$$\partial x = \frac{\partial w}{\partial y} = a_3 + a_5 x + 2a_6 y + 2a_7 x^2 + 2a_8 xy + 3a_9 y^2 + a_{10} x^3 + 3a_{11} x^2 + 3a_{12} xy^2$$

$$[A] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \hline 1 & a & 0 & a^2 & 0 & 0 & a^3 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & a & 0 & 0 & a^2 & 0 & 0 & a^3 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & -2a & 0 & 0 & -3a^2 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \hline 1 & a & b & a^2 & ab & b^2 & a^3 & a^2b & ab^2 & b^3 & a^3b & a^2b^2 & ab^3 & 3ab^2 & -b^3 & 3ab^2 & -b^3 & 3ab^2 & -b^3 & 3ab^2 \\ 0 & 0 & 1 & 0 & a & 2b & 0 & a^2 & 2ab & b^2 & a^2b & ab^2 & a^3 & 3ab^2 & -b^3 & 3ab^2 & -b^3 & 3ab^2 & -b^3 & 3ab^2 \\ 0 & -1 & 0 & -2a & -b & 0 & -3a^2 & -2ab & -b^2 & 0 & -3a^2b & -3ab^2 & -b^3 & -3ab^2 & -b^3 & -3ab^2 & -b^3 & -3ab^2 & -b^3 & -b^3 \\ \hline 1 & 0 & b & 0 & 0 & b^2 & 0 & 0 & 0 & b^2 & 0 & 0 & 0 & b^2 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 2b & 0 & 0 & 0 & 3b^2 & 0 & 0 & 0 & 3b^2 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & -b & 0 & 0 & 0 & -b^2 & 0 & 0 & 0 & 0 & -b^2 & 0 & 0 & 0 & 0 & 0 & -b^3 \end{bmatrix}$$

$$[B] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & -1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ -3/a^2 & 0 & 2/a & 3/a^2 & 0 & 1/a & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ -1/ab & -1/a & 1/b & 1/ab & 1/a & 0 & -1/ab & 0 & 0 & 1/ab & 0 & 0 & -1/b \\ -3/b^2 & -2/b & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 3/b^2 & -1/b & 0 & 0 \\ 2/a^3 & 0 & -1/a^2 & -2/a^3 & 0 & -1/a^2 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 3/a^2b & 0 & -2/ab & -3/a^2b & 0 & -1/ab & 3/a^2b & 0 & 1/ab & -3/a^2b & 0 & 2/ab & 0 \\ 3/ab^2 & 2/ab & 0 & -3/ab^2 & -2/ab & 0 & 3/ab^2 & -1/ab & 0 & -3/ab^2 & 1/ab & 0 & 0 \\ 2/b^3 & 1/b^2 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -2/b^3 & 1/b^2 & 0 & 0 \\ -2/a^3b & 0 & 1/a^2b & 2/a^3b & 0 & 1/a^2b & -2/a^3b & 0 & -1/a^2b & 2/a^3b & 0 & -1/a^2b & 0 \\ -2/ab^3 & -1/ab^2 & 0 & 2/ab^3 & 1/ab^2 & 0 & -2/ab^3 & 1/ab^2 & 0 & 2/ab^3 & -1/ab^2 & 0 & 0 \end{bmatrix}$$

$[B] =$

$$\underline{[N] = [\phi(x,y)] [B]} \quad \text{beginnen mit};$$

$$N_1 = \frac{2x^3}{a^3} - \frac{3x^2}{a^2} - \frac{3y^2}{b^2} + \frac{2y^3}{b^3} + \frac{3xy^2}{ab^2} + \frac{3x^2y}{a^2b} - \frac{2xy^3}{ab^3} - \frac{2x^3y}{a^3b} - \frac{xy}{ab} + 1$$

$$N_2 = y - (2y^2)/b + \frac{y^3}{b^2} - \frac{xy}{a} + \frac{2xy^2}{ab} - \frac{xy^3}{ab^2}$$

$$N_3 = \frac{2x^2}{a} - x - \frac{x^3}{a^2} + \frac{xy}{b} - \frac{2x^2y}{ab} + \frac{x^3y}{a^2b}$$

$$N_4 = \frac{3x^2}{a^2} - \frac{2x^3}{a^3} - \frac{3xy^2}{ab^2} - \frac{3x^2y}{a^2b} + \frac{2xy^3}{ab^3} + \frac{2x^3y}{a^3b} + \frac{xy}{ab}$$

$$N_5 = \frac{xy}{a} - \frac{2xy^2}{ab} + \frac{xy^3}{ab^2}$$

$$N_6 = \frac{x^2}{a} - \frac{x^3}{a^2} - \frac{x^2y}{ab} + \frac{x^3y}{a^2b}$$

$$N_7 = \frac{3xy^2}{ab^2} + \frac{3x^2y}{a^2b} - \frac{2xy^3}{ab^3} - \frac{2x^3y}{a^3b} - \frac{xy}{ab}$$

$$N_8 = \frac{xy^2}{ab^2} - \frac{xy^2}{ab}$$

$$N_9 = -\frac{x^3y}{a^2b} + \frac{x^2y}{ab}$$

$$N_{10} = \frac{3y^2}{b^2} - \frac{2y^3}{b^3} - \frac{3xy^2}{ab^2} - \frac{3x^2y}{a^2b} + \frac{2xy^3}{ab^3} + \frac{2x^3y}{a^3b} + \frac{xy}{ab}$$

$$N_{11} = \frac{y^3}{b^2} - \frac{y^2}{b} + \frac{xy^2}{ab} - \frac{xy^3}{ab^2}$$

$$N_{12} = -\frac{x^3y}{a^2b} + \frac{2x^2y}{ab} - \frac{xy}{b}$$

$\xi = \frac{x}{a}$  i  $\eta = \frac{y}{b}$  dersek ve gerekli düzenlemeleri yaparsak şekil fonksiyonları;

$$N_1 = 1 - \xi\eta - (3-2\xi)(1-\eta)\xi^2 - (1-\xi)(3-2\eta)\eta^2$$

$$N_2 = [(1-\xi)(1-\eta)^2\eta]b$$

$$N_3 = [-\xi(1-\xi)^2(1-\eta)]a$$

---


$$N_4 = (3-2\xi)\xi^2(1-\eta) + \xi\eta(1-\eta)(1-2\eta)$$

$$N_5 = [\xi\eta(1-\eta)^2]b$$

$$N_6 = [(1-\xi)\xi^2(1-\eta)]a$$

---


$$N_7 = (3-2\xi)\xi^2\eta - \xi\eta(1-\eta)(1-2\eta)$$

$$N_8 = [-\xi(1-\eta)\eta^2]b$$

$$N_9 = [(-1-\xi)\xi^2\eta]a$$

---


$$N_{10} = (1-\xi)(3-2\eta)\eta^2 + \xi(1-\xi)(1-2\xi)\eta$$

$$N_{11} = [-(1-\xi)(1-\eta)\eta^2]b$$

$$N_{12} = [-\xi(1-\xi)^2\eta]a$$

şeklinde elde edilir.

## Sekül Değiştirme - Yer Değiştirme Bağlantıları

İnce plakalarda sekül değiştirme - yer değiştirme bağlantısı;

$$\{\varepsilon\} = \begin{Bmatrix} -\frac{\partial^2 w}{\partial x^2} \\ -\frac{\partial^2 w}{\partial y^2} \\ -2 \frac{\partial^2 w}{\partial x \partial y} \end{Bmatrix}$$

sekünderdir. Diğer yandan  $w = [N] \{d\}$   
o yüzden bağlı;

$$\textcircled{1} \quad \{\varepsilon\} = \begin{Bmatrix} -\frac{\partial^2}{\partial x^2} \\ -\frac{\partial^2}{\partial y^2} \\ -2 \frac{\partial^2}{\partial x \partial y} \end{Bmatrix} [N_i] \{d\} = [\Delta] [N_i] \{d\}$$

$$\{\varepsilon\} = \begin{bmatrix} -N_{1,xx} & -N_{2,xx} & -N_{3,xx} & \dots & -N_{12,xx} \\ -N_{1,yy} & -N_{2,yy} & -N_{3,yy} & \dots & -N_{12,yy} \\ -2N_{1,xy} & -2N_{2,xy} & -2N_{3,xy} & \dots & -2N_{12,xy} \end{bmatrix} \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ \vdots \\ d_{12} \end{Bmatrix}$$

$[\Delta][N_i]$

## Diferansiyel Bağlantılar

$$\frac{\partial}{\partial x} = \frac{1}{a} \frac{\partial}{\partial \xi}$$

$$\frac{\partial}{\partial y} = \frac{1}{b} \frac{\partial}{\partial \eta}$$

$$\frac{\partial^2}{\partial x^2} = \frac{1}{a^2} \frac{\partial^2}{\partial \xi^2}$$

$$\frac{\partial^2}{\partial y^2} = \frac{1}{b^2} \frac{\partial^2}{\partial \eta^2}$$

$$\frac{\partial^2}{\partial x \partial y} = \frac{1}{ab} \frac{\partial^2}{\partial \xi \partial \eta}$$

$$-N_{1,xx} = -\frac{\partial^2 N_1}{\partial x^2} = -\frac{1}{a^2} \frac{\partial^2 N_1}{\partial \xi^2} = (1-2\xi)(1-\eta) \frac{6}{a^2}$$

$$-N_{1,yy} = -\frac{\partial^2 N_1}{\partial y^2} = -\frac{1}{b^2} \frac{\partial^2 N_1}{\partial \eta^2} = (1-\xi)(1-2\eta) \frac{6}{b^2}$$

$$-2N_{1,xy} = -2 \frac{\partial^2 N_1}{\partial x \partial y} = -\frac{2}{ab} \frac{\partial^2 N_1}{\partial \xi \partial \eta}$$

$$= \frac{2}{ab} [1 - 6\xi(1-\xi) - 6\eta(1-\eta)]$$



veya bilindiği gibi  $\{\varepsilon\} = [F][B]\{d\}$  idi. Bu bağıntıdan faydalanarak da  $\{\varepsilon\}$  bulunabilir. Türev matrisi olan  $[F]$ ;

$$-\frac{\partial^2 w}{\partial x^2} = -2a_4 - 6a_7x - 2a_8y - 6a_{11}xy$$

$$-\frac{\partial^2 w}{\partial y^2} = -2a_6 - 2a_9x - 6a_{10}y - 6a_{12}xy$$

$$-2\frac{\partial^2 w}{\partial x \partial y} = -2a_5 - 4a_8x - 4a_9y - 6a_{11}x^2 - 6a_{12}y^2$$

$$[F] = \begin{bmatrix} 0 & 0 & 0 & -2 & 0 & 0 & -6x & -2y & 0 & 0 & -6xy & 0 \\ 0 & 0 & 0 & 0 & 0 & -2 & 0 & 0 & -2x & -6y & 0 & -6xy \\ 0 & 0 & 0 & 0 & -2 & 0 & 0 & -4x & -4y & 0 & -6x^2 & -6y^2 \end{bmatrix}$$

şeklinde elde edilir. Bu durumda;

$$\textcircled{2} \quad \{\varepsilon\} = [F][B]\{d\}$$

bağıntı ile hesap yapılsa önceden bulunan  $\{\varepsilon\}$

tablosu elde edilmiş olur.

Örneğin 1. sıtım denklemleri;

$$G_{11} = \frac{6}{a^2} - \frac{12x}{a^3} - \frac{6y}{a^2b} + \frac{12xy}{a^3b}$$

$$= \frac{6}{a^2} \left( 1 - \frac{2x}{a} - \frac{y}{b} + \frac{2xy}{ab} \right)$$

$$= \frac{6}{a^2} (1 - 2\xi - \eta + 2\xi\eta)$$

$$= \frac{6}{a^2} (1 - 2\xi)(1 - \eta)$$

$$G_{21} = \frac{6}{b^2} - \frac{12y}{b^3} - \frac{6x}{ab^2} + \frac{12xy}{ab^3}$$

$$= \frac{6}{b^2} \left( 1 - \frac{2y}{b} - \frac{x}{a} + \frac{2xy}{ab} \right)$$

$$= \frac{6}{b^2} (1 - 2\eta - \xi + 2\xi\eta)$$

$$= \frac{6}{b^2} (1 - \xi)(1 - 2\eta)$$

$$G_{31} = \frac{2}{ab} - \frac{12x}{a^2b} - \frac{12y}{ab^2} + \frac{12x^2}{a^3b} + \frac{12y^2}{ab^3}$$

$$= \frac{2}{ab} \left( 1 - \frac{x}{a} + \frac{6y}{b} + \frac{6x^2}{a^2} + \frac{6y^2}{b^2} \right)$$

$$= \frac{2}{ab} (1 - \xi + 6\eta + 6\xi^2 + 6\eta^2)$$

$$= \frac{2}{ab} [1 - 6\xi(1 - \xi) - 6\eta(1 - \eta)]$$

$$([F] * [B])^T =$$

veya

$$([A] * [N_i])^T =$$

|                                  |                                  |                                              |
|----------------------------------|----------------------------------|----------------------------------------------|
| $(1-2\xi)(1-\eta)\frac{6}{a^2}$  | $(1-\xi)(1-2\eta)\frac{6}{b^2}$  | $[1-6\xi(1-\xi)-6\eta(1-\eta)]\frac{2}{ab}$  |
| 0                                | $(1-\xi)(2-3\eta)\frac{2}{b}$    | $(1-3\eta)(1-\eta)\frac{2}{a}$               |
| $-(2-3\xi)(1-\eta)\frac{2}{a}$   | 0                                | $-(1-3\xi)(1-\xi)\frac{2}{b}$                |
| $-(1-2\xi)(1-\eta)\frac{6}{a^2}$ | $\xi(1-2\eta)\frac{6}{b^2}$      | $[-1+6\xi(1-\xi)+6\eta(1-\eta)]\frac{2}{ab}$ |
| 0                                | $\xi(2-3\eta)\frac{2}{b}$        | $(1-3\eta)(1-\eta)\frac{2}{a}$               |
| $-(1-3\xi)(1-\eta)\frac{2}{a}$   | 0                                | $\xi(2-3\xi)\frac{2}{b}$                     |
| $-(1-2\xi)\eta\frac{6}{a^2}$     | $-\xi(1-2\eta)\frac{6}{b^2}$     | $[1-6\xi(1-\xi)-6\eta(1-\eta)]\frac{2}{ab}$  |
| 0                                | $\xi(1-3\eta)\frac{2}{b}$        | $\eta(2-3\eta)\frac{2}{a}$                   |
| $-(1-3\xi)\eta\frac{2}{a}$       | 0                                | $-\xi(2-3\xi)\frac{2}{b}$                    |
| $(1-2\xi)\eta\frac{6}{a^2}$      | $-(1-\xi)(1-2\eta)\frac{6}{b^2}$ | $[-1+6\xi(1-\xi)+6\eta(1-\eta)]\frac{2}{ab}$ |
| 0                                | $(1-\xi)(1-3\eta)\frac{2}{b}$    | $-\eta(2-3\eta)\frac{2}{a}$                  |
| $-(2-3\xi)\eta\frac{2}{a}$       | 0                                | $(1-3\xi)(1-\xi)\frac{2}{b}$                 |

## Geçirne Bağıntıları

Bazılangıç selü? değışikliklerle bazılangıç geçirnelele lıkıkate alındığında geçirne bağıntıları;

$$\{G\} = \{M\} = [D] (\{\varepsilon\} - \{\varepsilon_0\}) + \{G_0\} \quad \text{' dir. Bazılangıç}$$

durumunun lıkıkate alınımadığı durumlarda bağıntıları;

$$\{G\} = \{M\} = [D] \{\varepsilon\} \quad \text{olacaktır. Materyale homojen}$$

ve izotrop dir;

$$[D] = \frac{Et^3}{12(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \quad \text{olacaktır. Bu durumda}$$

geçirne bağıntıları;

$$\{G\} = \begin{Bmatrix} M_x \\ M_y \\ M_z \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} -\frac{\partial^2 w}{\partial x^2} \\ -\frac{\partial^2 w}{\partial y^2} \\ -2\frac{\partial^2 w}{\partial x \partial y} \end{Bmatrix}$$

halini olacaktır.

Gesirne bağıntısının koordinatları 2 şekilde yazabiliriz;

$$(1) \quad \{\sigma\} = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \end{Bmatrix} = [D][\epsilon][N_r]\{\delta\}$$

$$= \frac{Et^3}{12(1-\nu^2)} \underbrace{\begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} -N_{1,xx} & -N_{2,xx} & \dots & -N_{12,xx} \\ -N_{1,yy} & -N_{2,yy} & \dots & -N_{12,yy} \\ -2N_{1,xy} & -2N_{2,xy} & \dots & -2N_{12,xy} \end{bmatrix}}_{[G]} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \\ \vdots \\ \delta_{12} \end{Bmatrix}$$

$$= [D][G]\{\delta\}$$

$$(2) \quad \{\sigma\} = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \end{Bmatrix} = [D] \underbrace{[F][B]}_{[G]} \{\delta\}$$

$$= [D][G]\{\delta\}$$

Bu durumda elde edilen  $[D][G]$  matrisi  
aşağıda verdiğimiz  $[D][G]$  matrisinden hemen sonra  
gesirne matrisi  $\{\sigma\}$  yazılmaktadır.

|                                                                    |                                                                    |                                                  |
|--------------------------------------------------------------------|--------------------------------------------------------------------|--------------------------------------------------|
| $\frac{6}{a^2}(1-2\xi)(1-\eta) + \frac{6\nu}{b^2}(1-\xi)(1-2\eta)$ | $\frac{6\nu}{a^2}(1-2\xi)(1-\eta) + \frac{6}{b^2}(1-\xi)(1-2\eta)$ | $\frac{1-\nu}{ab}[1-6\xi(1-\xi)-6\eta(1-\eta)]$  |
| $\frac{2\nu}{b}(1-\xi)(2-3\eta)$                                   | $\frac{2}{b}(1-\xi)(2-3\eta)$                                      | $\frac{1-\nu}{a}(1-3\eta)(1-\eta)$               |
| $-\frac{2}{a}(2-3\xi)(1-\eta)$                                     | $-\frac{2\nu}{a}(2-3\xi)(1-\eta)$                                  | $-\frac{1-\nu}{b}(1-3\xi)(1-\xi)$                |
| $-\frac{6}{a^2}(1-2\xi)(1-\eta) + \frac{6\nu}{b^2}(1-2\eta)\xi$    | $-\frac{6\nu}{a^2}(1-2\xi)(1-\eta) + \frac{6}{b^2}(1-2\eta)\xi$    | $\frac{1-\nu}{ab}[-1+6\xi(1-\xi)+6\eta(1-\eta)]$ |
| $\frac{2\nu}{b}(2-3\eta)\xi$                                       | $\frac{2}{b}(2-3\eta)\xi$                                          | $-\frac{1-\nu}{a}(1-3\eta)(1-\eta)$              |
| $-\frac{2}{a}(1-3\xi)(1-\eta)$                                     | $-\frac{2\nu}{a}(1-3\xi)(1-\eta)$                                  | $\frac{1-\nu}{b}(2-3\xi)\xi$                     |
| $-\frac{6}{a^2}(1-2\xi)\eta - \frac{6\nu}{b^2}(1-2\eta)\xi$        | $-\frac{6\nu}{a^2}(1-2\xi)\eta - \frac{6}{b^2}(1-2\eta)\xi$        | $\frac{1-\nu}{ab}[1-6\xi(1-\xi)-6\eta(1-\eta)]$  |
| $\frac{2\nu}{b}(1-3\eta)\xi$                                       | $\frac{2}{b}(1-3\eta)\xi$                                          | $\frac{1-\nu}{a}(2-3\eta)\eta$                   |
| $-\frac{2}{a}(1-3\xi)\eta$                                         | $-\frac{2\nu}{a}(1-3\xi)\eta$                                      | $-\frac{1-\nu}{a}(2-3\xi)\xi$                    |
| $\frac{6}{a^2}(1-2\xi)\eta - \frac{6\nu}{b^2}(1-\xi)(1-2\eta)$     | $\frac{6\nu}{a^2}(1-2\xi)\eta - \frac{6}{b^2}(1-\xi)(1-2\eta)$     | $\frac{1-\nu}{ab}[-1+6\xi(1-\xi)+6\eta(1-\eta)]$ |
| $\frac{2\nu}{b}(1-\xi)(1-3\eta)$                                   | $\frac{2}{b}(1-\xi)(1-3\eta)$                                      | $\frac{1-\nu}{a}(2-3\eta)\eta$                   |
| $-\frac{2}{a}(2-3\xi)\eta$                                         | $-\frac{2\nu}{a}(2-3\xi)\eta$                                      | $\frac{1-\nu}{b}(1-3\xi)(1-\xi)$                 |

$([D][F][B])^T =$

veya

$([D][\Delta][N])^T =$

$$\{\sigma\} = \begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

|                                                                    |                                                                    |                                                  |
|--------------------------------------------------------------------|--------------------------------------------------------------------|--------------------------------------------------|
| $\frac{6}{a^2}(1-2\xi)(1-\eta) + \frac{6\nu}{b^2}(1-\xi)(1-2\eta)$ | $\frac{6\nu}{a^2}(1-2\xi)(1-\eta) + \frac{6}{b^2}(1-\xi)(1-2\eta)$ | $\frac{1-\nu}{ab}[1-6\xi(1-\xi)-6\eta(1-\eta)]$  |
| $\frac{2\nu}{b}(1-\xi)(2-3\eta)$                                   | $\frac{2}{b}(1-\xi)(2-3\eta)$                                      | $\frac{1-\nu}{a}(1-3\eta)(1-\eta)$               |
| $-\frac{2}{a}(2-3\xi)(1-\eta)$                                     | $-\frac{2\nu}{a}(2-3\xi)(1-\eta)$                                  | $-\frac{1-\nu}{b}(1-3\xi)(1-\xi)$                |
| $-\frac{6}{a^2}(1-2\xi)(1-\eta) + \frac{6\nu}{b^2}(1-2\eta)\xi$    | $-\frac{6\nu}{a^2}(1-2\xi)(1-\eta) + \frac{6}{b^2}(1-2\eta)\xi$    | $\frac{1-\nu}{ab}[-1+6\xi(1-\xi)+6\eta(1-\eta)]$ |
| $\frac{2\nu}{b}(2-3\eta)\xi$                                       | $\frac{2}{b}(2-3\eta)\xi$                                          | $\frac{1-\nu}{a}(1-3\eta)(1-\eta)$               |
| $-\frac{2}{a}(1-3\xi)(1-\eta)$                                     | $-\frac{2\nu}{a}(1-3\xi)(1-\eta)$                                  | $\frac{1-\nu}{b}(2-3\xi)\xi$                     |
| $-\frac{6}{a^2}(1-2\xi)\eta - \frac{6\nu}{b^2}(1-2\eta)\xi$        | $-\frac{6\nu}{a^2}(1-2\xi)\eta - \frac{6}{b^2}(1-2\eta)\xi$        | $\frac{1-\nu}{ab}[1-6\xi(1-\xi)-6\eta(1-\eta)]$  |
| $\frac{2\nu}{b}(1-3\eta)\xi$                                       | $\frac{2}{b}(1-3\eta)\xi$                                          | $\frac{1-\nu}{a}(2-3\eta)\eta$                   |
| $-\frac{2}{a}(1-3\xi)\eta$                                         | $-\frac{2\nu}{a}(1-3\xi)\eta$                                      | $-\frac{1-\nu}{a}(2-3\xi)\xi$                    |
| $\frac{6}{a^2}(1-2\xi)\eta - \frac{6\nu}{b^2}(1-\xi)(1-2\eta)$     | $\frac{6\nu}{a^2}(1-2\xi)\eta - \frac{6}{b^2}(1-\xi)(1-2\eta)$     | $\frac{1-\nu}{ab}[-1+6\xi(1-\xi)+6\eta(1-\eta)]$ |
| $\frac{2\nu}{b}(1-\xi)(1-3\eta)$                                   | $\frac{2}{b}(1-\xi)(1-3\eta)$                                      | $\frac{1-\nu}{a}(2-3\eta)\eta$                   |
| $-\frac{2}{a}(2-3\xi)\eta$                                         | $-\frac{2\nu}{a}(2-3\xi)\eta$                                      | $\frac{1-\nu}{b}(1-3\xi)(1-\xi)$                 |



## Eleman Rijitlik Matrisinin Hesabı

Seçilen dikdörtgen elemanın rijitlik matrisi de şu şekilde hesaplanabilmektedir. Buralardan ilki;

$$① [k_e] = \int_0^a \int_0^b [B]^T [F]^T [D] [F] [B] dx dy$$

şeklinde dir. Rijitlik matrisinin ikinci hesaplanış şekli

$$② [k_e] = \int_0^a \int_0^b [\Delta N]^T [D] [\Delta N] dx dy \quad \text{'dir.}$$

Burada  $\int_0^a \int_0^b dx dy = ab d\xi d\eta$  'dir. Bu durumda bağıntı;

$$[k_e] = ab \int_0^1 \int_0^1 [\Delta N]^T [D] [\Delta N] d\xi d\eta \quad \text{dur.}$$

Bağıntıyı açık olarak yazarsak;

$$[k_e] = \frac{Et^3}{12(1-\nu^2)} \frac{1}{ab} \int_0^1 \int_0^1 \begin{bmatrix} -N_{1,xx} & -N_{1,yy} \\ -N_{2,xx} & -N_{2,yy} \\ -N_{3,xx} & -N_{3,yy} \\ \vdots & \vdots \\ -N_{12,xx} & -N_{12,yy} \end{bmatrix} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} -N_{1,xx} & -N_{2,xx} & \dots & -N_{12,xx} \\ -N_{1,yy} & -N_{2,yy} & \dots & -N_{12,yy} \\ -2N_{1,xy} & -2N_{2,xy} & \dots & -2N_{12,xy} \end{bmatrix} d\xi d\eta$$

Her iki bağıntıya göre hesaplanan eleman rijitlik matrisi aşağıda yer almaktadır.



## Eleman Yük Matrisi

Plâga etkiyen statik bir yük de; bu durumda yük, tekil, yaygınlı veya ağırlıklı yük olabilir. Ağırlıklı yük sabuk konusunda verildiği gibi ele alınır ve yük hangi düğüm noktalarına arasında ve diğer sabuk eleman gibi ele alınır ve yükün düğüm noktalarına etkimesi sağlanır. Yük tekil bir yük de ilgili düğüm noktasında yükün yönüne göre dağılımı yapılır. Yük yaygınlı bir halde de;

$$\{q_e\} = \int_0^a \int_0^b [N]^T \{p\} dx dy$$

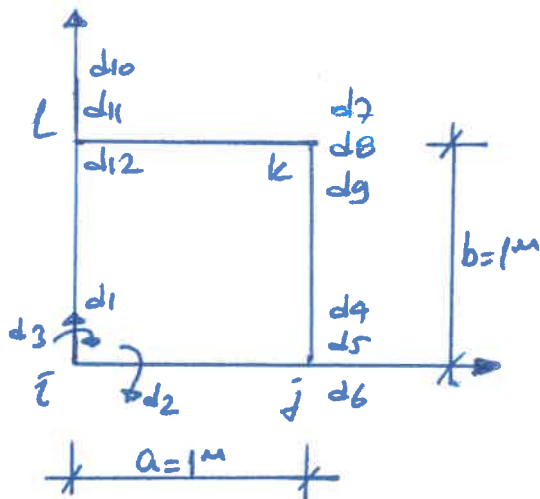
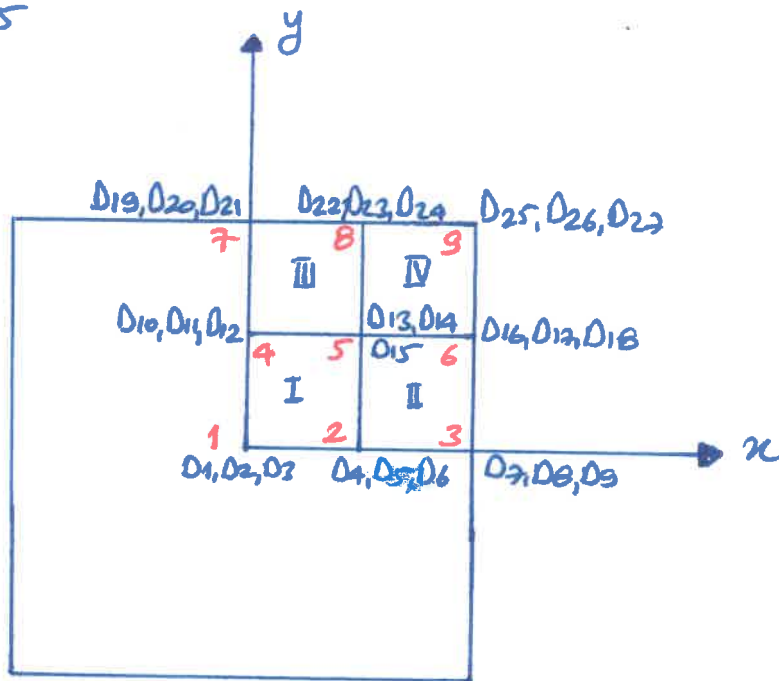
$$\{q_e\} = ab \int_0^1 \int_0^1 [N_1 \ N_2 \ \dots \ N_{12}]^T \{p\} d\xi d\eta$$

bağıntısı ile hesaplanabilir.

Şimdiye kadar 4 kez anket yapılmış ve sonuçları değerlendirilmiştir.

$$E = 21 \cdot 10^7 \text{ kN/m}^2$$

A diagram of a square plate with a central hole. The plate is represented by a square with a cross-hatched border. A coordinate system is centered on the plate, with the  $x$ -axis pointing to the right and the  $y$ -axis pointing upwards. The dimensions of the plate are indicated as  $4m$  on both the horizontal and vertical sides.



$$\xi = \frac{\kappa}{a}$$

$$\eta = \frac{y}{b}$$

1) Planen RgHk materinin dukturalınam

$$\beta = \frac{b}{a} = 1, \quad \frac{1}{\beta} = \frac{a}{b} = 1; \quad \nu = 0,15; \quad E = 21 \cdot 10^7 \text{ kN/m}^2$$

$d = 15 \text{ cm}$

## 2) Eleman Yok Matrisi'

$$\{q_e\} = \iint [N]^T \{P\} dx dy$$

$$\{q_e\} = ab \int_0^1 \int_0^1 [N_1, N_2, \dots, N_{12}]^T \{P\} d\xi d\eta$$

$$\{q_e\} = pab \int_0^1 \int_0^1 \begin{bmatrix} N_1 \\ N_2 \\ N_3 \\ \vdots \\ N_{12} \end{bmatrix} d\xi d\eta = \begin{bmatrix} q_{e1} \\ q_{e2} \\ q_{e3} \\ \vdots \\ q_{e12} \end{bmatrix}$$

Bu integreynden;

$$\begin{aligned} \{q_e\}^T = & \left[ pab \frac{1}{4} ; pab^2 \frac{1}{24} ; -pa^2b \frac{1}{24} ; pab \frac{1}{4} ; +pab^2 \frac{1}{24} ; \right. \\ & pa^2b \frac{1}{24} ; pab \frac{1}{4} ; -pab^2 \frac{1}{24} ; pa^2b \frac{1}{24} ; pab \frac{1}{4} ; \\ & \left. -pab^2 \frac{1}{24} ; -pa^2b \frac{1}{24} \right] \end{aligned}$$

## 3) Genişle Matrisleri

Yük Matrisinden her bir genişle matrisleri verilir.

$$[k_e] = \frac{Et^3}{2(1-\nu^2)ab}^*$$

| 1     | 2     | 3     | 4     | 5     | 6     | 7     | 8     | 9     | 10    | 11    | 12    |
|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 10.68 | 2.32  | -2.32 | -4.68 | 0.68  | -2.17 | -1.32 | 0.83  | -0.83 | -4.68 | 2.17  | -0.68 |
| 2.32  | 1.56  | -0.15 | 0.68  | 0.44  | 0     | -0.83 | 0.39  | 0     | -2.17 | 0.61  | 0     |
| -2.32 | -0.15 | 1.56  | 2.17  | 0     | 0.61  | 0.83  | 0     | 0.39  | -0.68 | 0     | 0.44  |
| -4.68 | 0.68  | 2.17  | 10.68 | 2.32  | 2.32  | -4.68 | 2.17  | 0.68  | -1.32 | 0.83  | 0.83  |
| 0.68  | 0.44  | 0     | 2.32  | 1.56  | 0.15  | -2.17 | 0.61  | 0     | -0.83 | 0.39  | 0     |
| -2.17 | 0     | 0.61  | 2.32  | 0.15  | 1.56  | 0.68  | 0     | 0.44  | -0.83 | 0     | 0.39  |
| -1.32 | -0.83 | 0.83  | -4.68 | -2.17 | 0.68  | 10.68 | -2.32 | 2.32  | -4.68 | -0.68 | 2.17  |
| 0.83  | 0.39  | 0     | 2.17  | 0.61  | 0     | -2.32 | 1.56  | -0.15 | -0.68 | 0.44  | 0     |
| -0.83 | 0     | 0.39  | 0.68  | 0     | 0.44  | 2.32  | -0.15 | 1.56  | -2.17 | 0     | 0.61  |
| -4.68 | -2.17 | -0.68 | -1.32 | -0.83 | -0.83 | -4.68 | -0.68 | -2.17 | 10.68 | -2.32 | -2.32 |
| 2.17  | 0.61  | 0     | 0.83  | 0.39  | 0     | -0.68 | 0.44  | 0     | -2.32 | 1.56  | 0.15  |
| -0.68 | 0     | 0.44  | 0.83  | 0     | 0.39  | 2.17  | 0     | 0.61  | -2.32 | 0.15  | 1.56  |

$$\{q_e\} =$$

|        |    |
|--------|----|
| 3.75   | 1  |
| 0.625  | 2  |
| -0.625 | 3  |
| 3.75   | 4  |
| 0.625  | 5  |
| 0.625  | 6  |
| 3.75   | 7  |
| -0.625 | 8  |
| 0.625  | 9  |
| 3.75   | 10 |
| -0.625 | 11 |
| -0.625 | 12 |



Çevirme Matrisleri

| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 |
|---|---|---|---|---|---|---|---|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|
| 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 1  | 0  | 0  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 0  | 1  |

$\{c_{e1}\} =$



### Çevirme Matrisleri

$$= \{c_{e3}\}$$

[illegible]

## Çevirme Matrisleri

$$= \{c_{e4}\}$$

[illegible]

Sistem rijitlik ve yük matrisi

|    | 1     | 2     | 3     | 4     | 5     | 6     | 7     | 8     | 9     | 10    | 11    | 12    | 13    | 14    | 15    | 16    | 17    | 18    | 19    | 20    | 21    | 22    | 23    | 24    | 25    | 26    | 27    | Q   |        |
|----|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-----|--------|
| 1  | 10.68 | 2.32  | -2.32 | -4.68 | 0.68  | -2.17 | 0     | 0     | 0     | -4.68 | 2.17  | -0.68 | -1.32 | 0.83  | -0.83 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D1  | 3.75   |
| 2  | 2.32  | 1.56  | -0.15 | 0.68  | 0.44  | 0     | 0     | 0     | 0     | -2.17 | 0.61  | 0     | -0.83 | 0.39  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D2  | 0.625  |
| 3  | -2.32 | -0.15 | 1.56  | 2.17  | 0     | 0.61  | 0     | 0     | 0     | -0.68 | 0     | 0.44  | 0.83  | 0     | 0.39  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D3  | -0.625 |
| 4  | -4.68 | 0.68  | 2.17  | 21.36 | 4.64  | 0     | -4.68 | 0.68  | -2.17 | -1.32 | 0.83  | 0.83  | -9.36 | 4.34  | 0     | -1.32 | 0.83  | -0.83 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D4  | 7.5    |
| 5  | 0.68  | 0.44  | 0     | 4.64  | 3.12  | 0     | 0.68  | 0.44  | 0     | -0.83 | 0.39  | 0     | -4.34 | 1.22  | 0     | -0.83 | 0.39  | 0     | 0.39  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D5  | 1.25   |
| 6  | -2.17 | 0     | 0.61  | 0     | 0     | 3.12  | 2.17  | 0     | 0.61  | -0.83 | 0     | 0.39  | 0     | 0.88  | 0.83  | 0     | 0.39  | 0     | 0.39  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D6  | 0      |
| 7  | 0     | 0     | 0     | -4.68 | 0.68  | 2.17  | 10.68 | 2.32  | 2.32  | 0     | 0     | 0     | -1.32 | 0.83  | 0.83  | -4.68 | 2.17  | 0.68  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D7  | 3.75   |
| 8  | 0     | 0     | 0     | 0.68  | 0.44  | 0     | 2.32  | 1.55  | 0.15  | 0     | 0     | 0     | -0.83 | 0.39  | 0     | 2.17  | 0.61  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D8  | 0.625  |
| 9  | 0     | 0     | 0     | -2.17 | 0     | 0.61  | 2.32  | 0.15  | 1.56  | 0     | 0     | -4.64 | -9.36 | 0     | 0.39  | 0.68  | 0     | 0.44  | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | D9  | 0.625  |
| 10 | -4.68 | -2.17 | -0.68 | -1.32 | -0.83 | -0.83 | 0     | 0     | 0     | 21.36 | 0     | -4.64 | -9.36 | 0     | -4.34 | 0     | 0     | 0     | -4.68 | 2.17  | -0.68 | -1.32 | 0.83  | -0.83 | 0     | 0     | 0     | D10 | 7.5    |
| 11 | 2.17  | 0.61  | 0     | 0.83  | 0.39  | 0     | 0     | 0     | 0     | -4.64 | 3.12  | 0     | 4.34  | 0     | 1.22  | 0     | 0     | 0     | -2.17 | 0.61  | 0     | -0.83 | 0.39  | 0     | 0.39  | 0     | 0     | D11 | 0      |
| 12 | -0.68 | 0     | 0.44  | 0.83  | 0     | 0.39  | 0     | 0     | 0     | 0     | 4.34  | 42.72 | 0     | 6.24  | 0     | -9.36 | 0     | -4.34 | -1.32 | 0.83  | 0.44  | -9.36 | 4.34  | 0     | 0.39  | 0     | 0     | D12 | -1.25  |
| 13 | -1.32 | -0.83 | 0.83  | -9.36 | -4.34 | 0     | 0.83  | 0.39  | 0     | 0.88  | 0     | 0     | 0     | 6.24  | 0     | 0     | 0.88  | 0     | -4.34 | -1.32 | 0.83  | -9.36 | 4.34  | 0     | -0.83 | 0.39  | 0     | D13 | 15     |
| 14 | 0.83  | 0.39  | 0     | 4.34  | 1.22  | 0     | 0.83  | 0.39  | 0     | -4.34 | 1.22  | 0     | -9.36 | 0     | 4.34  | 4.34  | 0     | 1.22  | -0.83 | 0.39  | 0     | 0     | 0.88  | 0.83  | 0.83  | 2.17  | 0.68  | D14 | 0      |
| 15 | -0.83 | 0     | 0.39  | 0     | 0     | 0.88  | 0.83  | 0     | 0     | 0     | 0     | 0     | 0     | 0.88  | 0     | 21.36 | 0     | 4.64  | 0     | 0     | 0     | 0     | 0     | 0.88  | 0.83  | 0     | 0.39  | D15 | 0      |
| 16 | 0     | 0     | 0     | -1.32 | -0.83 | 0.83  | -4.68 | -2.17 | 0.68  | -4.34 | 0     | 1.22  | 0     | 0     | 6.24  | 0     | 0     | 0     | 0     | 0     | 0     | -1.32 | 0.83  | 0.83  | 2.17  | 0.68  | 0     | D16 | 7.5    |
| 17 | 0     | 0     | 0     | 0.83  | 0.39  | 0     | 2.17  | 0.61  | 0     | 0     | 0     | 0     | -9.36 | 0     | 4.34  | 21.36 | 0     | 4.64  | 0     | 0     | 0     | -0.83 | 0.39  | 0     | 0.39  | 0.61  | 0     | D17 | 0      |
| 18 | 0     | 0     | 0     | -0.83 | 0     | 0.39  | 0.68  | 0     | 0.44  | 0     | 0     | 0     | 0     | 0.88  | 0     | 0     | 3.12  | 0     | 0     | 0     | 0     | -0.83 | 0     | 0.39  | 0.68  | 0     | 0.44  | D18 | 1.25   |
| 19 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | -4.68 | -2.17 | -0.68 | -1.32 | -0.83 | -0.83 | 0     | 0     | 0     | 0.68  | -2.32 | -2.32 | -4.68 | -0.68 | -2.17 | 0     | 0     | 0     | D19 | 3.75   |
| 20 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 2.17  | 0.61  | 0     | 0.83  | 0.39  | 0     | 0     | 0     | 0     | -2.32 | 1.56  | 0.15  | -0.68 | 0.44  | 0     | 0     | 0     | 0     | D20 | -0.625 |
| 21 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | -0.68 | 0     | -1.32 | 0.83  | 0.39  | 0     | 0     | 0     | 0     | -2.32 | 1.56  | -1.56 | 2.17  | -4.64 | 0     | 0.61  | 0     | 0     | D21 | -0.625 |
| 22 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | -1.32 | -0.83 | 0.83  | -9.36 | -4.34 | 0     | -1.32 | -0.83 | -0.83 | -4.68 | -0.68 | 2.17  | 21.36 | -4.64 | 0     | -4.68 | -2.17 | -2.17 | D22 | 7.5    |
| 23 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0.83  | 0.39  | 0     | 4.34  | 1.22  | 0     | 0.83  | 0.39  | 0     | -4.68 | -0.68 | 0     | -4.64 | 3.12  | 0     | -0.68 | 0.44  | 0     | D23 | -1.25  |
| 24 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | -0.83 | 0     | 0.39  | -1.32 | 0.83  | 0     | 0.88  | 0.83  | 0.39  | -2.17 | 0     | 0.61  | 0     | 0     | 3.12  | 2.17  | 0     | 0.61  | D24 | 0      |
| 25 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | -1.32 | -0.83 | 0.83  | -4.68 | 2.17  | 0.68  | 0     | 0     | 0     | -4.68 | -0.68 | 2.17  | 10.68 | -2.32 | 2.32  | D25 | 3.75   |
| 26 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0.83  | 0.39  | 0     | 2.17  | 0.61  | 0     | 0     | 0     | 0     | -0.68 | 0.44  | 0     | -2.32 | 1.56  | -0.15 | D26 | -0.625 |
| 27 | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | 0     | -0.83 | 0     | 0.39  | 0.68  | 0     | 0.44  | 0     | 0     | 0     | -2.17 | 0     | 0.61  | 2.32  | -0.15 | 1.56  | D27 | 0.625  |

[ke]=



|    | 1     | 4     | 6     | 10    | 11   | 13    | 14   | 15    | Q   |
|----|-------|-------|-------|-------|------|-------|------|-------|-----|
| 1  | 10.68 | -4.68 | -2.17 | -4.68 | 2.17 | -1.32 | 0.83 | -0.83 | D1  |
| 4  | -4.68 | 21.36 | 0     | -1.32 | 0.83 | -9.36 | 4.34 | 0     | D4  |
| 6  | -2.17 | 0     | 3.12  | -0.83 | 0    | 0     | 0    | 0.88  | D6  |
| 10 | -4.68 | -1.32 | -0.83 | 21.36 | 0    | -9.36 | 0    | -4.34 | D10 |
| 11 | 2.17  | 0.83  | 0     | 0     | 3.12 | 0     | 0.88 | 0     | D11 |
| 13 | -1.32 | -9.36 | 0     | -9.36 | 0    | 42.72 | 0    | 0     | D13 |
| 14 | 0.83  | 4.34  | 0     | 0     | 0.88 | 0     | 6.24 | 0     | D14 |
| 15 | -0.83 | 0     | 0.88  | -4.34 | 0    | 0     | 0    | 6.24  | D15 |

$$K = \frac{Et^3}{12(1-\nu^2)}$$

$$D1=5.34/D$$

$$D4=3.18/D$$

$$D6=3.88/D$$

$$D10=3.18/D$$

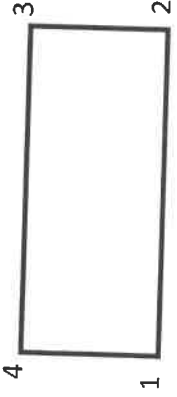
$$D11=-3.88/D$$

$$D13=1.92/D$$

$$D14=-2.38/D$$

$$D15=2.38/D$$





1 nolu eleman için gerilme hesabı;

1 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x1} \\ M_{y1} \\ M_{xy1} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} * \begin{Bmatrix} 6.9 & 0.6 & -4 & -6 & 0 & -2 & 0 & 0 & 0 & 0 & -0.9 & 0.3 & 0 \\ 6.9 & 4 & -0.6 & -0.9 & 0 & -0.3 & 0 & 0 & 0 & 0 & -6 & 2 & 0 \\ 0.85 & 0.85 & -0.85 & -0.85 & 0 & 0 & 0.85 & 0 & 0 & 0 & -0.85 & 0 & 0.85 \end{Bmatrix}$$

$$M_{x1}=5.98 \text{ kNm/m}$$

$$M_{y1}=5.98 \text{ kNm/m}$$

$$M_{xy1}=0.76 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 5.34/D  | D1  |
| 0       | D2  |
| 0       | D3  |
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |

1 nolu eleman için gerilme hesabı;

2 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x2} \\ M_{y2} \\ M_{xy2} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} \begin{Bmatrix} -6 & 0 & 2 & 6.9 & 0.6 & 4 \\ -0.9 & 0 & 0.3 & 6.9 & 4 & 0.6 \\ 0.85 & 0.85 & 0 & -0.85 & -0.85 & -0.85 \end{Bmatrix} \begin{Bmatrix} 3 \\ 2 \\ 0.3 \\ -0.9 & 0.3 & 0 \\ -6 & 2 & 0 \\ 0.85 & 0 & 0.85 \end{Bmatrix} \begin{Bmatrix} 4 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ -0.85 & 0 & 0 \end{Bmatrix}$$

$$M_{x2}=2.98 \text{ kNm/m}$$

$$M_{y2}=3.18 \text{ kNm/m}$$

$$M_{xy2}=-0.51 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 5.34/D  | D1  |
| 0       | D2  |
| 0       | D3  |
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |

1 nolu eleman için gerilme hesabı;

3 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x3} \\ M_{y3} \\ M_{xy3} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} \begin{Bmatrix} 0 & 0 & 0 & -0.9 & -0.3 & 0 & 6.9 & -0.6 & 4 & -6 & 0 & 2 \\ 0 & 0 & 0 & -6 & -2 & 0 & 6.9 & -4 & 0.6 & -0.9 & 0 & 0.3 \\ 0.85 & 0 & 0 & -0.85 & 0 & -0.85 & 0.85 & -0.85 & 0.85 & -0.85 & 0.85 & 0 \end{Bmatrix}$$

$$M_{x2}=2.25 \text{ kNm/m}$$

$$M_{y2}=2.25 \text{ kNm/m}$$

$$M_{xy2}=-1.79 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 5.34/D  | D1  |
| 0       | D2  |
| 0       | D3  |
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |

1 nolu eleman için gerilme hesabı;

4 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=1$

$$\text{Eleman Düğüm Noktası} \longrightarrow \begin{matrix} & 1 & & 2 & & 3 & & 4 \end{matrix}$$

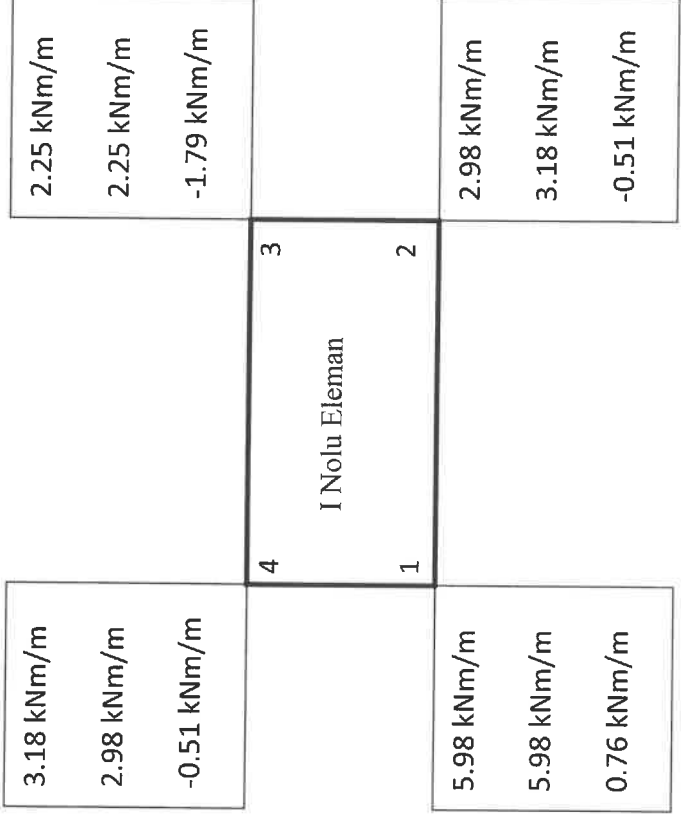
$$\begin{cases} M_{x4} \\ M_{y4} \\ M_{xy4} \end{cases} = \frac{Et^3}{12(1-\nu^2)} \cdot \begin{matrix} \begin{matrix} -0.9 & -0.3 & 0 & 0 & 0 & 0 & -6 & 0 & 6.9 & -0.6 & -4 \end{matrix} \\ \begin{matrix} -6 & -2 & 0 & 0 & 0 & 0 & -0.9 & 0 & 6.9 & -4 & -0.6 \end{matrix} \\ \begin{matrix} 0.85 & 0 & -0.85 & -0.85 & 0 & 0 & 0.85 & -0.85 & 0 & 0.85 & 0.85 \end{matrix} \end{matrix}$$

$$M_{x2}=3.18 \text{ kNm/m}$$

$$M_{y2}=2.98 \text{ kNm/m}$$

$$M_{xy2}=-0.51 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 5.34/D  | D1  |
| 0       | D2  |
| 0       | D3  |
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |



2 nolu eleman için gerilme hesabı;

1 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x1} \\ M_{y1} \\ M_{xy1} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} *$$

|           | 2     | 3    | 4    |
|-----------|-------|------|------|
| 6.9 0.6   | -4    | -2   | 0    |
| 6.9 4     | -0.9  | -0.3 | 0    |
| 0.85 0.85 | -0.85 | 0    | 0.85 |

$$M_{x1}=3.98 \text{ kNm/m}$$

$$M_{y1}=3.33 \text{ kNm/m}$$

$$M_{xy1}=-0.20 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 0       | D7  |
| 0       | D8  |
| 0       | D9  |
| 0       | D16 |
| 0       | D17 |
| 0       | D18 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |



2 nolu eleman için gerilme hesabı;

3 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x3} \\ M_{y3} \\ M_{xy3} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} \begin{matrix} * \\ \begin{matrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0.85 & 0 & 0 \end{matrix} \end{matrix}$$

|           | 2     | 3    | 4    |
|-----------|-------|------|------|
| $M_{x3}$  | -0.9  | -0.6 | 4    |
| $M_{y3}$  | -6    | -4   | 0.6  |
| $M_{xy3}$ | -0.85 | 0.85 | 0.85 |

$$M_{x2} = -6.76 \text{ kNm/m}$$

$$M_{y2} = -1.01 \text{ kNm/m}$$

$$M_{xy2} = -0.95 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 0       | D7  |
| 0       | D8  |
| 0       | D9  |
| 0       | D16 |
| 0       | D17 |
| 0       | D18 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |

2 nolu eleman için gerilme hesabı;

4 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow$  1

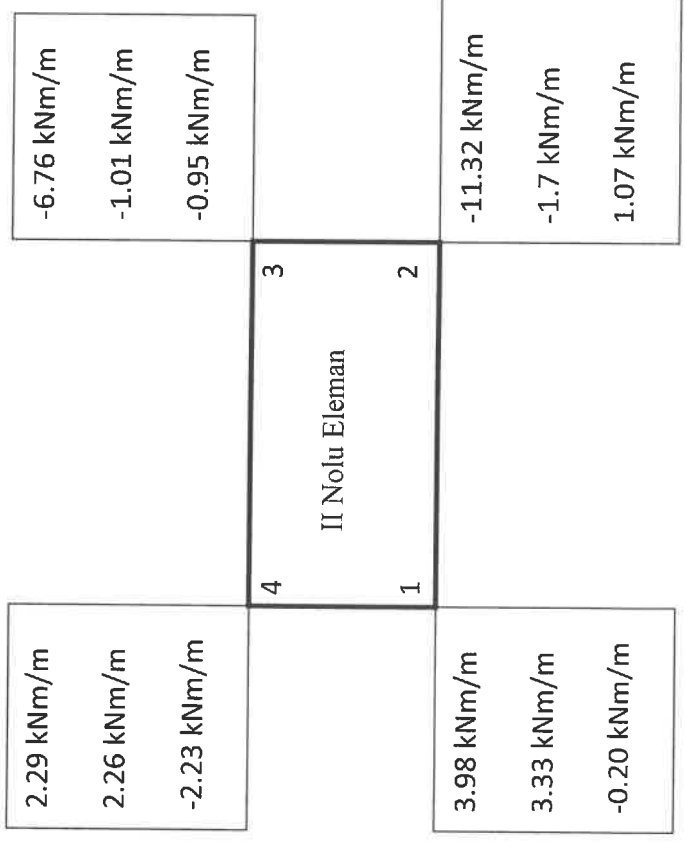
$$\begin{Bmatrix} M_{x4} \\ M_{y4} \\ M_{xy4} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} \begin{Bmatrix} -0.9 & -0.3 & 0 & 0 & 0 & 0 \\ -6 & -2 & 0 & 0 & 0 & 0 \\ 0.85 & 0 & -0.85 & -0.85 & 0 & 0 \end{Bmatrix}$$

$M_{x2}=2.29 \text{ kNm/m}$

$M_{y2}=2.26 \text{ kNm/m}$

$M_{xy2}=-2.23 \text{ kNm/m}$

|         |     |
|---------|-----|
| 3.18/D  | D4  |
| 0       | D5  |
| 3.88/D  | D6  |
| 0       | D7  |
| 0       | D8  |
| 0       | D9  |
| 0       | D16 |
| 0       | D17 |
| 0       | D18 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |





3 nolu eleman için gerilme hesabı;

1 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\longrightarrow 1$

$$\begin{Bmatrix} M_{x1} \\ M_{y1} \\ M_{xy1} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} *$$

|      |      |       |       |       |      |      |   |   |       |     |      |
|------|------|-------|-------|-------|------|------|---|---|-------|-----|------|
| 6.9  | 0.6  | -4    | -6    | 0     | -2   | 0    | 0 | 0 | -0.9  | 0.3 | 0    |
| 6.9  | 4    | -0.6  | -0.9  | 0     | -0.3 | 0    | 0 | 0 | -6    | 2   | 0    |
| 0.85 | 0.85 | -0.85 | -0.85 | -0.85 | 0    | 0.85 | 0 | 0 | -0.85 | 0   | 0.85 |

$M_{x1}=3.33 \text{ kNm/m}$

$M_{y1}=3.98 \text{ kNm/m}$

$M_{xy1}=-0.20 \text{ kNm/m}$

|         |     |
|---------|-----|
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |
| 0       | D19 |
| 0       | D20 |
| 0       | D21 |

3 nolu eleman için gerilme hesabı;

2 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\rightarrow 1$

$$\begin{Bmatrix} M_{x2} \\ M_{y2} \\ M_{xy2} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} *$$

|       | 2     | 3    | 4     |
|-------|-------|------|-------|
| -6    | 0     | 2    | 0     |
| -0.9  | 0     | 0.3  | 0     |
| 0.85  | 0.85  | 0    | 0     |
| 6.9   | 0.6   | -0.9 | 0     |
| 6.9   | 4     | -6   | 0     |
| -0.85 | -0.85 | 0.85 | -0.85 |

$$M_{x2}=2.26 \text{ kNm/m}$$

$$M_{y2}=2.29 \text{ kNm/m}$$

$$M_{xy2}=-2.23 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |
| 0       | D19 |
| 0       | D20 |
| 0       | D21 |

3 nolu eleman için gerilme hesabı;

3 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow 1$

$$\begin{Bmatrix} M_{x3} \\ M_{y3} \\ M_{xy3} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} *$$

|      |   |   |       |      |       |      |       |      |       |      |     |
|------|---|---|-------|------|-------|------|-------|------|-------|------|-----|
| 0    | 0 | 0 | -0.9  | -0.3 | 0     | 6.9  | -0.6  | 4    | -6    | 0    | 2   |
| 0    | 0 | 0 | -6    | -2   | 0     | 6.9  | -4    | 0.6  | -0.9  | 0    | 0.6 |
| 0.85 | 0 | 0 | -0.85 | 0    | -0.85 | 0.85 | -0.85 | 0.85 | -0.85 | 0.85 | 0   |

$$M_{x2} = -1.01 \text{ kNm/m}$$

$$M_{y2} = -6.76 \text{ kNm/m}$$

$$M_{xy2} = -0.95 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |
| 0       | D19 |
| 0       | D20 |
| 0       | D21 |

3 nolu eleman için gerilme hesabı;

4 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow$  1

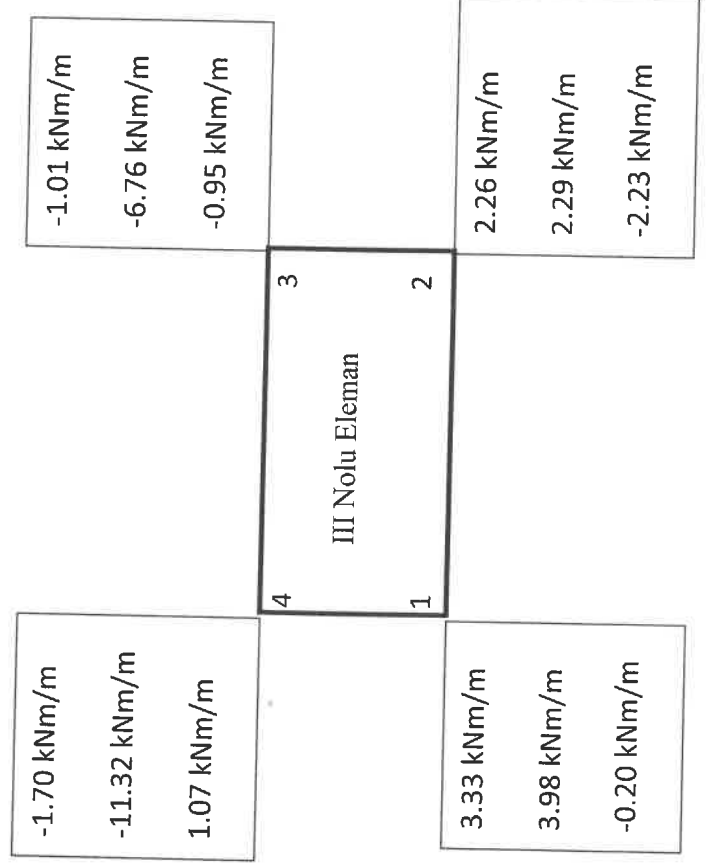
$$\begin{Bmatrix} M_{x4} \\ M_{y4} \\ M_{xy4} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} * \begin{matrix} & \begin{matrix} 2 & 3 & 4 \end{matrix} \\ \begin{matrix} -0.9 & -0.3 & 0 \\ -6 & -2 & 0 \\ 0.85 & 0 & -0.85 \end{matrix} & \begin{matrix} \begin{matrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ -0.85 & -0.85 & 0 \end{matrix} & \begin{matrix} \begin{matrix} -6 & 0 & -2 \\ -0.9 & 0 & -0.3 \\ 0.85 & -0.85 & 0 \end{matrix} & \begin{matrix} \begin{matrix} 0 & 6.9 & -0.6 \\ 0 & 6.9 & -4 \\ -0.85 & 0.85 & 0.85 \end{matrix} \end{matrix} \end{matrix}$$

$$M_{x2}=-1.70 \text{ kNm/m}$$

$$M_{y2}=-11.32 \text{ kNm/m}$$

$$M_{xy2}=1.07 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 3.18/D  | D10 |
| -3.88/D | D11 |
| 0       | D12 |
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |
| 0       | D19 |
| 0       | D20 |
| 0       | D21 |



4 nolu eleman için gerilme hesabı;

1 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x1} \\ M_{y1} \\ M_{xy1} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} * \begin{Bmatrix} 6.9 & 0.6 & -4 & -6 & 0 & -2 & 0 & 0 & 0 & -0.9 & 0.3 & 0 \\ 6.9 & 4 & -0.6 & -0.9 & 0 & -0.3 & 0 & 0 & 0 & -6 & 2 & 0 \\ 0.85 & 0.85 & -0.85 & -0.85 & -0.85 & 0 & 0.85 & 0 & 0 & -0.85 & 0 & 0.85 \end{Bmatrix}$$

$$M_{x1}=2.30 \text{ kNm/m}$$

$$M_{y1}=2.30 \text{ kNm/m}$$

$$M_{xy1}=-2.41 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D16 |
| 0       | D17 |
| 0       | D18 |
| 0       | D25 |
| 0       | D26 |
| 0       | D27 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |

4 nolu eleman için gerilme hesabı;

2 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=0$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x2} \\ M_{y2} \\ M_{xy2} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} *$$

|      | 2    | 3     | 4 |
|------|------|-------|---|
| -6   | 0    | 2     | 0 |
| -0.9 | 0    | 0.3   | 0 |
| 0.85 | 0.85 | -0.85 | 0 |

$$M_{x2} = -6.76 \text{ kNm/m}$$

$$M_{y2} = -1.01 \text{ kNm/m}$$

$$M_{xy2} = -0.39 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D16 |
| 0       | D17 |
| 0       | D18 |
| 0       | D25 |
| 0       | D26 |
| 0       | D27 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |

4 nolu eleman için gerilme hesabı;

3 nolu köşe için  $\xi=x/a=1$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow$  1

$$\begin{Bmatrix} M_{x3} \\ M_{y3} \\ M_{xy3} \end{Bmatrix} = \frac{Et^3}{12(1-\nu^2)} *$$

|      | 2     | 3    | 4    |
|------|-------|------|------|
| 0    | 0     | 0    | 2    |
| 0    | -0.9  | -0.6 | 0    |
| 0    | -6    | -4   | 0    |
| 0.85 | -0.85 | 0.85 | 0.85 |

$$M_{x2}=0 \text{ kNm/m}$$

$$M_{y2}=0 \text{ kNm/m}$$

$$M_{xy2}=1.63 \text{ kNm/m}$$

|         |     |
|---------|-----|
| 1.92/D  | D13 |
| -2.38/D | D14 |
| 2.38/D  | D15 |
| 0       | D16 |
| 0       | D17 |
| 0       | D18 |
| 0       | D25 |
| 0       | D26 |
| 0       | D27 |
| 0       | D22 |
| 0       | D23 |
| 0       | D24 |

4 nolu eleman için gerilme hesabı;

4 nolu köşe için  $\xi=x/a=0$  ;  $\eta=y/b=1$

Eleman Düğüm Noktası  $\longrightarrow 1$

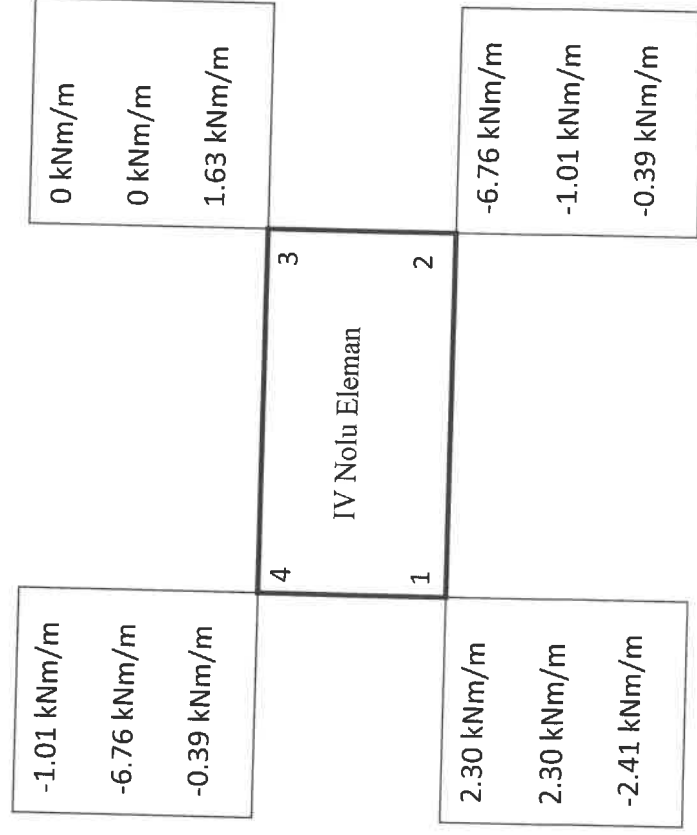
$$= \frac{Et^3}{12(1-\nu^2)} *$$

|      | → 1  | 2     |       | 3 |      | 4     |      |       |      |      |
|------|------|-------|-------|---|------|-------|------|-------|------|------|
| -0.9 | -0.3 | 0     | 0     | 0 | -6   | 0     | -2   | 6.9   | -0.6 | -4   |
| -6   | -2   | 0     | 0     | 0 | -0.9 | 0     | -0.3 | 6.9   | -4   | -0.6 |
| 0.85 | 0    | -0.85 | -0.85 | 0 | 0.85 | -0.85 | 0    | -0.85 | 0.85 | 0.85 |

$$M_{x2} = -1.01 \text{ kNm/m}$$

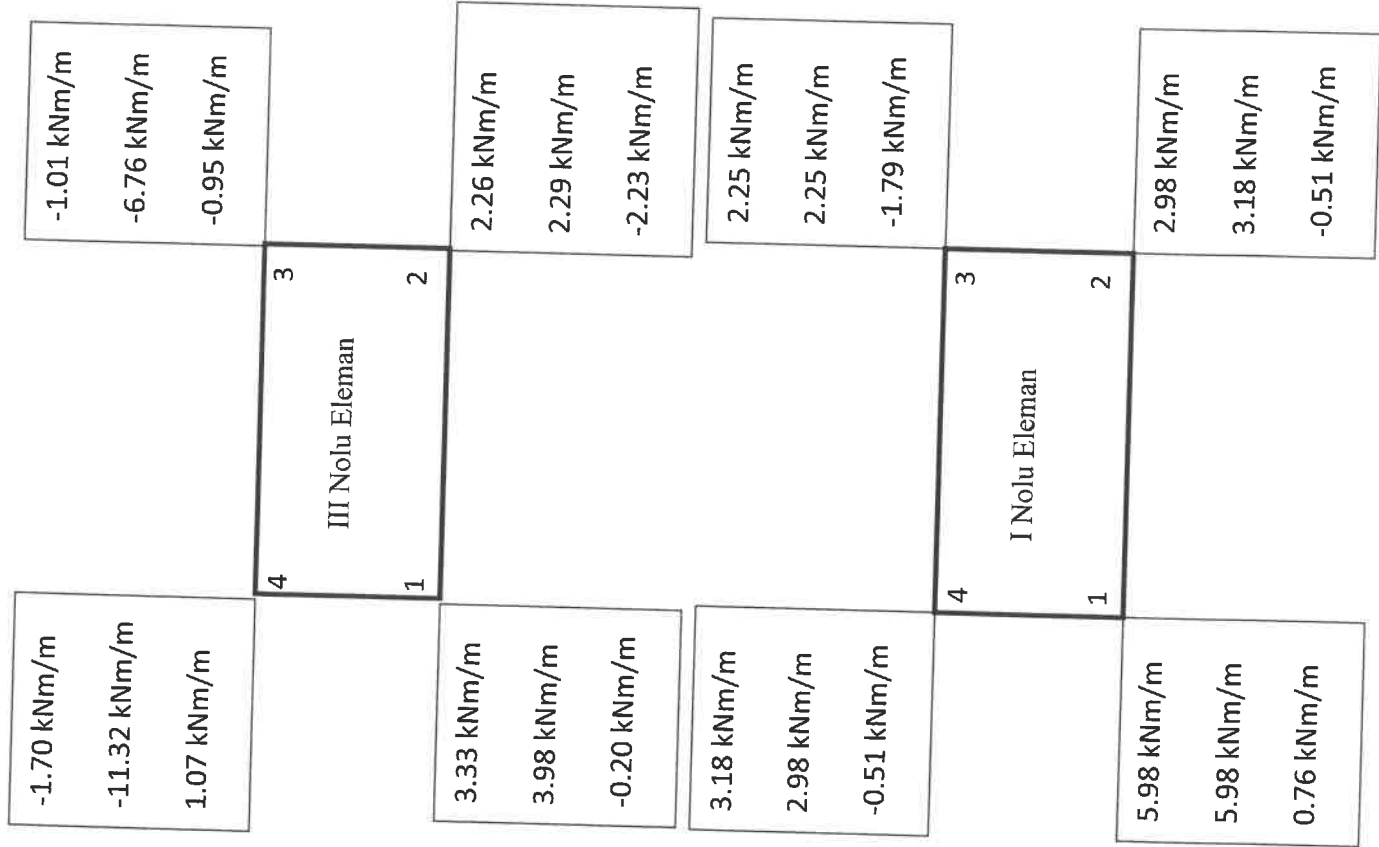
$$M_{y2} = -6.76 \text{ kNm/m}$$

$$M_{xy2} = -0.39 \text{ kNm/m}$$



|     |         |  |
|-----|---------|--|
| D13 | 1.92/D  |  |
| D14 | -2.38/D |  |
| D15 | 2.38/D  |  |
| D16 | 0       |  |
| D17 | 0       |  |
| D18 | 0       |  |
| D25 | 0       |  |
| D26 | 0       |  |
| D27 | 0       |  |
| D22 | 0       |  |
| D23 | 0       |  |
| D24 | 0       |  |





Tam sütündeki 2 nolu diğünün momentleri;

$$M_x = \frac{2,98 + 3,98}{2} = 3,48 \text{ kNm/m}$$

$$M_y = \frac{3,18 + 3,33}{2} = 3,26 \quad ^\circ$$

$$M_{xy} = \frac{-0,51 - 0,26}{2} = -0,36 \quad ^\circ$$

Tam sütündeki 5 nolu diğünün momentleri

$$M_x = \frac{2,25 + 2,29 + 2,26 + 2,30}{4} = 2,28 \text{ kNm/m}$$

$$M_y = \frac{2,25 + 2,26 + 2,29 + 2,30}{4} = 2,28 \quad ^\circ$$

$$M_{xy} = \frac{-1,79 - 2,23 - 2,23 - 2,41}{4} = -2,17 \quad ^\circ$$

Diğer diğün momentleri de benzer şekilde bulunur



## ORTHOTROP PLAKLARIN SONLU ELEMANLAR YÖNTEMİ

### İLE HESABI

Orthotrop plaklarda deplasman farksiyon, rijitlik matrisi ve yük matrisinin oluşturulma yönteminde izotrop plaklardan bir farkı yoktur. Ancak elastisite bağıntısı

$$[D] = \begin{bmatrix} C_{44} & C_{45} & 0 \\ C_{45} & C_{55} & 0 \\ 0 & 0 & C_{66} \end{bmatrix}$$

$$C_{44} = D + \frac{E(I_x + A x_0 e_x^2)}{dx}$$

$$C_{55} = D + \frac{E(I_y + A y_0 e_y^2)}{dy}$$

$$C_{66} = (1-\nu)D + \frac{1}{2} \left( \frac{GI_x}{dx} + \frac{GI_y}{dy} \right)$$

$$C_{45} = \nu D$$

şeklinde olacaktır.

### Elastik Zemin Oturan İnce Plakların Eğilimli Hesabı

Plak deplasman farksiyon ve eleman rijitlik matrisi plak teorisinde verildiği gibidir.

$$w = a_1 + a_2 x + a_3 y + a_4 x^2 + a_5 xy + a_6 y^2 + a_7 x^3 + a_8 x^2 y + a_9 xy^2 + a_{10} y^3 + a_{11} x^2 y + a_{12} xy^3$$

$$[k_e] = \iint [B]^T [F]^T [D] [F] [B] dx dy \quad \text{veya}$$

$$[k_e] = ab \int_0^1 \int_0^1 [\Delta N]^T [D] [\Delta N] d\xi d\eta$$

## Zemin Etki Matrisi

Zemin etki matrisine elastik yataklama matrisi de  
denir. Burada;

$$[S_e] = c \iint [N]^T [N] dx dy$$

$$[N] = [\phi(x,y)] [B]$$

$$[S_e] = c \iint [B]^T [\phi(x,y)]^T [\phi(x,y)] [B] dx dy \quad \text{dir. Burada}$$

$c$  elastik yataklama katsayısıdır.

## Söfeme Geçir

$([K] + [S]) \{ \Delta \} = \{ B \}$  bağıntısından  $\{ \Delta \}$  yer-  
değiştirmeler bulunur. Buradan söfemde oluşan  
gerilmeler hesaplanır.

EK:3

## MATLAB ÖRNEKLERİ

## KOD1-İKİ MATRİS ÇARPIMI

$$[C1] = [A1] * [B1]$$

$$[A1] * [B1] = \begin{bmatrix} 3 & 4 & 7 & -4 & 2 \\ 2 & -1 & 0 & -7 & 1 \\ 3 & 5 & 11 & 1 & 2 \end{bmatrix} * \begin{bmatrix} 1 & 2 & -6 & 3 \\ 3 & 4 & 5 & 10 \\ -1 & 11 & 3 & -2 \\ 0 & 5 & 1 & 5 \\ 3 & 7 & 2 & 3 \end{bmatrix}$$

```
>> A1=[3 4 7 -4 2; 2 -1 0 -7 1; 3 5 11 1 2];
```

$$A1 = \begin{bmatrix} 3 & 4 & 7 & -4 & 2 \\ 2 & -1 & 0 & -7 & 1 \\ 3 & 5 & 11 & 1 & 2 \end{bmatrix}$$

```
>> B1=[1 2 -6 3; 3 4 5 10; -1 11 3 -2; 0 5 1 5; 3 7 2 3];
```

$$B1 = \begin{bmatrix} 1 & 2 & -6 & 3 \\ 3 & 4 & 5 & 10 \\ -1 & 11 & 3 & -2 \\ 0 & 5 & 1 & 5 \\ 3 & 7 & 2 & 3 \end{bmatrix}$$

```
>> C1=A1*B1
```

$$C1 = \begin{bmatrix} 14 & 93 & 23 & 21 \\ 2 & -28 & -22 & -36 \\ 13 & 166 & 45 & 48 \end{bmatrix}$$

## KOD2-ÜÇ MATRİSİN ÇARPIMI

$$[D2] = [A2] * [B2] * [C2]$$

$$[A2] * [B2] * [C2] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} * \begin{bmatrix} 12 & 6 & -12 & 6 \\ 6 & 4 & -6 & 2 \\ -12 & -6 & 12 & -6 \\ 6 & 2 & -6 & 4 \end{bmatrix} * \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix}$$

clear, clc

>>syms L

>>syms x

>> A2=[1 0 0 0; 0 1 0 0; 0 0 1 0; 0 0 0 1; 0 0 0 0; 0 0 0 0; 0 0 0 0; 0 0 0 0]

$$A2 = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

>> B2=[12 6 -12 6; 6 4 -6 2; -12 -6 12 -6; 6 2 -6 4]

$$B2 = \begin{bmatrix} 12 & 6 & -12 & 6 \\ 6 & 4 & -6 & 2 \\ -12 & -6 & 12 & -6 \\ 6 & 2 & -6 & 4 \end{bmatrix}$$



```
>> C2=[1 0 0 0 0 0 0 0;0 1 0 0 0 0 0 0;0 0 1 0 0 0 0 0;0 0 0 1 0 0 0 0]
```

$$C2 = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix}$$

```
>> D2=A2*B2*C2
```

$$D2 = \begin{bmatrix} 12 & 6 & -12 & 6 & 0 & 0 & 0 & 0 \\ 6 & 4 & -6 & 2 & 0 & 0 & 0 & 0 \\ -12 & -6 & 12 & -6 & 0 & 0 & 0 & 0 \\ 6 & 2 & -6 & 4 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

### KOD3-DÖRT MATRİSİN ÇARPIMI

$$Carp\_3 = [A3] * [B3] * [C3] * [D3]$$

$$[A3] * [B3] * [C3] * [D3] = \begin{bmatrix} 1 & 0 & -\frac{3}{L^2} & \frac{2}{L^3} \\ 0 & 1 & -\frac{2}{L} & \frac{1}{L^2} \\ 0 & 0 & \frac{3}{L^2} & -\frac{2}{L^3} \\ 0 & 0 & -\frac{1}{L} & \frac{1}{L^2} \end{bmatrix} * \begin{bmatrix} 0 \\ 0 \\ -2 \\ -6x \end{bmatrix} * [0 \quad 0 \quad -2 \quad -6x] * \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{3}{L^2} & -\frac{2}{L} & \frac{3}{L^2} & -\frac{1}{L} \\ \frac{2}{L^3} & \frac{1}{L^2} & -\frac{2}{L^3} & \frac{1}{L^2} \end{bmatrix}$$

clear, clc

syms L

syms x

>> A3=[ 1 0 -3/(L^2) 2/(L^3); 0 1 -2/L 1/(L^2); 0 0 3/(L^2) -2/(L^3); 0 0 -1/L 1/(L^2)]

$$A3 = \begin{bmatrix} 1 & 0 & -\frac{3}{L^2} & \frac{2}{L^3} \\ 0 & 1 & -\frac{2}{L} & \frac{1}{L^2} \\ 0 & 0 & \frac{3}{L^2} & -\frac{2}{L^3} \\ 0 & 0 & -\frac{1}{L} & \frac{1}{L^2} \end{bmatrix}$$

>> B3=[ 0; 0; -2; -6\*x]

$$B3 = \begin{bmatrix} 0 \\ 0 \\ -2 \\ -6x \end{bmatrix}$$

```
>> C3=[ 0, 0, -2, -6*x]
```

```
C3 = [0  0  -2  -6x]
```

```
>> D3=[ 1 0 0 0 ; 0 1 0 0 ; -3/(L^2) -2/L 3/(L^2) -1/L; 2/(L^3) 1/(L^2) -2/(L^3) 1/(L^2)];
```

$$D3 = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{3}{L^2} & -\frac{2}{L} & \frac{3}{L^2} & -\frac{1}{L} \\ \frac{2}{L^3} & \frac{1}{L^2} & -\frac{2}{L^3} & \frac{1}{L^2} \end{bmatrix}$$

>> Carp\_3= A3\*B3\*C3\*D3

$$Carp_3 = \begin{bmatrix} 12x \frac{\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)}{L^3} - 3 \frac{\frac{24x}{L^3} - \frac{12}{L^2}}{L^2} & \frac{\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)}{6x} - 2 \frac{\frac{24x}{L^3} - \frac{12}{L^2}}{L} & \frac{\left(\frac{24x}{L^3} - \frac{12}{L^2}\right)}{3} \frac{L^2}{L^2} - 12x \frac{L^3}{L^3} - \frac{12x}{L^3} - \frac{6}{L^2} & \frac{\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)}{6x} \frac{L^2}{L^2} - \frac{24x}{L^3} - \frac{12}{L^2} & \frac{\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)}{6x} \frac{L^2}{L^2} - \frac{24x}{L^3} - \frac{12}{L^2} \\ 12x \frac{\left(\frac{6x}{L^2} - \frac{4}{L}\right)}{L^3} - 3 \frac{\frac{12x}{L^2} - \frac{8}{L}}{L^2} & \frac{\left(\frac{6x}{L^2} - \frac{4}{L}\right)}{6x} \frac{L^2}{L^2} - 2 \frac{\frac{12x}{L^2} - \frac{8}{L}}{L} & \frac{\left(\frac{12x}{L^2} - \frac{8}{L}\right)}{3} \frac{L^2}{L^2} - 12x \frac{L^3}{L^3} - \frac{6x}{L^2} - \frac{4}{L} & \frac{\left(\frac{6x}{L^2} - \frac{4}{L}\right)}{6x} \frac{L^2}{L^2} - \frac{12x}{L^2} - \frac{8}{L} & \frac{\left(\frac{6x}{L^2} - \frac{4}{L}\right)}{6x} \frac{L^2}{L^2} - \frac{12x}{L^2} - \frac{8}{L} \\ 3 \frac{\left(\frac{24x}{L^3} - \frac{12}{L^2}\right)}{L^2} - 12x \frac{\frac{12x}{L^3} - \frac{6}{L^2}}{L^3} & 2 \frac{\left(\frac{24x}{L^3} - \frac{12}{L^2}\right)}{L} - 6x \frac{\frac{12x}{L^3} - \frac{6}{L^2}}{L^2} & 12x \frac{\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)}{L^3} - 3 \frac{\frac{24x}{L^3} - \frac{12}{L^2}}{L^2} & \frac{\left(\frac{24x}{L^3} - \frac{12}{L^2}\right)}{L} \frac{L^2}{L^2} - 6x \frac{L^2}{L^2} & \frac{\left(\frac{24x}{L^3} - \frac{12}{L^2}\right)}{L} \frac{L^2}{L^2} - 6x \frac{L^2}{L^2} \\ 12x \frac{\left(\frac{6x}{L^2} - \frac{2}{L}\right)}{L^3} - 3 \frac{\frac{12x}{L^2} - \frac{4}{L}}{L^2} & 6x \frac{\left(\frac{6x}{L^2} - \frac{2}{L}\right)}{L^2} - 2 \frac{\frac{12x}{L^2} - \frac{4}{L}}{L} & \frac{\left(\frac{12x}{L^2} - \frac{4}{L}\right)}{3} \frac{L^2}{L^2} - 12x \frac{L^3}{L^3} & \frac{\left(\frac{12x}{L^2} - \frac{4}{L}\right)}{6x} \frac{L^2}{L^2} - \frac{12x}{L^2} - \frac{2}{L} & \frac{\left(\frac{12x}{L^2} - \frac{4}{L}\right)}{6x} \frac{L^2}{L^2} - \frac{12x}{L^2} - \frac{2}{L} \end{bmatrix}$$

# KOD4-DÖRT MATRİSİN ÇARPIMININ İNTEGRALİ

Çubuk eleman rijitlik matrisinin hesabı [ke]

$$[Carp\_3\_intg] = \int_0^L [A3] * [B3] * [C3] * [D3]$$

>> InT\_D3=int(Carp\_3, x, 0, L)

$$Carp\_3 = \int_0^L \begin{bmatrix} 12x \frac{(12x-6)}{L^3} - 3 \frac{24x-12}{L^2} & 6x \frac{(12x-6)}{L^2} - 2 \frac{24x-12}{L} & 3 \frac{(24x-12)}{L^2} - 12x \frac{6}{L^3} & 6x \frac{(12x-6)}{L^2} - \frac{24x-12}{L^2} \\ 12x \frac{(6x-4)}{L^2} - 3 \frac{12x-8}{L} & 6x \frac{(6x-4)}{L^2} - 2 \frac{12x-8}{L} & 3 \frac{(12x-8)}{L^2} - 12x \frac{4}{L^3} & 6x \frac{(6x-4)}{L^2} - \frac{12x-8}{L} \\ 3 \frac{(24x-12)}{L^2} - 12x \frac{6}{L^3} & 2 \frac{(24x-12)}{L} - 6x \frac{12x-6}{L^2} & 12x \frac{(12x-6)}{L^3} - 3 \frac{24x-12}{L^2} & 6x \frac{(24x-12)}{L^2} - 6x \frac{12x-6}{L^2} \\ 12x \frac{(6x-2)}{L^3} - 3 \frac{12x-4}{L^2} & 6x \frac{(6x-2)}{L^2} - 2 \frac{12x-4}{L} & 3 \frac{(12x-4)}{L^2} - 12x \frac{2}{L^3} & 6x \frac{(6x-2)}{L^2} - \frac{12x-4}{L} \end{bmatrix}$$

$$InT_{D3} = \begin{bmatrix} 12 \frac{12}{L^3} & 6 \frac{6}{L^2} & 12 \frac{12}{L^3} & 6 \frac{6}{L^2} \\ 6 \frac{6}{L^2} & 4 \frac{4}{L} & 6 \frac{6}{L^2} & 2 \frac{2}{L} \\ 12 \frac{12}{L^3} & 6 \frac{6}{L^2} & 12 \frac{12}{L^3} & 6 \frac{6}{L^2} \\ 6 \frac{6}{L^2} & 2 \frac{2}{L} & 6 \frac{6}{L^2} & 4 \frac{4}{L} \end{bmatrix}$$

**KOD5-ŞEKİL FONKSİYONUNDAN FAYDALANARAK ELEMAN RİJİTLİK  
MATRİSİNİN HESABI**

$$[k_e] = \int_0^L \begin{bmatrix} -N_1'' \\ -N_2'' \\ -N_3'' \\ -N_4'' \end{bmatrix} \begin{bmatrix} -N_1'' & -N_2'' & -N_3'' & -N_4'' \end{bmatrix} dx$$

clear, clc

>>syms L

>>syms x

>> N1=1-3\*(x^2)/(L^2)+2\*(x^3)/(L^3)

$$N1 = 1 - \frac{3x^2}{L^2} + \frac{2x^3}{L^3}$$

>> N2=L\*(x/L-2\*(x^2)/(L^2)+(x/L)^3)

$$N2 = L\left(\frac{x}{L} - \frac{2x^2}{L^2} + \frac{x^3}{L^3}\right)$$

>> N3=3\*x/(L^2)-2\*((x/L)^3)

$$N3 = \frac{3x^2}{L^2} - \frac{2x^3}{L^3}$$

>> N4=-1\*(x^2)/L+(x^3)/(L^2)

$$N4 = -\frac{x^2}{L} + \frac{x^3}{L^2}$$

```
>> N1_dev2=diff(diff(N1))
```

```
>> N2_dev2=diff(diff(N2))
```

```
>> N3_dev2=diff(diff(N3))
```

```
>> N4_dev2=diff(diff(N4))
```

$$N1\_dev2 = -\frac{6}{L^2} + \frac{12x}{L^3}$$

$$N2\_dev2 = L(-4/L^2 + 6x/L^3)$$

$$N3\_dev2 = \frac{6}{L^2} - \frac{12x}{L^3}$$

$$N4\_dev2 = -\frac{2}{L} + \frac{6x}{L^2}$$

>> NN\_VEC1=[N1\_dev2; N2\_dev2; N3\_dev2; N4\_dev2]

$$NN\_VEC1 = \begin{bmatrix} \frac{6}{L^2} - \frac{12x}{L^3} \\ -L(-4/L^2 + 6x/L^3) \\ -\frac{6}{L^2} + \frac{12x}{L^3} \\ \frac{2}{L} - \frac{6x}{L^2} \end{bmatrix}$$

>> NN\_VEC2=[N1\_dev2 N2\_dev2 N3\_dev2 N4\_dev2]

$$NN\_VEC2 = \begin{bmatrix} \frac{6}{L^2} - \frac{12x}{L^3} & -L(-4/L^2 + 6x/L^3) & -\frac{6}{L^2} + \frac{12x}{L^3} & \frac{2}{L} - \frac{6x}{L^2} \end{bmatrix}$$

>> kL=NN\_VEC1\*NN\_VEC2;

$$kL = \begin{bmatrix} \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & L^2\left(\frac{6x}{L^3} - \frac{4}{L^2}\right)^2 & L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) \\ L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & -\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & -L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & -\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 \\ -\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & L^2\left(\frac{6x}{L^3} - \frac{4}{L^2}\right)^2 & -L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) \\ \left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) & \left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) & L\left(\left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & -\left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) \end{bmatrix}$$



>> InT\_NN\_VEC=int(kL, x, 0, L)

$$kL = \int_0^L \begin{bmatrix} \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & -\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & \left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) \\ L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & L^2\left(\frac{6x}{L^3} - \frac{4}{L^2}\right)^2 & L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & L\left(\left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) \\ -\left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & -L\left(\left(\frac{6x}{L^3} - \frac{4}{L^2}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)\right) & \left(\frac{12x}{L^3} - \frac{6}{L^2}\right)^2 & -\left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) \\ \left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) & L\left(\left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{6x}{L^3} - \frac{4}{L^2}\right)\right) & \left(\frac{6x}{L^2} - \frac{2}{L}\right) * \left(\frac{12x}{L^3} - \frac{6}{L^2}\right) & \left(\frac{6x}{L^2} - \frac{2}{L}\right)^2 \end{bmatrix}$$

$$InT\_NN\_VEC = \begin{bmatrix} \frac{12}{L^3} & \frac{6}{L^2} & \frac{12}{L^3} & \frac{6}{L^2} \\ \frac{6}{L^2} & \frac{4}{L} & -\frac{6}{L^2} & \frac{2}{L} \\ -\frac{12}{L^3} & -\frac{6}{L^2} & \frac{12}{L^3} & \frac{6}{L^2} \\ \frac{6}{L^2} & \frac{2}{L} & -\frac{6}{L^2} & \frac{4}{L} \end{bmatrix}$$

## KOD6-KİRİŞ MOMENTLERİNİN HESABI

$$M_1 = -\frac{EI}{L^2} \begin{bmatrix} 0 & 0 & \frac{3}{8}q\frac{L^4}{EI} & \frac{1}{2}q\frac{L^3}{EI} \\ -N_1'' & -N_2'' & -N_3'' & -N_4'' \end{bmatrix}$$

clear, clc

syms L

syms x

syms q

syms E

syms I

>> N1=1-3\*(x^2)/(L^2)+2\*(x^3)/(L^3)

$$N1 = 1 - \frac{3x^2}{L^2} + \frac{2x^3}{L^3}$$

>> N2=L\*(x/L-2\*(x^2)/(L^2)+(x/L)^3)

$$N2 = L\left(\frac{x}{L} - \frac{2x^2}{L^2} + \frac{x^3}{L^3}\right)$$

>> N3=3\*x/(L^2)-2\*((x/L)^3)

$$N3 = \frac{3x^2}{L^2} - \frac{2x^3}{L^3}$$

>> N4=-1\*(x^2)/L+(x^3)/(L^2)

$$N4 = -\frac{x^2}{L} + \frac{x^3}{L^2}$$

>> N1\_dev2=diff(diff(N1))

>> N2\_dev2=diff(diff(N2))

>> N3\_dev2=diff(diff(N3))

>> N4\_dev2=diff(diff(N4))

$$N1\_dev2 = -\frac{6}{L^2} + \frac{12x}{L^3}$$

$$N2\_dev2 = L(-4/L^2 + 6x/L^3)$$

$$N3\_dev2 = \frac{6}{L^2} - \frac{12x}{L^3}$$

$$N4_{dev2} = -\frac{2}{L} + \frac{6x}{L^2}$$

$$>> NN\_VEC2 = [N1\_dev2 \ N2\_dev2 \ N3\_dev2 \ N4\_dev2]$$

$$NN\_VEC2 = \begin{bmatrix} \frac{6}{L^2} - \frac{12x}{L^3} & -L(-4/L^2 + 6x/L^3) & -\frac{6}{L^2} + \frac{12x}{L^3} & \frac{2}{L} - \frac{6x}{L^2} \end{bmatrix}$$

$$>> M1 = -(E*I/(L^2))*(-NN\_VEC2)*[0; 0; 3*q*(L^4)/(8*E*I); (1/2)*q*(L^3)/(E*I)]$$

$$M1 = \frac{3qL^2}{8} \left( \frac{12x}{L^3} - \frac{6}{L^2} \right) - \frac{qL}{2} \left( \frac{6x}{L^2} - \frac{2}{L} \right)$$

$$>> M1\_x\_0 = \text{subs}(M1, x, 0)$$

$$M1\_x\_0 =$$

$$-(5*q)/4$$

$$>> M1\_x\_L = \text{subs}(M1, x, L)$$

$$M1\_x\_L =$$

$$q/4$$

$$>> M2 = -(E*I/(L^2))*(-NN\_VEC2)*[3*q*(L^4)/(8*E*I); (1/2)*q*(L^3)/(E*I); ((2/3)*q*(L^4))/(E*I); 0]$$

$$M2 = \frac{7qL^2}{24} \left( \frac{12x}{L^3} - \frac{6}{L^2} \right) - \frac{qL^2}{2} \left( \frac{6x}{L^3} - \frac{4}{L^2} \right)$$

$$>> M2\_x\_0 = \text{subs}(M2, x, 0)$$

$$M2\_x\_0 =$$

$$q/4$$

$$>> M2\_x\_L = \text{subs}(M2, x, L)$$

$$M2\_x\_L =$$

$$(3*q)/4$$

## KOD7-ÇUBUK ELEMAN RJİTLİK MATRİSİNİN HESABI

$$[k_e] = EI \int_0^{L/4} \begin{bmatrix} 1 & 0 & -\frac{48}{L^2} & \frac{128}{L^3} \\ 0 & 1 & -\frac{8}{L} & \frac{16}{L^2} \\ 0 & 0 & \frac{48}{L^2} & -\frac{128}{L^3} \\ 0 & 0 & -\frac{4}{L} & \frac{16}{L^2} \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ -2 \\ -6x \end{bmatrix} [0 \quad 0 \quad -2 \quad -6x] \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix}$$

clear, clc

syms L

syms x

syms q

syms E

syms I

>> A4=[1 0 -48/L^2 128/L^3;0 1 -8/L 16/L^2;0 0 48/L^2 -128/L^3;0 0 -4/L 16/L^2]

$$A4 = \begin{bmatrix} 1 & 0 & -\frac{48}{L^2} & \frac{128}{L^3} \\ 0 & 1 & -\frac{8}{L} & \frac{16}{L^2} \\ 0 & 0 & \frac{48}{L^2} & -\frac{128}{L^3} \\ 0 & 0 & -\frac{4}{L} & \frac{16}{L^2} \end{bmatrix}$$

>> B4=[0;0;-2;-6\*x]

$$B4 = \begin{bmatrix} 0 \\ 0 \\ -2 \\ -6x \end{bmatrix}$$

>> C4=[0 0 -2 -6\*x]

$$C4 = [0 \quad 0 \quad -2 \quad -6x]$$

>> D4=A4.'

$$D4 = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix}$$

>> kL\_2=A4\*B4\*C4\*D4

$$kL_2 = \begin{bmatrix} 768x \frac{\left(\frac{768x}{L^3} - \frac{96}{L^2}\right)}{L^3} - 48 \frac{L^2}{L^2} - \frac{1536x}{L^3} - \frac{192}{L^2} & 96x \frac{\left(\frac{768x}{L^3} - \frac{96}{L^2}\right)}{L^2} - 8 \frac{L}{L} - \frac{1536x}{L^3} - \frac{192}{L^2} & \frac{1536x}{L^3} - \frac{192}{L^2} & \frac{1536x}{L^3} - \frac{192}{L^2} \\ 768x \frac{\left(\frac{96x}{L^2} - \frac{16}{L}\right)}{L^3} - 48 \frac{L^2}{L^2} - \frac{192x}{L^2} - \frac{32}{L} & 96x \frac{\left(\frac{96x}{L^2} - \frac{16}{L}\right)}{L^2} - 8 \frac{L}{L} - \frac{192x}{L^2} - \frac{32}{L} & \frac{192x}{L^2} - \frac{32}{L} & \frac{192x}{L^2} - \frac{32}{L} \\ 48 \frac{\left(\frac{1536x}{L^3} - \frac{192}{L^2}\right)}{L^2} - 768x \frac{L^3}{L^3} - \frac{768x}{L^3} - \frac{96}{L^2} & 8 \frac{\left(\frac{1536x}{L^3} - \frac{192}{L^2}\right)}{L} - 96x \frac{L^2}{L^2} - \frac{768x}{L^3} - \frac{96}{L^2} & \frac{768x}{L^3} - \frac{96}{L^2} & \frac{768x}{L^3} - \frac{96}{L^2} \\ 768x \frac{\left(\frac{96x}{L^2} - \frac{8}{L}\right)}{L^3} - 48 \frac{L^2}{L^2} - \frac{192x}{L^2} - \frac{16}{L} & 96x \frac{\left(\frac{96x}{L^2} - \frac{8}{L}\right)}{L^2} - 8 \frac{L}{L} - \frac{192x}{L^2} - \frac{16}{L} & \frac{192x}{L^2} - \frac{16}{L} & \frac{192x}{L^2} - \frac{16}{L} \end{bmatrix}$$

>> INT\_kL\_2=E\*I\*int(kL\_2,0,L/4)

$$kL_2 = \int_0^{L/4} \begin{bmatrix} 768x \frac{\left(\frac{768x}{L^3} - \frac{96}{L^2}\right)}{L^3} - 48 \frac{L^2}{L^2} - \frac{1536x}{L^3} - \frac{192}{L^2} & 96x \frac{\left(\frac{768x}{L^3} - \frac{96}{L^2}\right)}{L^2} - 8 \frac{L}{L} - \frac{1536x}{L^3} - \frac{192}{L^2} & \frac{1536x}{L^3} - \frac{192}{L^2} & \frac{1536x}{L^3} - \frac{192}{L^2} \\ 768x \frac{\left(\frac{96x}{L^2} - \frac{16}{L}\right)}{L^3} - 48 \frac{L^2}{L^2} - \frac{192x}{L^2} - \frac{32}{L} & 96x \frac{\left(\frac{96x}{L^2} - \frac{16}{L}\right)}{L^2} - 8 \frac{L}{L} - \frac{192x}{L^2} - \frac{32}{L} & \frac{192x}{L^2} - \frac{32}{L} & \frac{192x}{L^2} - \frac{32}{L} \\ 48 \frac{\left(\frac{1536x}{L^3} - \frac{192}{L^2}\right)}{L^2} - 768x \frac{L^3}{L^3} - \frac{768x}{L^3} - \frac{96}{L^2} & 8 \frac{\left(\frac{1536x}{L^3} - \frac{192}{L^2}\right)}{L} - 96x \frac{L^2}{L^2} - \frac{768x}{L^3} - \frac{96}{L^2} & \frac{768x}{L^3} - \frac{96}{L^2} & \frac{768x}{L^3} - \frac{96}{L^2} \\ 768x \frac{\left(\frac{96x}{L^2} - \frac{8}{L}\right)}{L^3} - 48 \frac{L^2}{L^2} - \frac{192x}{L^2} - \frac{16}{L} & 96x \frac{\left(\frac{96x}{L^2} - \frac{8}{L}\right)}{L^2} - 8 \frac{L}{L} - \frac{192x}{L^2} - \frac{16}{L} & \frac{192x}{L^2} - \frac{16}{L} & \frac{192x}{L^2} - \frac{16}{L} \end{bmatrix}$$

$$INT\_kL\_2 = \begin{bmatrix} 768 & 96 & \frac{768}{L^3} - \frac{96}{L^2} & \frac{768}{L^3} - \frac{96}{L^2} \\ -\frac{768}{L^3} - \frac{96}{L^2} & 96 & 8 & 8 \\ \frac{768}{L^3} - \frac{96}{L^2} & 8 & \frac{192x}{L^2} - \frac{32}{L} & \frac{192x}{L^2} - \frac{32}{L} \\ \frac{768}{L^3} - \frac{96}{L^2} & 8 & \frac{192x}{L^2} - \frac{32}{L} & \frac{192x}{L^2} - \frac{32}{L} \end{bmatrix}$$

## KOD8-GEOMETRİK YÜK MATRİSİNİN HESABI

ÇUBUK ELEMANI  $L_s=L/4$  için

$$[n_e] = EI \int_0^{L/4} \begin{bmatrix} 1 & 0 & -\frac{48}{L^2} & \frac{128}{L^3} \\ 0 & 1 & -\frac{8}{L} & \frac{16}{L^2} \\ 0 & 0 & \frac{48}{L^2} & -\frac{128}{L^3} \\ 0 & 0 & -\frac{4}{L} & \frac{16}{L^2} \end{bmatrix} \begin{bmatrix} 0 \\ 1 \\ 2x \\ 3x^2 \end{bmatrix} [0 \quad 1 \quad 2x \quad 3x^2] \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix}$$

clear, clc

syms L

syms x

>> A5=[1 0 -48/L^2 128/L^3;0 1 -8/L 16/L^2;0 0 48/L^2 -128/L^3;0 0 -4/L 16/L^2]

$$A5 = \begin{bmatrix} 1 & 0 & -\frac{48}{L^2} & \frac{128}{L^3} \\ 0 & 1 & -\frac{8}{L} & \frac{16}{L^2} \\ 0 & 0 & \frac{48}{L^2} & -\frac{128}{L^3} \\ 0 & 0 & -\frac{4}{L} & \frac{16}{L^2} \end{bmatrix}$$

>> B5=[0;1;2\*x;3\*x^2]

$$B5 = \begin{bmatrix} 0 \\ 1 \\ 2x \\ 3x^2 \end{bmatrix}$$

>> C5=[0 1 2\*x 3\*x^2]

$$C5 = [0 \quad 1 \quad 2x \quad 3x^2]$$

$$>> D5 = [1 \ 0 \ -48/L^2 \ 128/L^3; 0 \ 1 \ -8/L \ 16/L^2; 0 \ 0 \ 48/L^2 \ -128/L^3; 0 \ 0 \ -4/L \ 16/L^2]$$

$$D5 = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -\frac{48}{L^2} & -\frac{8}{L} & \frac{48}{L^2} & -\frac{4}{L} \\ \frac{128}{L^3} & \frac{16}{L^2} & -\frac{128}{L^3} & \frac{16}{L^2} \end{bmatrix}$$

$$>> nL = A5 * B5 * C5 * D5$$

$$nL = \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{41} & k_{43} & k_{44} \end{bmatrix}$$

$$k_{11} = \frac{96x \left( \frac{96x}{L^2} - \frac{9384x^2}{L^3} \right)}{L^2} - \frac{384x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^3}$$

$$k_{12} = \frac{384x^2}{L^3} - \frac{96x}{L^2} + \frac{16x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L} - \frac{48x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2}$$

$$k_{13} = \frac{384x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2} - \frac{96x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2}$$

$$k_{14} = \frac{8x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L} - \frac{48x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2}$$

$$k_{21} = \frac{384x^2 \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L^3} - \frac{96x \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L^2}$$

$$k_{22} = \frac{48x^2}{L^2} - \frac{16x}{L} - \frac{16x \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L} + \frac{48x^2 \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L^2} + 1$$

$$k_{23} = \frac{96x \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L^2} - \frac{384x^2 \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L^3}$$

$$k_{24} = \frac{48x^2 \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L^2} - \frac{8x \left( \frac{48x^2}{L^2} - \frac{16x}{L} + 1 \right)}{L}$$



$$k_{31} = \frac{384x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^3} - \frac{96x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2}$$

$$k_{32} = \frac{96x}{L^2} - \frac{384x^2}{L^3} - \frac{16x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L} + \frac{48x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2}$$

$$k_{33} = \frac{96x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2} - \frac{384x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^3}$$

$$k_{34} = \frac{48x^2 \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L^2} - \frac{8x \left( \frac{96x}{L^2} - \frac{384x^2}{L^3} \right)}{L}$$

$$k_{41} = \frac{96x \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L^2} - \frac{384x^2 \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L^3}$$

$$k_{42} = \frac{48x^2}{L^2} - \frac{8x}{L} + \frac{16x \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L} - \frac{48x^2 \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L^2}$$

$$k_{43} = \frac{384x^2 \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L^3} - \frac{96x \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L^2}$$

$$k_{44} = \frac{8x \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L} - \frac{48x^2 \left( \frac{8x}{L} - \frac{48x^2}{L^2} \right)}{L^2}$$

$$>> \text{INT\_nL} = \text{int}(nL, 0, L/4)$$

$$\text{INT\_nL} = \int_0^{L/4} \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{41} & k_{43} & k_{44} \end{bmatrix}$$

$$\text{INT\_nL} = \begin{bmatrix} \frac{24}{5L} & \frac{1}{10} & -\frac{24}{5L} & \frac{1}{10} \\ \frac{1}{10} & \frac{L}{30} & -\frac{1}{10} & -\frac{L}{120} \\ -\frac{24}{5L} & -\frac{1}{10} & \frac{24}{5L} & \frac{1}{10} \\ \frac{1}{10} & -\frac{L}{120} & -\frac{1}{10} & \frac{L}{30} \end{bmatrix}$$

## KOD9 -GEOMETRİK YÜK MATRİSİNİN HESABI

ÇUBUK ELEMANI Ls=1 m. için

$$[n_e] = EI \int_0^1 \begin{bmatrix} 1 & 0 & -3 & 2 \\ 0 & 1 & -2 & 1 \\ 0 & 0 & 3 & -2 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ 1 \\ 2x \\ 3x^2 \end{bmatrix} \begin{bmatrix} 0 & 1 & 2x & 3x^2 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -3 & -2 & 3 & -1 \\ 2 & 1 & -2 & 1 \end{bmatrix}$$

clear, clc

syms L

syms x

>> A6=[1 0 0 0;0 1 0 0;-3 -2 3 -1;2 1 -2 1].'

$$A6 = \begin{bmatrix} 1 & 0 & -3 & 2 \\ 0 & 1 & -2 & 1 \\ 0 & 0 & 3 & -2 \\ 0 & 0 & -1 & 1 \end{bmatrix}$$

>> B6=[0;1;2\*x;3\*x^2]

$$B6 = \begin{bmatrix} 0 \\ 1 \\ 2x \\ 3x^2 \end{bmatrix}$$

>> C6=[0 1 2\*x 3\*x^2]

$$C6 = [0 \quad 1 \quad 2x \quad 3x^2]$$

>> D6=[1 0 0 0;0 1 0 0;-3 -2 3 -1;2 1 -2 1].'

$$D6 = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -3 & -2 & 3 & -1 \\ 2 & 1 & -2 & 1 \end{bmatrix}$$

>> nL\_2=A6\*B6\*C6\*D6

$$nL\_2 = \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{41} & k_{43} & k_{44} \end{bmatrix}$$

$$k_{11} = 0$$

$$k_{12} = 6x^2 - 6x$$

$$k_{13} = 12x - 6x(-6x^2 + 6x) + 3x^2(-6x^2 + 6x) - 12x^2$$

$$k_{14} = 4x(-6x^2 + 6x) - 6x - 3x^2(-6x^2 + 6x) + 6x^2$$

$$k_{21} = 0$$

$$k_{22} = 3x^2 - 4x + 1$$

$$k_{23} = 8x + 6x(3x^2 - 4x + 1) - 3x^2(3x^2 - 4x + 1) - 6x^2 - 2$$

$$k_{24} = 3x^2(3x^2 - 4x + 1) - 4x(3x^2 - 4x + 1) - 4x + 3x^2 + 1$$

$$k_{31} = 0$$

$$k_{32} = -6x^2 + 6x$$

$$k_{33} = 6x(-6x^2 + 6x) - 12x - 3x^2(-6x^2 + 6x) + 12x^2$$

$$k_{34} = 6x - 4x(-6x^2 + 6x) + 3x^2(-6x^2 + 6x) - 6x^2$$

$$k_{41} = 0$$

$$k_{42} = 3x^2 - 2x$$

$$k_{43} = 4x - 6x(-3x^2 + 2x) + 3x^2(-3x^2 + 2x) - 6x^2$$

$$k_{44} = 4x(-3x^2 + 2x) - 2x - 3x^2(-3x^2 + 2x) + 3x^2$$

$$>> \text{INT\_nL\_2} = \text{int}(nL\_2, 0, 1)$$

$$\text{INT\_nL\_2} = \int_0^1 \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{41} & k_{43} & k_{44} \end{bmatrix}$$

$$\text{INT\_nL\_2} = \begin{bmatrix} 0 & -1 & -\frac{1}{10} & \frac{1}{10} \\ 0 & 0 & -\frac{3}{10} & \frac{2}{15} \\ 0 & 1 & \frac{1}{10} & -\frac{1}{10} \\ 0 & 0 & \frac{1}{5} & -\frac{1}{30} \end{bmatrix}$$